



June 5, 2025

Christopher M. Gratz, AICP
City of Margate, Development Services Department
901 NW 66th Avenue
Margate, FL 33063

**Re: Abundant Life Church
Trip Generation Analysis**

Dear Mr. Gratz:

Kimley-Horn and Associates, Inc. has completed a trip generation analysis for the proposed modification to the existing Abundant Life Church located at 7955 Royal Palm Boulevard in Margate, Florida. The proposed modification consists of a 17,085 square-foot expansion to the existing building to provide additional space for lobby and space for daycare during church hours. Based on information provided by the applicant, the expansion does not have an impact on the current use of the building. The childcare facilities will continue to be ancillary to the site and will be utilized during hours of service only, as they are currently.

A trip generation analysis was conducted using the Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 11th Edition for the existing development and proposed expansion utilizing ITE Land Use Code (LUC) 560 (Church). Note that to provide a conservative analysis, a multimodal (public transit, bicycle, and pedestrian) was not applied.

As shown in Table 1, the proposed expansion is expected to result in an additional 92 net new weekday daily trips, six (6) net new weekday trips during the A.M. peak period, and six (6) net new weekday trips during the P.M. peak period. Detailed trip generation calculations are included in Attachment A.

Table 1: Proposed Net New Trip Generation				
Daily (A.M. Peak Hour) [P.M. Peak Hour]				
Land Use (ITE Code)	Scale	Entering Trips	Exiting Trips	Net New External Trips
<i>Existing Development</i>				
Church (560)	53,278 square feet	169 (11) [11]	169 (7) [13]	338 (18) [24]
<i>Proposed Redevelopment</i>				
Church (560)	70,363 square feet	215 (15) [13]	215 (9) [17]	430 (24) [30]
<i>Net New Redevelopment</i>				
Net New Vehicle Trips		46 (4) [2]	46 (2) [4]	92 (6) [6]

The results of the trip generation analysis indicates that the proposed expansion does not warrant further study as it generates less than 500 net new daily trips.

If you have any questions, please feel free to contact me.

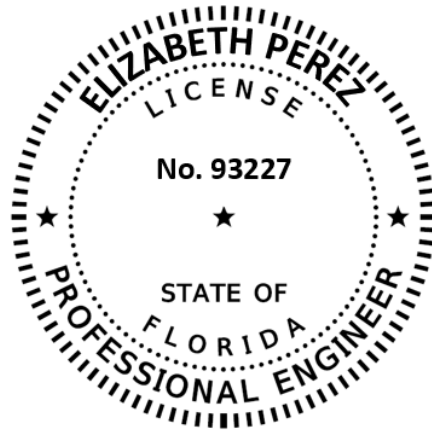
Sincerely,

KIMLEY-HORN AND ASSOCIATED, INC.



Elizabeth Perez, P.E. (FL, PR), PTOE, IMSA Level II

Attachment A – Trip Generation Calculations



Elizabeth Perez, P.E.
Florida Registration Number 93227
Kimley-Horn and Associates, Inc.
8201 Peters Road, Suite 2200
Plantation, Florida 33324

This item has been digitally signed and sealed by Elizabeth Perez, P.E. on the date adjacent to the seal using a SHA authentication code.

Printed copies of this document are not considered signed and sealed and the SHA authentication code must be verified on any electronic copies.

Attachment A
Trip Generation Calculations

DAILY TRIP GENERATION COMPARISON

EXISTING DAILY TRIP GENERATION

		ITE TRIP GENERATION CHARACTERISTICS					DIRECTIONAL DISTRIBUTION		BASELINE TRIPS			MULTIMODAL REDUCTION		GROSS TRIPS			INTERNAL CAPTURE		EXTERNAL VEHICLE TRIPS			PASS-BY CAPTURE		NET NEW EXTERNAL TRIPS		
		Land Use	ITE Edition	ITE Code	Scale	ITE Units	Percent		In	Out	Total	Percent	MR Trips	In	Out	Total	Percent	IC Trips	In	Out	Total	Percent	PB Trips	In	Out	Total
							In	Out																		
GROUP 1	1	Church	11	560	53.278	ksf	50%	50%	169	169	338	0.0%	0	169	169	338	0.0%	0	169	169	338	0.0%	0	169	169	338
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	12																									
	13																									
	14																									
	15																									
		ITE Land Use Code		Rate or Equation		Total:		169	169	338	0.0%	0	169	169	338	0.0%	0	169	169	338	0.0%	0	169	169	338	
		560		Y=5.4*(X)+50.83																						

PROPOSED DAILY TRIP GENERATION

ITE TRIP GENERATION CHARACTERISTICS						DIRECTIONAL DISTRIBUTION		BASELINE TRIPS			MULTIMODAL REDUCTION		GROSS TRIPS			INTERNAL CAPTURE		EXTERNAL VEHICLE TRIPS			PASS-BY CAPTURE		NET NEW EXTERNAL TRIPS				
Land Use		ITE Edition	ITE Code	Scale	ITE Units	Percent		In	Out	Total	Percent	MR Trips	In	Out	Total	Percent	IC Trips	In	Out	Total	Percent	PB Trips	In	Out	Total		
						In	Out																				
GROUP 2	1	Church	11	560	70.363	ksf	50%	50%	215	215	430	0.0%	0	215	215	430	0.0%	0	215	215	430	0.0%	0	215	215	430	
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ITE Land Use Code		Rate or Equation				Total:		215	215	430	0.0%	0	215	215	430	0.0%	0	215	215	430	0.0%	0	215	215	430		
560		Y=5.4*(X)+50.83																									
																						IN		OUT		TOTAL	
NET NEW TRIPS																						46		46		92	

AM PEAK HOUR TRIP GENERATION COMPARISON

EXISTING WEEKDAY AM PEAK HOUR TRIP GENERATION

ITE TRIP GENERATION CHARACTERISTICS						DIRECTIONAL DISTRIBUTION		BASELINE TRIPS			MULTIMODAL REDUCTION		GROSS TRIPS			INTERNAL CAPTURE		EXTERNAL VEHICLE TRIPS			PASS-BY CAPTURE		NET NEW EXTERNAL TRIPS		
	Land Use	ITE Edition	ITE Code	Scale	ITE Units	Percent		In	Out	Total	Percent	MR Trips	In	Out	Total	Percent	IC Trips	In	Out	Total	Percent	PB Trips	In	Out	Total
						In	Out																		
GROUP 1	1 Church	11	560	53.278	ksf	62%	38%	11	7	18	0.0%	0	11	7	18	0.0%	0	11	7	18	0.0%	0	11	7	18
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	15																								
ITE Land Use Code						Rate or Equation		Total:																	
560						Y=0.37*(X)+1.84																			

PROPOSED WEEKDAY AM PEAK HOUR TRIP GENERATION

ITE TRIP GENERATION CHARACTERISTICS						DIRECTIONAL DISTRIBUTION		BASELINE TRIPS			MULTIMODAL REDUCTION		GROSS TRIPS			INTERNAL CAPTURE		EXTERNAL VEHICLE TRIPS			PASS-BY CAPTURE		NET NEW EXTERNAL TRIPS		
	Land Use	ITE Edition	ITE Code	Scale	ITE Units	Percent		In	Out	Total	Percent	MR Trips	In	Out	Total	Percent	IC Trips	In	Out	Total	Percent	PB Trips	In	Out	Total
						In	Out																		
GROUP 2	1 Church	11	560	70.363	ksf	62%	38%	15	9	24	0.0%	0	15	9	24	0.0%	0	15	9	24	0.0%	0	15	9	24
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	14																								
	15																								
ITE Land Use Code						Rate or Equation		Total:																	
560						Y=0.37*(X)+1.84																			

	IN	OUT	TOTAL
NET NEW TRIPS	4	2	6

PM PEAK HOUR TRIP GENERATION COMPARISON

EXISTING WEEKDAY PM PEAK HOUR TRIP GENERATION

		ITE TRIP GENERATION CHARACTERISTICS					DIRECTIONAL DISTRIBUTION		BASELINE TRIPS			MULTIMODAL REDUCTION		GROSS TRIPS			INTERNAL CAPTURE		EXTERNAL VEHICLE TRIPS			PASS-BY CAPTURE		NET NEW EXTERNAL TRIPS		
		Land Use	ITE Edition	ITE Code	Scale	ITE Units	Percent		In	Out	Total	Percent	MR Trips	In	Out	Total	Percent	IC Trips	In	Out	Total	Percent	PB Trips	In	Out	Total
							In	Out																		
GROUP 1	1	Church	11	560	53.278	ksf	44%	56%	11	13	24	0.0%	0	11	13	24	0.0%	0	11	13	24	0.0%	0	11	13	24
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		ITE Land Use Code		Rate or Equation		Total:		11	13	24	0.0%	0	11	13	24	0.0%	0	11	13	24	0.0%	0	11	13	24	
		560		Y=0.36*(X)+4.7																						

PROPOSED WEEKDAY PM PEAK HOUR TRIP GENERATION

		ITE TRIP GENERATION CHARACTERISTICS					DIRECTIONAL DISTRIBUTION		BASELINE TRIPS			MULTIMODAL REDUCTION		GROSS TRIPS			INTERNAL CAPTURE		EXTERNAL VEHICLE TRIPS			PASS-BY CAPTURE		NET NEW EXTERNAL TRIPS		
		Land Use	ITE Edition	ITE Code	Scale	ITE Units	Percent		In	Out	Total	Percent	MR Trips	In	Out	Total	Percent	IC Trips	In	Out	Total	Percent	PB Trips	In	Out	Total
							In	Out																		
GROUP 2	1	Church	11	560	70.363	ksf	44%	56%	13	17	30	0.0%	0	13	17	30	0.0%	0	13	17	30	0.0%	0	13	17	30
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		ITE Land Use Code		Rate or Equation		Total:		13	17	30	0.0%	0	13	17	30	0.0%	0	13	17	30	0.0%	0	13	17	30	
		560		Y=0.36*(X)+4.7																						

	IN	OUT	TOTAL
NET NEW TRIPS	2	4	6



July 3, 2025

Sean Thomas
Executive Pastor
7955 Royal Palm Blvd
Margate Florida 33063

***Re: Abundant Life Church, Margate FL
Parking Analysis***

Dear Mr. Thomas:

Requirements Per City Code

According to Section 40.705 of Division 1, Article 7 of the City of Margate Municode, before any building or engineering permit for a new parking area, change of use, or substantial modification to an existing parking area (such as alterations to vehicle circulation or expansion/reduction of the parking area) can be issued, a property owner must submit a master parking plan to the City for review and approval. This master parking plan must be prepared by a professional engineer licensed in the State of Florida and include a justification for the number of parking spaces provided.

Justification

The proposed expansion will increase the size of the children's area and office space of the church. However, no additional vehicular trips are expected as a result of the expansion, therefore the in-place parking capacity should be maintained. The expansion aims to enhance the facilities for existing users. Since the expansion will occupy part of the existing parking lot, several parking spots will be put out of use. The proposed improvements add additional parking spaces to compensate for the parking spaces lost due to the expansion. The existing total parking capacity is 403 parking spaces, and the proposed total parking capacity will be increased to 404 parking spaces, maintaining the existing parking capacity.

If you have any questions regarding this analysis, please feel free to contact me.

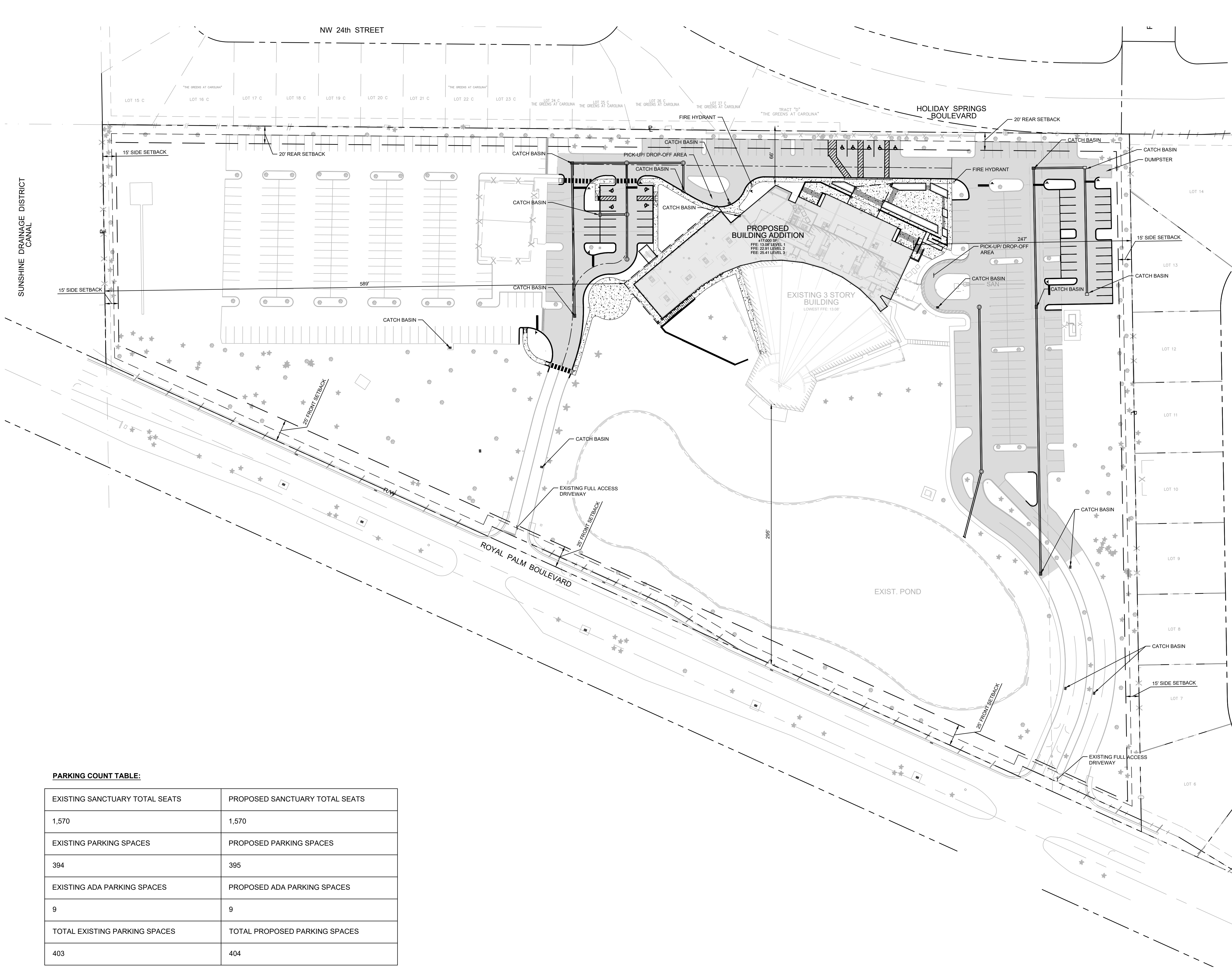
Sincerely,
Kimley-Horn and Associates, Inc.
Rodrigo Torres, P.E.

Attachment
Master Parking Plan

Plotted By: Torres, Rodrigo. Sheet Set: ABUNDANT LIFE CHURCH Layout: SP-02 MASTER PARKING PLAN. July 03, 2025, 02:54:11pm. K:\181_civil\143 Jobs\143788000 abundant life church\margate\Design\CADD\plansheets\SP-02 MASTER PARKING PLAN.dwg. This document, together with the concepts and designs presented herein, as an instrument of service, is intended only for the specific purpose and client for which it was prepared. Reuse of and improper reliance on this document without written authorization and adaptation by Kimley-Horn and Associates, Inc. shall be without liability to Kimley-Horn and Associates, Inc.

PARKING COUNT TABLE:

EXISTING SANCTUARY TOTAL SEATS	PROPOSED SANCTUARY TOTAL SEATS
1,570	1,570
EXISTING PARKING SPACES	PROPOSED PARKING SPACES
394	395
EXISTING ADA PARKING SPACES	PROPOSED ADA PARKING SPACES
9	9
TOTAL EXISTING PARKING SPACES	TOTAL PROPOSED PARKING SPACES
403	404



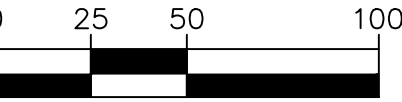
NOTES:

1. A REGISTERED LAND SURVEYOR SHALL REPLACE SURVEY MONUMENTS OR BENCHMARKS, WHICH HAVE TO BE DISTURBED BY THIS WORK, UPON COMPLETION OF WORK.
2. HANDICAP PARKING SPACES SHALL BE MARKED BY THE INTERNATIONAL HANDICAP SYMBOL AND EACH SPACE SHALL BE PROVIDED WITH A SIGN STATING "PARKING BY DISABLED PERMIT ONLY". ALL HANDICAP ACCESS POINTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH APPLICABLE HANDICAP CODES.
3. ALL CONSTRUCTION SHALL CONFORM TO THE SPECIFICATIONS AND STANDARDS OF BROWARD COUNTY.
4. PRIOR TO STARTING CONSTRUCTION, THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE TO VERIFY ALL REQUIRED PERMITS AND APPROVALS HAVE BEEN OBTAINED. NO CONSTRUCTION OR FABRICATION OF ANY ITEM SHALL BEGIN UNTIL THE CONTRACTOR HAS RECEIVED ALL PLANS AND ANY OTHER DOCUMENTATION FROM ALL OF THE PERMITTING AND ANY OTHER REGULATORY AUTHORITIES.
5. CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL SETBACKS AND/OR EASEMENTS BEFORE BEGINNING CONSTRUCTION.
6. ALL BUILDING DIMENSIONS ARE TO THE FACE OF THE BUILDING.
7. PARKING STALLS TO BE MARKED WITH WHITE PAINT.
8. WHERE APPLICABLE DOUBLE YELLOW STRIPING, HANDICAP, STOP BARS, AND FIRE LANE STRIPING TO BE THERMOPLASTIC.
9. THIS PROJECT IS LOCATED IN FLOOD ZONE 12011C0185H, WITH AN EFFECTIVE DATE OF 08/18/2014.

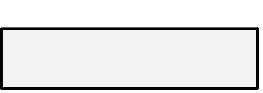


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Check positive response codes before you dig!

GRAPHIC SCALE IN FEET



LEGEND

	PROPERTY LINE
	EXIST. RIGHT OF WAY
	PROP. ROAD CENTERLINE
	PROP. BUILDING ADDITION

CHURCH SITE IMPROVEMENTS PREPARED FOR ABUNDANT LIFE CHURCH CITY OF MARGATE FL

SHEET NUMBER SP-02

MASTER PARKING PLAN

FL



© 2025 KIMLEY-HORN AND ASSOCIATES, INC.
WWW.KIMLEY-HORN.COM REGISTRY No. 35108
PHONE 954-535-5100 FAX 954-739-2247
8200 PETERS ROAD, SUITE 2200, PLANTATION, FL 33324

No.	REVISIONS	DATE	BY



Concurrency Analysis Report for
The City of Margate & Broward County

Abundant Life Church

7955 Royal Palm Blvd, Margate, FL 33063

Prepared by:

Kimley-Horn and Associates, Inc.
8201 Peters Road, Plantation, FL 33324
Project Number: 143788000

July 3, 2025

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Kimley»»Horn



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3 Level of Service (LOS) Standards Compliance	1
3.1 Potable Water	1
3.2 Sanitary Sewer	2
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3.4 Drainage	2
3.5 Transportation	2
3.6 Parks and Recreation	3
4 Conclusion	3

LIST OF APPENDICIES

- A SITE PLAN
- B TRAFFIC IMPACT STATEMENT
- C BOUNDARY SURVEY
- D PROOF OF OWNERSHIP
- E ERC SUMMARY
- F DRAINAGE REPORT

1 PROJECT DESCRIPTION

The proposed project will consist of a building expansion and various site modifications to the existing Abundant Life Church. The site currently consists of an existing +/- 29,000 SF structure, paved parking lot, and grass lined detention pond. These site improvements are to include sidewalk modifications and minor grading modifications as required. The building expansion will add 17,000 square feet to the existing 29,000 square feet, dedicated solely to a proposed children's center and play area. The church's current seating capacity and interior sanctuary square footage will remain unchanged.

2 PURPOSE

The purpose of this Concurrency Analysis Report is to demonstrate compliance with the City of Margate's Concurrency Management System as outlined in Division 5, Article III of the City of Margate's Land Development Code. This report ensures that the proposed development project will not negatively impact the city's ability to maintain adopted Level of Service (LOS) standards for essential public facilities and services, including potable water, sanitary sewer, solid waste, drainage, transportation and parks and recreation. By evaluating the capacity and availability of these services in relation to the projected demands of the development, this analysis confirms that the project aligns with the city's growth management objectives and infrastructure planning strategies. The findings herein support the issuance of a concurrency determination, affirming that the development may proceed without compromising the health, safety, and welfare of the community or overburdening public resources.

3 LEVEL OF SERVICE (LOS) STANDARDS COMPLIANCE

Development activity shall not be approved unless it is demonstrated that essential public services will be available at the required levels of service (LOS) to accommodate the impacts of the proposed development. However, temporary reductions in LOS may be permitted during the construction of new facilities within a specific area, provided that the required LOS will be achieved upon the completion of those facilities.

Per Section 40.345 of the City of Margate's Municode, the following public facilities must maintain adopted LOS standards:

3.1 POTABLE WATER

The City of Margate uses a gallons per capita per day (gpcd) metric to define its LOS for potable water. This standard is based on historical average daily consumption and projected population growth. Development permits may only be issued if it can be demonstrated that sufficient potable water capacity exists to meet the projected demand and if the demand from the proposed development, when added to existing and committed demands, does not exceed the available capacity. Any new facilities required to meet demand will be operational concurrent with the development's impact. Refer to Appendix E of this report for the ERC Summary breakdown. This breakdown illustrates that the proposed expansion and improvements will increase the water intake in gallons per day per ERC. However, the existing and proposed infrastructure together will suffice for the added flow taken on by the development, which will satisfy the concurrency standards set forth by the City of Margate's Municode and Comprehensive Plan.

3.2 SANITARY SEWER

Concurrency wastewater requirements for the City of Margate specify that the development must not exceed the available capacity of the wastewater system. The system must be able to serve both existing and projected users at a level that meets health, safety, and performance objectives. Developers must demonstrate that the proposed project will not degrade the LOS below acceptable thresholds. Refer to Appendix E of this report for the ERC Summary breakdown. This breakdown illustrates that the proposed expansion and improvements will increase the usage in sewer gallons per day per ERC. However, the existing and proposed infrastructure together will suffice for the added flow taken on by the development, which will satisfy the concurrency standards set forth by the City of Margate's Municode and Comprehensive Plan.

3.3 SOLID WASTE

The LOS for solid waste is typically expressed in pounds per capita per day (lbs/capita/day) or cubic yards per dwelling unit per year, depending on the type of development. These standards are based on historical waste generation rates and are used to project future capacity needs. New development must demonstrate that it will not exceed the city's solid waste collection and disposal capacity. The city must verify that adequate collection services and disposal infrastructure are available concurrent with the development's impact. There is no change in the total amount of waste generated as the use of the building will remain a church on Wednesdays and Sundays, so the amount of trash generated is not expected to increase. Therefore, this will meet concurrency standards set forth by the City of Margate's Municode and Comprehensive Plan.

3.4 DRAINAGE

The City of Margate requires that all new development and redevelopment projects be designed to manage stormwater runoff from a 25-year, 3-day design storm event. This standard ensures that stormwater systems can handle significant rainfall without causing localized flooding or system failure. Adequate drainage capacity must be available to serve the proposed development concurrent with its impact. Developers must demonstrate that the project will not degrade the LOS of the existing drainage system or contribute to flooding. Drainage calculations were run on the proposed site including the site modifications and improvements. Please refer to Appendix F of this report for the Drainage Report and calculations. These calculations reflected the pre vs. post conditions the 25-year, 3-day storm event had on the proposed site. The post development conditions ended up producing less runoff than the predevelopment conditions which ensures that the designed system can handle significant rainfall without the system failing or causing localized flooding, thus meeting the concurrency standards set forth by the City of Margate's Municode and Comprehensive Plan.

3.5 TRANSPORTATION

The City of Margate adopts Level of Service "D" as the minimum acceptable standard for most roadways within its jurisdiction. For constrained or regionally significant corridors, the City of Margate may coordinate with Broward County and the Florida Department of Transportation (FDOT) to apply alternative LOS standards. Development proposals must demonstrate that the additional traffic generated will not cause the LOS on adjacent or impacted roadways to fall below the adopted standard. A Traffic Impact Statement (TIS) is required for developments generating 500 or more daily trips and must evaluate

impacts within a one-mile radius of the site. Appendix B of this report outline the traffic impact analysis associated with the proposed improvements for the project. This traffic impact analysis reflects that the proposed expansion and improvements does not warrant further study as it generates less than 500 net new daily trips. This means that the traffic level of service associated with this project is not increasing or decreasing from the existing level, therefore meeting the concurrency standards set forth by the City of Margate's Municode and Comprehensive Plan.

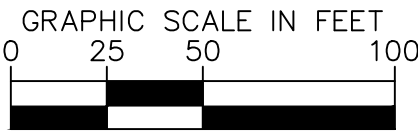
3.6 PARKS AND RECREATION

The City of Margate has adopted a minimum LOS standard of 3.0 acres of parkland per 1,000 residents. This includes both active (e.g., sports fields, playgrounds) and passive (e.g., walking trails, picnic areas) recreational facilities. New residential development must demonstrate that it will not reduce the citywide LOS below the adopted standard. If the development increases demand beyond available capacity, the developer may be required to dedicate land for park use, pay a parks impact fee, or provide on-site recreational amenities. Due to the project not being a residential zone or residential area, this concurrency standard does not apply and therefore does not need to be calculated for this concurrency analysis report.

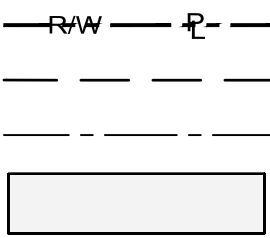
4 CONCLUSION

Based on the comprehensive review and analysis presented in this report, the proposed development has been evaluated against all applicable concurrency requirements as outlined in Division 5 of the City of Margate's Land Development Code and the City's Comprehensive Plan. The project has demonstrated compliance with the adopted Level of Service (LOS) standards for all essential public facilities, including potable water, wastewater, solid waste, drainage, transportation, and parks and recreation. The analysis confirms that adequate capacity exists within each service area to accommodate the projected demands of the development without degrading existing service levels. Where applicable, supporting documentation such as traffic impact studies and utility capacity confirmations have been provided to substantiate this compliance. Furthermore, the development aligns with the City's growth management objectives and infrastructure planning strategies, ensuring that public services will be available concurrent with the impacts of the project. As such, the proposed development satisfies all concurrency requirements and is eligible to proceed through the approval process in accordance with the City of Margate's regulations.

Appendix A – Site Plan



LEGEND



PROPERTY LINE
EXIST. RIGHT OF WAY
PROP. ROAD CENTERLINE
PROP. BUILDING ADDITION

NOTES:

1. A REGISTERED LAND SURVEYOR SHALL REPLACE SURVEY MONUMENTS OR BENCHMARKS, WHICH HAVE TO BE DISTURBED BY THIS WORK, UPON COMPLETION OF WORK.
 2. HANDICAP PARKING SPACES SHALL BE MARKED BY THE INTERNATIONAL HANDICAP SYMBOL AND EACH SPACE SHALL BE PROVIDED WITH A SIGN STATING "PARKING BY DISABLED PERMIT ONLY". ALL HANDICAP ACCESS POINTS SHALL BE CONSTRUCTED IN ACCORDANCE WITH APPLICABLE HANDICAP CODES.
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 4. PRIOR TO STARTING CONSTRUCTION, THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE TO VERIFY ALL NECESSARY PERMITS HAVE BEEN OBTAINED. NO CONSTRUCTION OR FABRICATION OF ANY ITEM SHALL BEGIN UNTIL THE CONTRACTOR HAS RECEIVED ALL PLANS AND ALL OTHER DOCUMENTATION FROM ALL OF THE PERMITTING AND ANY OTHER REGULATORY AUTHORITIES.
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 7. PARKING STALLS TO BE MARKED WITH WHITE PAINT.
 8. WHERE APPLICABLE DOUBLE YELLOW STRIPING, HANDICAP STOP BARS, AND FIRE LANE STRIPINGS TO BE THERMOPLASTIC.
- THIS PROJECT IS LOCATED IN FLOOD ZONE 1201C0165H, WITH AN EFFECTIVE DATE OF 08/18/2014.

ZONING		
SETBACK	REQUIRED	PROVIDED
FRONT	25 Ft	295 Ft
SIDE	15 Ft	247 Ft
REAR	20 Ft	60 Ft
FAR	0.30	0.07



Call 811 or visit sunshine811.com two full business days before digging to have buried facilities located and marked.

Check positive response codes before you dig!

**CHURCH SITE
IMPROVEMENTS**

PREPARED FOR
ABUNDANT LIFE CHURCH
CITY OF MARGATE

SHEET NUMBER
SP-01

OVERALL SITE PLAN

KHA PROJECT 143788000	DATE 7/2/2025	SCALE	AS SHOWN	RT	KH	ARM
		DESIGNED BY				
		DRAWN BY				
		CHECKED BY				

Kimley»»Horn

© 2025 KIMLEY-HORN AND ASSOCIATES, INC.
WWW.KIMLEY-HORN.COM REGISTRY No. 35106
PHONE: 954-535-5100 FAX: 954-739-2247
8200 PETERS ROAD, SUITE 2200, PLANTATION, FL 33324

No.	REVISIONS	DATE	BY
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Appendix B – Traffic Impact Statement



June 5, 2025

Christopher M. Gratz, AICP
City of Margate, Development Services Department
901 NW 66th Avenue
Margate, FL 33063

**Re: Abundant Life Church
Trip Generation Analysis**

Dear Mr. Gratz:

Kimley-Horn and Associates, Inc. has completed a trip generation analysis for the proposed modification to the existing Abundant Life Church located at 7955 Royal Palm Boulevard in Margate, Florida. The proposed modification consists of a 17,085 square-foot expansion to the existing building to provide additional space for lobby and space for daycare during church hours. Based on information provided by the applicant, the expansion does not have an impact on the current use of the building. The childcare facilities will continue to be ancillary to the site and will be utilized during hours of service only, as they are currently.

A trip generation analysis was conducted using the Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 11th Edition for the existing development and proposed expansion utilizing ITE Land Use Code (LUC) 560 (Church). Note that to provide a conservative analysis, a multimodal (public transit, bicycle, and pedestrian) was not applied.

As shown in Table 1, the proposed expansion is expected to result in an additional 92 net new weekday daily trips, six (6) net new weekday trips during the A.M. peak period, and six (6) net new weekday trips during the P.M. peak period. Detailed trip generation calculations are included in Attachment A.

Table 1: Proposed Net New Trip Generation				
Daily (A.M. Peak Hour) [P.M. Peak Hour]				
Land Use (ITE Code)	Scale	Entering Trips	Exiting Trips	Net New External Trips
<i>Existing Development</i>				
Church (560)	53,278 square feet	169 (11) [11]	169 (7) [13]	338 (18) [24]
<i>Proposed Redevelopment</i>				
Church (560)	70,363 square feet	215 (15) [13]	215 (9) [17]	430 (24) [30]
<i>Net New Redevelopment</i>				
Net New Vehicle Trips		46 (4) [2]	46 (2) [4]	92 (6) [6]

The results of the trip generation analysis indicates that the proposed expansion does not warrant further study as it generates less than 500 net new daily trips.

If you have any questions, please feel free to contact me.

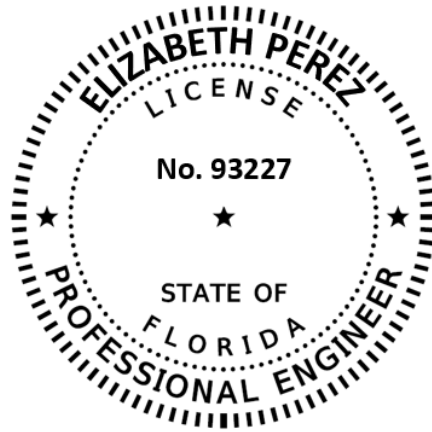
Sincerely,

KIMLEY-HORN AND ASSOCIATED, INC.



Elizabeth Perez, P.E. (FL, PR), PTOE, IMSA Level II

Attachment A – Trip Generation Calculations



Elizabeth Perez, P.E.
Florida Registration Number 93227
Kimley-Horn and Associates, Inc.
8201 Peters Road, Suite 2200
Plantation, Florida 33324

This item has been digitally signed and sealed by Elizabeth Perez, P.E. on the date adjacent to the seal using a SHA authentication code.

Printed copies of this document are not considered signed and sealed and the SHA authentication code must be verified on any electronic copies.

Attachment A
Trip Generation Calculations

DAILY TRIP GENERATION COMPARISON

EXISTING DAILY TRIP GENERATION

		ITE TRIP GENERATION CHARACTERISTICS					DIRECTIONAL DISTRIBUTION		BASELINE TRIPS			MULTIMODAL REDUCTION		GROSS TRIPS			INTERNAL CAPTURE		EXTERNAL VEHICLE TRIPS			PASS-BY CAPTURE		NET NEW EXTERNAL TRIPS		
		Land Use	ITE Edition	ITE Code	Scale	ITE Units	Percent		In	Out	Total	Percent	MR Trips	In	Out	Total	Percent	IC Trips	In	Out	Total	Percent	PB Trips	In	Out	Total
							In	Out																		
GROUP 1	1	Church	11	560	53.278	ksf	50%	50%	169	169	338	0.0%	0	169	169	338	0.0%	0	169	169	338	0.0%	0	169	169	338
	2																									
	3																									
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	9																									
	10																									
	11																									
	12																									
	13																									
	14																									
	15																									
		ITE Land Use Code		Rate or Equation		Total:		169	169	338	0.0%	0	169	169	338	0.0%	0	169	169	338	0.0%	0	169	169	338	
		560		Y=5.4*(X)+50.83																						

PROPOSED DAILY TRIP GENERATION

ITE TRIP GENERATION CHARACTERISTICS						DIRECTIONAL DISTRIBUTION		BASELINE TRIPS			MULTIMODAL REDUCTION		GROSS TRIPS			INTERNAL CAPTURE		EXTERNAL VEHICLE TRIPS			PASS-BY CAPTURE		NET NEW EXTERNAL TRIPS				
Land Use		ITE Edition	ITE Code	Scale	ITE Units	Percent		In	Out	Total	Percent	MR Trips	In	Out	Total	Percent	IC Trips	In	Out	Total	Percent	PB Trips	In	Out	Total		
						In	Out																				
GROUP 2	1	Church	11	560	70.363	ksf	50%	50%	215	215	430	0.0%	0	215	215	430	0.0%	0	215	215	430	0.0%	0	215	215	430	
	2																										
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	9																										
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	13																										
	14																										
	15																										
ITE Land Use Code		Rate or Equation				Total:		215	215	430	0.0%	0	215	215	430	0.0%	0	215	215	430	0.0%	0	215	215	430		
560		Y=5.4*(X)+50.83																									
																						IN		OUT		TOTAL	
NET NEW TRIPS																						46		46		92	

AM PEAK HOUR TRIP GENERATION COMPARISON

EXISTING WEEKDAY AM PEAK HOUR TRIP GENERATION

ITE TRIP GENERATION CHARACTERISTICS						DIRECTIONAL DISTRIBUTION		BASELINE TRIPS			MULTIMODAL REDUCTION		GROSS TRIPS			INTERNAL CAPTURE		EXTERNAL VEHICLE TRIPS			PASS-BY CAPTURE		NET NEW EXTERNAL TRIPS		
	Land Use	ITE Edition	ITE Code	Scale	ITE Units	Percent		In	Out	Total	Percent	MR Trips	In	Out	Total	Percent	IC Trips	In	Out	Total	Percent	PB Trips	In	Out	Total
						In	Out																		
GROUP 1	1 Church	11	560	53.278	ksf	62%	38%	11	7	18	0.0%	0	11	7	18	0.0%	0	11	7	18	0.0%	0	11	7	18
	2																								
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	9																								
	10																								
	11																								
	12																								
	13																								
	14																								
	15																								
ITE Land Use Code						Rate or Equation		Total:																	
560						Y=0.37*(X)+1.84																			

PROPOSED WEEKDAY AM PEAK HOUR TRIP GENERATION

ITE TRIP GENERATION CHARACTERISTICS						DIRECTIONAL DISTRIBUTION		BASELINE TRIPS			MULTIMODAL REDUCTION		GROSS TRIPS			INTERNAL CAPTURE		EXTERNAL VEHICLE TRIPS			PASS-BY CAPTURE		NET NEW EXTERNAL TRIPS		
	Land Use	ITE Edition	ITE Code	Scale	ITE Units	Percent		In	Out	Total	Percent	MR Trips	In	Out	Total	Percent	IC Trips	In	Out	Total	Percent	PB Trips	In	Out	Total
						In	Out																		
GROUP 2	1 Church	11	560	70.363	ksf	62%	38%	15	9	24	0.0%	0	15	9	24	0.0%	0	15	9	24	0.0%	0	15	9	24
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	10																								
	11																								
	12																								
	13																								
	14																								
	15																								
ITE Land Use Code						Rate or Equation		Total:																	
560						Y=0.37*(X)+1.84																			

	IN	OUT	TOTAL
NET NEW TRIPS	4	2	6

PM PEAK HOUR TRIP GENERATION COMPARISON

EXISTING WEEKDAY PM PEAK HOUR TRIP GENERATION

		ITE TRIP GENERATION CHARACTERISTICS					DIRECTIONAL DISTRIBUTION		BASELINE TRIPS			MULTIMODAL REDUCTION		GROSS TRIPS			INTERNAL CAPTURE		EXTERNAL VEHICLE TRIPS			PASS-BY CAPTURE		NET NEW EXTERNAL TRIPS		
		Land Use	ITE Edition	ITE Code	Scale	ITE Units	Percent		In	Out	Total	Percent	MR Trips	In	Out	Total	Percent	IC Trips	In	Out	Total	Percent	PB Trips	In	Out	Total
							In	Out																		
GROUP 1	1	Church	11	560	53.278	ksf	44%	56%	11	13	24	0.0%	0	11	13	24	0.0%	0	11	13	24	0.0%	0	11	13	24
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	13																									
	14																									
	15																									
		ITE Land Use Code		Rate or Equation		Total:		11	13	24	0.0%	0	11	13	24	0.0%	0	11	13	24	0.0%	0	11	13	24	
		560		Y=0.36*(X)+4.7																						

PROPOSED WEEKDAY PM PEAK HOUR TRIP GENERATION

		ITE TRIP GENERATION CHARACTERISTICS					DIRECTIONAL DISTRIBUTION		BASELINE TRIPS			MULTIMODAL REDUCTION		GROSS TRIPS			INTERNAL CAPTURE		EXTERNAL VEHICLE TRIPS			PASS-BY CAPTURE		NET NEW EXTERNAL TRIPS		
		Land Use	ITE Edition	ITE Code	Scale	ITE Units	Percent		In	Out	Total	Percent	MR Trips	In	Out	Total	Percent	IC Trips	In	Out	Total	Percent	PB Trips	In	Out	Total
							In	Out																		
GROUP 2	1	Church	11	560	70.363	ksf	44%	56%	13	17	30	0.0%	0	13	17	30	0.0%	0	13	17	30	0.0%	0	13	17	30
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	11																									
	12																									
	13																									
	14																									
	15																									
		ITE Land Use Code		Rate or Equation		Total:		13	17	30	0.0%	0	13	17	30	0.0%	0	13	17	30	0.0%	0	13	17	30	
		560		Y=0.36*(X)+4.7																						

	IN	OUT	TOTAL
NET NEW TRIPS	2	4	6

Appendix C – Boundary Survey

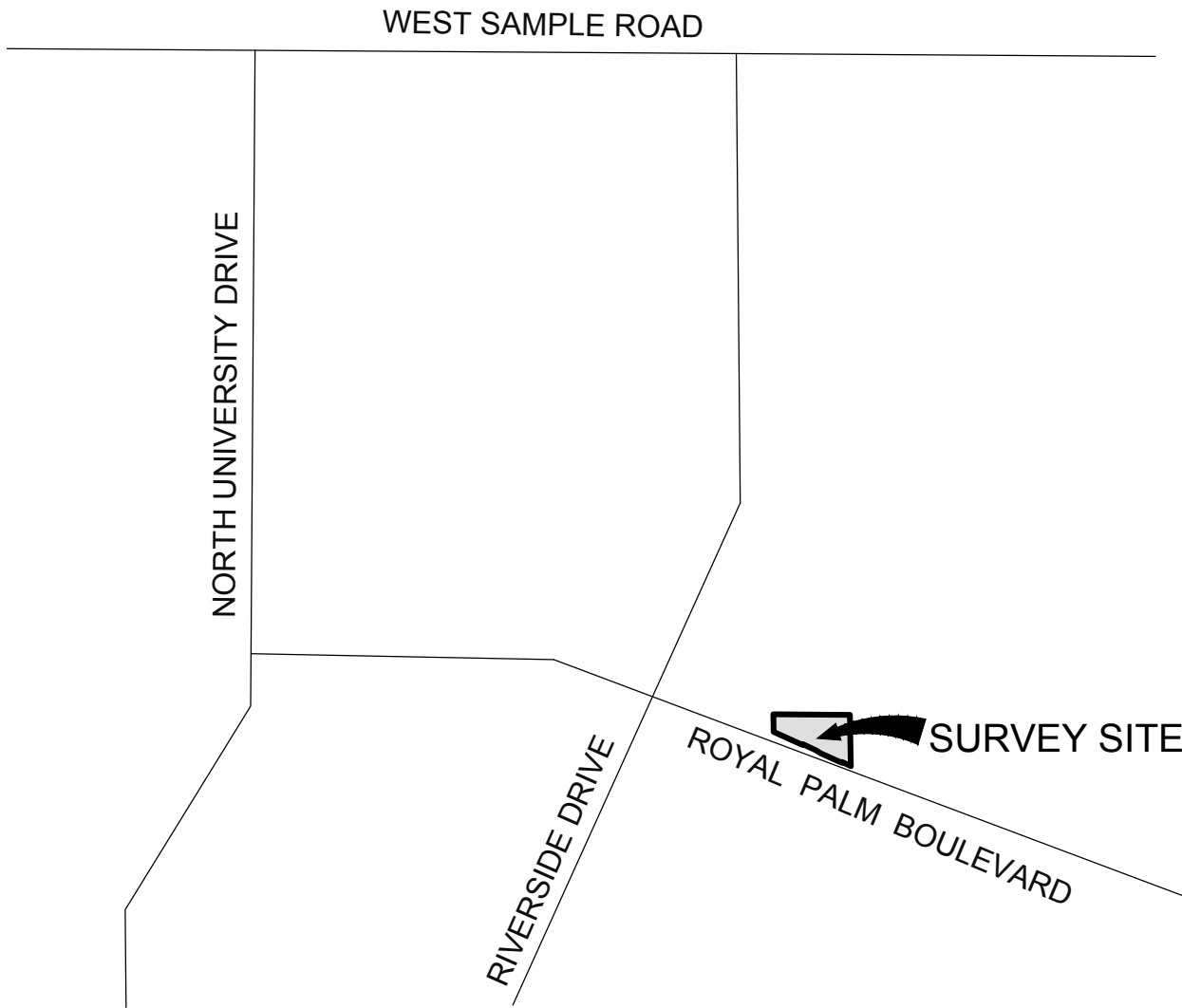
LEGAL DESCRIPTION:

PARCEL "A" AND "B" "THE LUPTON PLAT", ACCORDING TO THE PLAT THEREOF, AS RECORDED IN PLAT BOOK 122, PAGE 30, OF THE PUBLIC RECORDS OF BROWARD COUNTY, FLORIDA.

SAID LANDS LYING AND BEING IN THE CITY OF MARGATE, BROWARD COUNTY, FLORIDA AND CONTAINING A TOTAL OF 610,532 SQUARE FEET (14.016 ACRES), MORE OR LESS.

NOTES:

- THIS SURVEY MAP AND REPORT OR THE COPIES THEREOF ARE NOT VALID WITHOUT THE ORIGINAL SIGNATURE AND SEAL OR AN ELECTRONIC SIGNATURE AND ELECTRONIC SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER.
- ELEVATIONS ARE BASED ON NORTH AMERICAN VERTICAL DATUM OF 1988. BROWARD COUNTY BENCHMARK #2279; ELEVATION: 8.408 FEET.
- FLOOD ZONE: AE/X(0.2%)/X; BASE FLOOD ELEVATION: 12 FEET/NONE/NONE; PANEL #12011C0165H; COMMUNITY #120047; MAP DATE: 8/18/14.
- THIS SITE LIES IN SECTION 26, TOWNSHIP 48 SOUTH, RANGE 41 EAST, BROWARD COUNTY, FLORIDA.
- BEARINGS ARE BASED ON STATE PLANE COORDINATES AND ARE RELATIVE TO THE NORTH AMERICAN DATUM OF 1983 WITH 1990 ADJUSTMENT, FL-E ZONE, WITH THE NORTH LINE OF PARCEL "A" BEING S89°28'44"E.
- REASONABLE EFFORTS WERE MADE REGARDING THE EXISTENCE AND THE LOCATION OF UNDERGROUND UTILITIES. THIS FIRM, HOWEVER, DOES NOT ACCEPT RESPONSIBILITY FOR THIS INFORMATION. BEFORE EXCAVATION OR CONSTRUCTION CONTACT BLOOD HOUND UNDERGROUND UTILITY LOCATIONS FOR FIELD VERIFICATION.
- THIS SURVEY IS CERTIFIED EXCLUSIVELY TO: ABUNDANT LIFE CHURCH.
- THE HORIZONTAL POSITIONAL ACCURACY OF WELL DEFINED IMPROVEMENTS ON THIS SURVEY IS ±0.2'. THE VERTICAL ACCURACY OF ELEVATIONS OF WELL DEFINED IMPROVEMENTS ON THIS SURVEY IS ±0.1'.
- THIS SURVEY WAS PREPARED WITHOUT BENEFIT OF A COMMITMENT FOR TITLE INSURANCE. ONLY PLATTED OR KNOWN EASEMENTS ARE DEPICTED HEREON.
- ALL RECORDED DOCUMENTS ARE PER BROWARD COUNTY PUBLIC RECORDS.
- THE FLOOD ZONE DESIGNATION LINES DEPICTED HEREON ARE PLACED IN APPROXIMATE LOCATIONS, AS SCALED FROM THE IMAGE FROM THE FEMA.ORG WEBSITE.
- THIS SITE CONTAINS 403 TOTAL CLEARLY IDENTIFIABLE PARKING SPACES (394 REGULAR & 9 DISABLED).
- BOUNDARY CALCULATIONS PER COORDINATES ON THE LUPTON PLAT BOOK 122, PAGE 30.



CERTIFICATION:

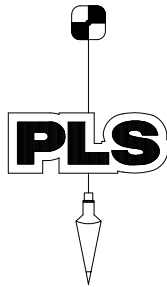
I HEREBY CERTIFY: THAT THIS MAP OF SURVEY MEETS THE STANDARDS OF PRACTICE SET FORTH BY THE FLORIDA STATE BOARD OF PROFESSIONAL SURVEYORS AND MAPPERS IN CHAPTER 5J-17, FLORIDA ADMINISTRATIVE CODE, PURSUANT TO SECTION 472.027, FLORIDA STATUTES.

- ☐ JOHN F. PULICE, PROFESSIONAL SURVEYOR AND MAPPER LS2691
- ☐ VICTOR R. GILBERT, PROFESSIONAL SURVEYOR AND MAPPER LS6274
- ☐ MICHAEL M. MOSSEY, PROFESSIONAL SURVEYOR AND MAPPER PSM5660
- STATE OF FLORIDA

5		
4		
3		
2	#72824 -UPDATE SURVEY-6/19/24	J.S.
1	#71148 -ORIGINAL SURVEY-3/21/23	R.V.
NO.	REVISIONS	BY

ABUNDANT LIFE CHURCH
7955 ROYAL PALM BOULEVARD
MARGATE, FLORIDA 33063
(CITY OF MARGATE, BROWARD COUNTY)

BOUNDARY AND TOPOGRAPHIC SURVEY



PULICE LAND SURVEYORS, INC.
5381 NOB HILL ROAD
SUNRISE, FLORIDA 33351
TELEPHONE: (954) 572-1777
FAX: (954) 572-1778
E-MAIL: surveys@pulicelandsurveyors.com
WEBSITE: www.pulicelandsurveyors.com
CERTIFICATE OF AUTHORIZATION LB#3870

DRAWN BY: R.V.

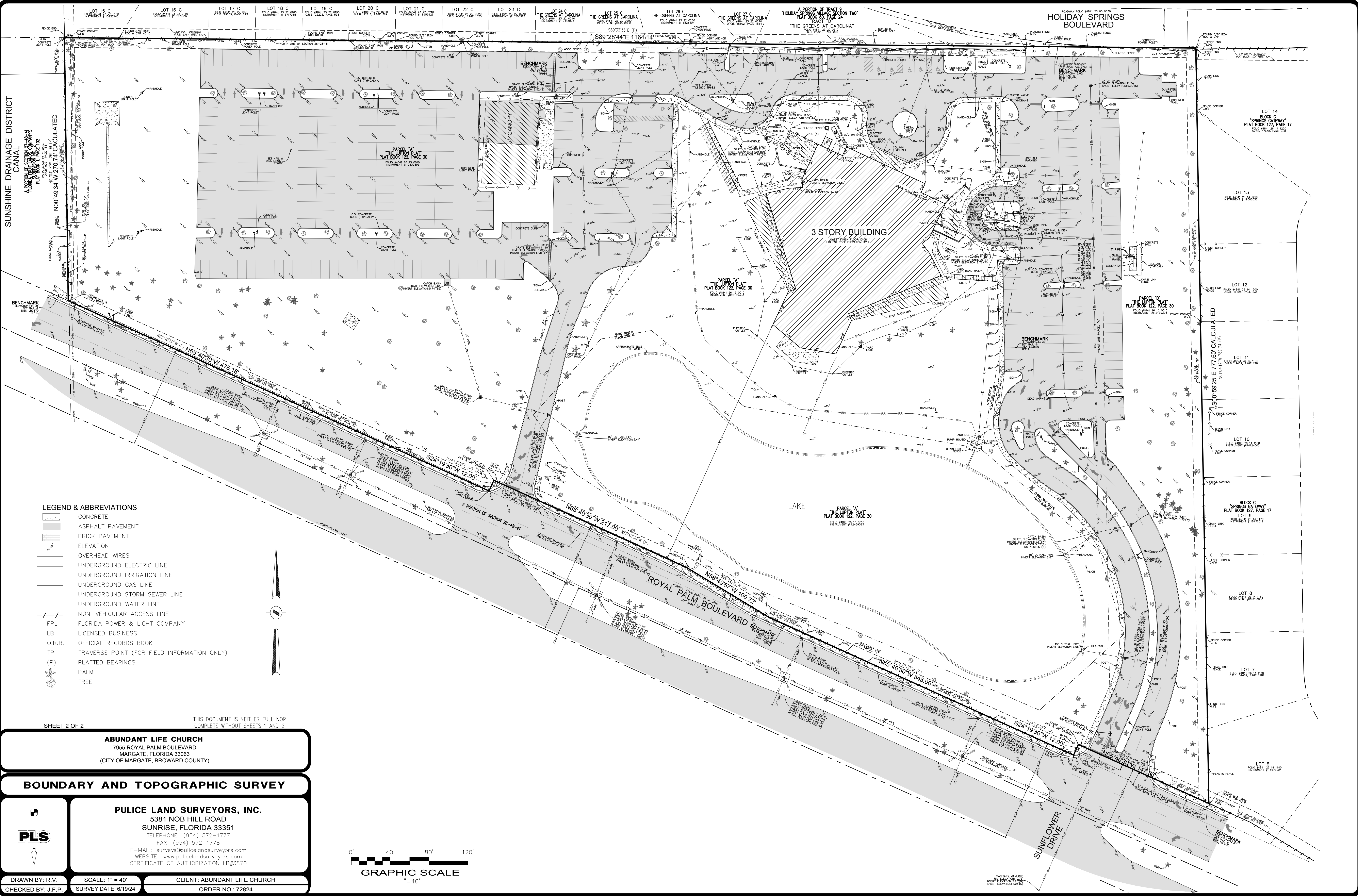
SCALE: 1" = 40'

CLIENT: ABUNDANT LIFE CHURCH

CHECKED BY: J.F.P.

SURVEY DATE: 6/19/24

ORDER NO.: 72824



LEGEND & ABBREVIATIONS

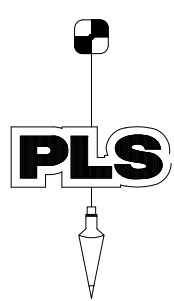
- CONCRETE
- ASPHALT PAVEMENT
- BRICK PAVEMENT
- ELEVATION
- OVERHEAD WIRES
- UNDERGROUND ELECTRIC LINE
- UNDERGROUND IRRIGATION LINE
- UNDERGROUND GAS LINE
- UNDERGROUND STORM SEWER LINE
- UNDERGROUND WATER LINE
- NON-VEHICULAR ACCESS LINE
- FPL FLORIDA POWER & LIGHT COMPANY
- LB LICENSED BUSINESS
- O.R.B. OFFICIAL RECORDS BOOK
- TP TRAVERSE POINT (FOR FIELD INFORMATION ONLY)
- (P) PLATTED BEARINGS
- PALM
- TREE

SHEET 2 OF 2

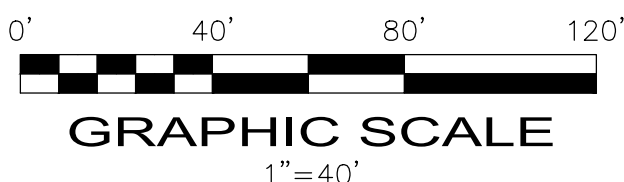
THIS DOCUMENT IS NEITHER FULL NOR COMPLETE WITHOUT SHEETS 1 AND 2

ABUNDANT LIFE CHURCH
7955 ROYAL PALM BOULEVARD
MARGATE, FLORIDA 33063
(CITY OF MARGATE, BROWARD COUNTY)

BOUNDARY AND TOPOGRAPHIC SURVEY



PULICE LAND SURVEYORS, INC.
5381 NOB HILL ROAD
SUNRISE, FLORIDA 33351
TELEPHONE: (954) 572-1777
FAX: (954) 572-1778
E-MAIL: surveys@puliceandsurveyors.com
WEBSITE: www.puliceandsurveyors.com
CERTIFICATE OF AUTHORIZATION LB#3870



DRAWN BY: R.V. SCALE: 1" = 40' CLIENT: ABUNDANT LIFE CHURCH
CHECKED BY: J.F.P. SURVEY DATE: 6/19/24 ORDER NO.: 72824

Appendix D – Proof of Ownership

Appendix E – ERC Summary



Rodrigo Torres, P.E.
Kimley-Horn
8201 Peters Rd, Suite 2200
Plantation, FL 33324
D: 954.519.2432

Matt Garde
Olsson, Inc.
11600 Broadway Extension, Suite 300
Oklahoma City, OK 73114
O: 405.242.6600

Rodrigo:

Please see information below for our expected water consumption regarding this project.

References

Per Margate, FL – Code of Ordinances, 1 ERC = 335 GPD.

https://library.municode.com/fl/margate/codes/code_of_ordinances?nodeId=PTIICOOR_CH39WASEEPENENSE_AR_TVIIUTRURERA_S39-73COCH

Design Water Supply Fixture Units: 221 WSFU (Refer to Appendix A for additional information)

Design Peak Flowrate: 80 GPM

Restroom Daily Usage and Durations:

The occupancy fluctuations of a church make the daily usage of restrooms vary based on when there are church services being provided. For a typical week, the highest usage will be on Sunday morning's when there will be a large amount of people coming and going to services. The other days of the week will be minimal use due to a decreased capacity and act more like an office schedule rather than a public assembly space. For these calculations, it's assumed that there are 20 minutes between services where occupants will fill the restrooms. Assuming there are 6 services, that equates to 120 minutes (2 hr). The other days of the week are assumed to have about 30 minutes of usage total per day (i.e. there most likely will not be 20 consecutive minutes of people flushing toilets, washing hands, etc.; the usage will be more intermittent). The other days of the week also won't see the same peak flowrate due to decreased occupancy count and simultaneous usage. For the purpose of this calculation, a diversity factor of 25% has been applied for Monday – Saturday. Taking the product of the duration and flowrate results in the design usage per day.

<i>Day</i>	<i>Usage Duration</i>	<i>Usage Flowrate</i>	<i>Usage per day</i>	<i>Equivalent Residential Connection (ERC)</i>
Sunday	120 min (2 hr)	80 GPM	9600 GPD	29
Monday	30 min (0.5 hr)	20 GPM	600 GPD	2
Tuesday	30 min (0.5 hr)	20 GPM	600 GPD	2
Wednesday	30 min (0.5 hr)	20 GPM	600 GPD	2
Thursday	30 min (0.5 hr)	20 GPM	600 GPD	2
Friday	30 min (0.5 hr)	20 GPM	600 GPD	2
Saturday	30 min (0.5 hr)	20 GPM	600 GPD	2

Table 1: Daily ERC Summary

With the design usage per day, we can then determine the ERC count by taking the quotient of the usage per day divided by 335 (from the city code). It's important to note that in these calculations the usage duration is not based on how much time occupants spend in the facilities but instead how much time the water is actually running. As an example, a person might spend 10 minutes in the restroom but the water might only run for 1 minute (while the watercloset is flushing / refilling & when the lavatory faucet is running). These calculations also exclude any equipment / fixtures outside the building such as yard hydrants, fountains, irrigation, etc.

Using these numbers, a weekly water consumption can also be determined by summing all of the Daily ERC values together. This leads to a weekly usage of 41 ERC.

APPENDIX A

EXISTING WSFU								
<u>Fixture Type</u>	<u>Qty.</u>	<u>Hot</u>	<u>Cold</u>	<u>Total</u>	<u>Sub-Total Hot</u>	<u>Sub-Total Cold</u>	<u>Sub-Total</u>	<u>Notes</u>
Water Closet (Tank)	35	0	2	2	0	70	70	WSFU updated to current code requirements
Urinal	9	0	5	5	0	45	45	WSFU updated to current code requirements
Lavatory	24	1.5	1.5	2	36	36	48	WSFU updated to current code requirements
Mop Sink	1	2.25	2.25	3	2.25	2.25	3	WSFU updated to current code requirements
Electric Water Cooler	4	0	0.25	0.25	0	1	1	WSFU updated to current code requirements
Sub-Total	73	3.75	11	12.25	38.25	154.25	167	
<u>Fixture Type (Fixed Flowrates)</u>	<u>Qty.</u>	<u>GPM</u>						<u>Notes</u>
3 Comp Sink	1	10						Estimate
WSFU Count	167	FU						
WSFU Flowrate	60	GPM						
Fixed Flowrate	10	GPM						
Total Flowrate	70	GPM						

APPENDIX A

RENOVATION & ADDITION WSFU								
<u>Demolition</u>	<u>Qty</u>							
Water Closet (Tank)	2							
Urinal	2							
Lavatory	3							
Mop Sink	1							
Electric Water Cooler	1							
<u>Added</u>	<u>Qty</u>							
Water Closet (Tank)	10							
Urinal	4							
Lavatory	7							
Mop Sink	2							
Electric Water Cooler	1							
Sinks (Bar/Prep)	9							
Sinks (Kitchen)	1							
<u>Fixture Type</u>	<u>Qty.</u>	<u>Hot</u>	<u>Cold</u>	<u>Total</u>	<u>Sub-Total Hot</u>	<u>Sub-Total Cold</u>	<u>Sub-Total</u>	<u>Notes</u>
Water Closet (Tank)	43	0	2	2	0	86	86	
Urinal	11	0	5	5	0	55	55	
Lavatory	28	1.5	1.5	2	42	42	56	
Mop Sink	2	2.25	2.25	3	4.5	4.5	6	
Electric Water Cooler	4	0	0.25	0.25	0	1	1	
Sinks (Bar/Prep)	9	1	1	1.4	9	9	12.6	
Sinks (Kitchen)	1	3	3	4	3	3	4	
Sub-Total	98	7.75	15	17.65	58.5	200.5	220.6	
<u>Fixture Type (Fixed Flowrates)</u>	<u>Qty.</u>	<u>GPM</u>						<u>Notes</u>
3 Comp Sink	1	10						Estimate
WSFU Count	220.6	FU						
WSFU Flowrate	70	GPM						
Fixed Flowrate	10	GPM						
Total Flowrate	80	GPM						

Appendix F – Drainage Report

Drainage Report

Abundant Life Church

Margate, Florida

March 2025

Prepared For:

Abundant Life Church
7955 Royal Palm Blvd,
Margate, Florida 33063

Prepared By:

Kimley-Horn and Associates, Inc.
8201 Peters Road, Suite 2200
Plantation, Florida 33328

Kimley»Horn



**Drainage Report
For
Abundant Life Church**

KHA Project Number: 143788000

March 2025

Prepared For:
Abundant Life Church
7955 Royal Palm Blvd
Margate, Florida 33063

Aaron Mackey, P.E.
FL P.E. Lic. No. 93208
Kimley-Horn & Associates, Inc.
8201 Peters Road, Suite 2200
Plantation, Florida 33324

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- C-2 ICPR Model Input
- C-3 ICPR Model Results

Objective

This report was prepared to summarize drainage calculations for proposed improvements to the Abundant Life Church as part of the analysis for permit and Surface Water Management License modifications.

Project Description

The subject site is located within Folio #4841 26 13 0010 in Township 48S, Range 41E, Section 26 in the City of Margate, Florida. The subject property has an existing Master Environmental Resource Permit (ERP) under permit No.: 06-02252-P, issued in 1996. The proposed development will be submitted as a permit modification to the existing permit.

In the existing conditions, the property is drained through a connected stormwater network surrounding the existing building and ultimately discharging to the retention pond on the southern boundary. The proposed drainage modification is for the building expansion on the existing Abundant Lift Church as well as a driveway addition. The project proposes new stormwater pipes, catch basins, manholes, and exfiltration trench. This building expansion replaces grass, concrete sidewalks, and asphalt pavement areas. The proposed improvements will include the following impervious and pervious modifications.

Table 1: Basin Summary

Overall Site	Pervious (ac)	Impervious (ac)	Pond (ac)	Building (ac)	Total (ac)
Existing	6.71	4.95	1.85	0.57	14.08
Proposed	6.36	4.76	1.85	1.11	14.08
Net Difference	(-) 0.35	(-) 0.19	-	(+) 0.54	-

See attached pre-development and post-development exhibits. Additionally, 801 linear feet of exfiltration trench have been proposed to provide additional water quality treatment and water quantity storage to the existing stormwater system.

Drainage Criteria

Water Quantity

A minimum finished floor elevation for future development of all buildings within the site limits is to be established to provide flood protection for all buildings from flooding generated by a 100-year, 72-hour storm event. For sites not located within a FEMA Special Flood Hazard Area (SFHA) (the retention pond is the only portion of the site within Zone AE) such as this site, the finished floor elevation will be determined by the Design Flood Elevation (DFE). The DFE is determined by the maximum stage calculated specifically for this site for a 100-year, 72-hour storm event. The DFE for this project is reported in the findings section of this memorandum.

Water Quality

Broward County and South Florida Water Management District (SFWMD) stormwater permit criteria and procedures were followed during this analysis. The total volume for water quality is provided for the greater of the first inch of storm runoff from the entire site, or the amount of 2.5 inches times the percentage of impervious area. For this project, the 2.5 inches times the area impervious will be used. Volumetric calculations for the required and provided amounts can be found in Exhibit B-1.

Hydraulic and Hydrologic Analysis

This 14.08-acre site was divided into four basins (see Appendix A-3 for the Basin Map) to compare existing and proposed conditions peak stages. Interconnected Channel Pond Routing (ICPR), version 4.07.04, software was used to model the rainfall events discussed in the previous section.

Existing conditions topography was obtained from 2018 USGS Southeast Florida Lidar. Proposed conditions topography incorporates the design finished grade surface for areas to be redeveloped.

Rainfall data was gathered from NOAA's National Weather Service Hydrometeorological Design Studies Center.

Additional assumptions used for the drainage calculations and model inputs:

Pervious Curve Number	= 84
Impervious Curve Number	= 98
5-year, 24-hour Rainfall Depth	= 7.08 in.
25-year, 72-hour Rainfall Depth	= 13.00 in.
100-year, 72-hour Rainfall Depth	= 18.00 in.

Manual basins in ICPR are used to implement traditional hydrologic methods such as the NRCS Unit Hydrograph Method with a distributed hydrologic approach to determine the rainfall excess. The impervious and pervious areas within each basin in existing and proposed conditions were designated with the curve numbers above for runoff calculations.

Water Quantity

A stage-area relationship was developed for both the existing and proposed conditions for the basins using lidar and finished grade surfaces and ICPR (see model inputs in Exhibit C-1 and results in Exhibit C-2).

Exfiltration trench was modeled as a french drain link. The hydraulic conductivity used was interpolated using results from the geotechnical report and using a safety factor of 2.

Water Quality

Water quality volume is provided on the site through an existing retention pond. Additionally, the project proposes an additional 801 LF of exfiltration trench, which provide more water quality. See required water quality calculations in Exhibit B-1.

The required water quality is 1.17 ac-ft, as stated in the previous permit (permit No.: 06-02252-P) from 1996. It is assumed that, in order to comply with this requirement, the 1.17 ac-ft of water quality was incorporated into the site; therefore, no additional water quality compensatory storage is required.

Results Summary

Broward County/FEMA Flood Zone (Base Flood Elevation)

The FEMA flood hazard designation, as adopted by Broward County, for Basins 1, 2, and 4 of this site is Flood Zone X - Unshaded. Basin 3 is designated as Flood Zone AE with a Base Flood Elevation (BFE) of 12 ft-NAVD.

100-Year, 72-Hour Storm Stage (Design Flood Elevation)

The following is a summary of the peak stages calculated by a storm routing model prepared in ICPR (see Exhibit B-5):

Table 2: 100-Year 72-Hour Storm Stage Results

Storm Event	Existing Peak Stage	Proposed Peak Stage
100-year, 72-hour	12.36	12.34

Finished Floor Elevation

Building finish floor elevations are at 13.08 NAVD, which is above the design flood elevation obtained from the 100-year, 72-hour storm event.

Table 3: Design Flood Elevation

Design Flood Elevation (DFE)	12.34 NAVD
Proposed Building Finished Floor Elevation	13.08 NAVD

Peak Stage Summary

Furthermore, the peak stages for the 5yr-24hr, 25yr-72hr, and 100yr-72hr storm events generally decrease from the existing to the proposed conditions scenario, as shown in the table below.

Table 4: Results

Scenario	Basin	Existing Peak Stage	Proposed Peak Stage
5-year, 24-hour	Basin 1	11.41	11.28
5-year, 24-hour	Basin 2	11.81	11.52
5-year, 24-hour	Basin 3	7.93	7.72
5-year, 24-hour	Basin 4	11.90	11.91
25-year, 72-hour	Basin 1	11.95	11.85
25-year, 72-hour	Basin 2	11.95	11.93
25-year, 72-hour	Basin 3	10.73	10.33
25-year, 72-hour	Basin 4	12.03	11.93
100-year, 72-hour	Basin 1	12.36	12.34
100-year, 72-hour	Basin 2	12.36	12.34
100-year, 72-hour	Basin 3	12.16	11.89
100-year, 72-hour	Basin 4	12.35	11.97

Appendix A - Exhibits

Drawing name: K:\FTL_Civil\143_Jobs\143788000_Abundant Life Church Margate\Design\Drainage References\Appendices\Location Maps\Location and Aerial Maps.dwg AERIAL Jul 15, 2024 9:29am by: Gabi Gallegos

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PHONE: 954-535-5100 FAX: 954-739-2247
WWW.KIMLEY-HORN.COM REGISTRY No. 696

DATE 7/15/24
PROJECT NO.

**AERIAL MAP
ABUNDANT LIFE
CHURCH**

DESIGN ENGINEER:
FLORIDA P.E. LICENSE NUMBER:
DATE:

SHEET NUMBER A-1

Drawing name: K:\FTL_Civil\143_Jobs\143788000_Abundant Life Church Margate\Design\Drainage References\Appendices\Location Maps\Location and Aerial Maps.dwg LOCATION Jul 15, 2024 9:29am by: Gabi Gallegos

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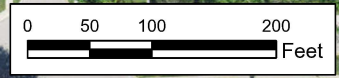
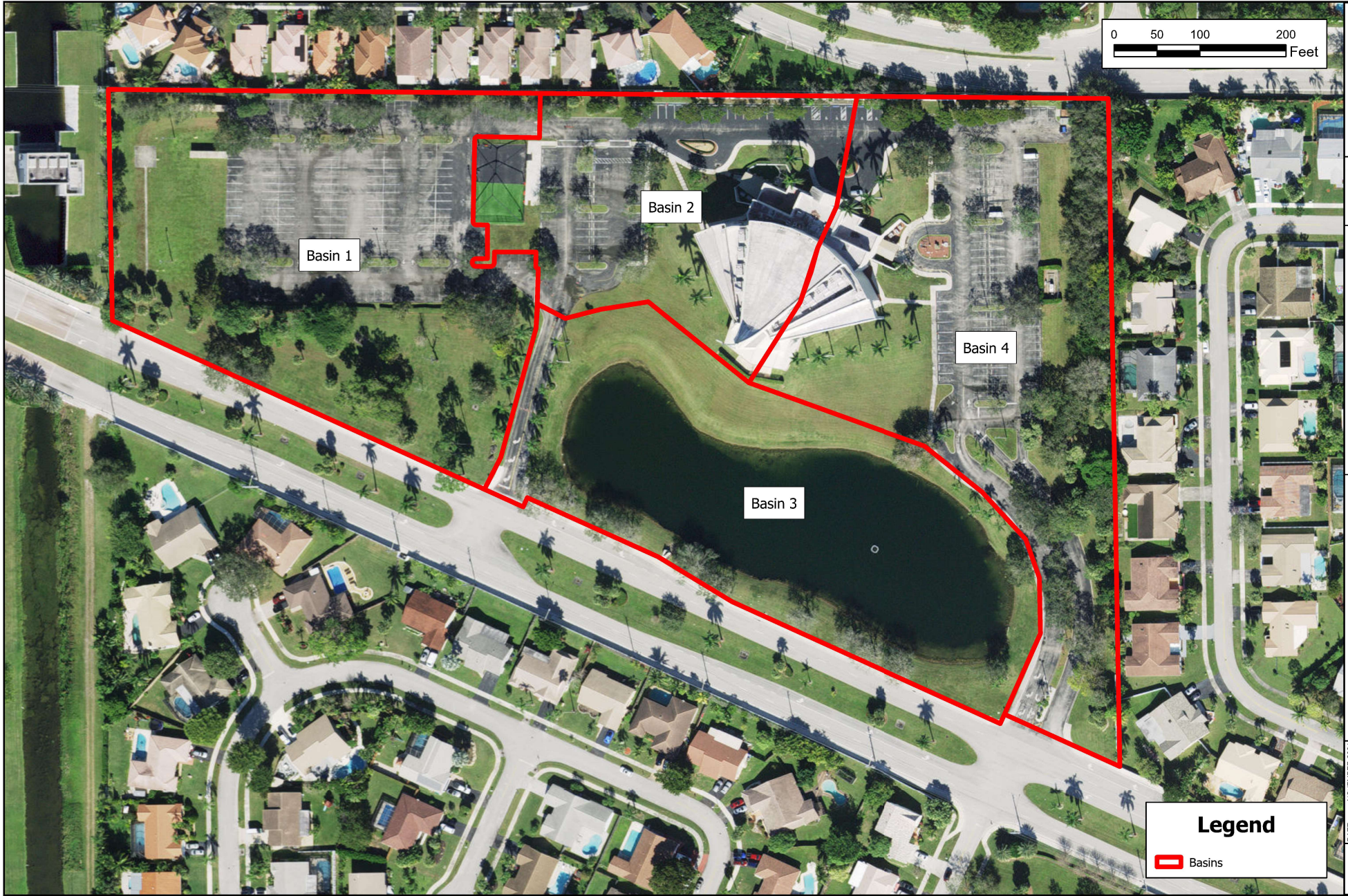
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DATE
7/15/24
PROJECT NO.

LOCATION MAP
ABUNDANT LIFE
CHURCH

DESIGN ENGINEER:
FLORIDA P.E. LICENSE NUMBER:
DATE:

SHEET NUMBER
A-2



Kimley-Horn
8201 Peters Road
Suite 2200, Ft. 33324
Florida
(954) 535-5100 Phone



CITY OF MARGATE
ABUNDANT LIFE CHURCH

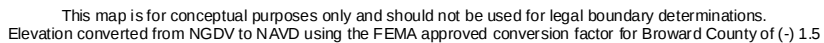
BASINS MAP

DATE:	NOVEMBER 2024
DESIGN:	
DRAWN:	
CHECKED:	
KH NO.:	143788002

SHEET
A-3

Legend

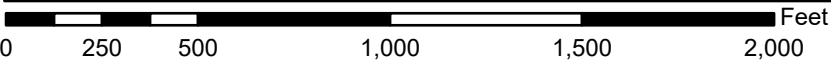
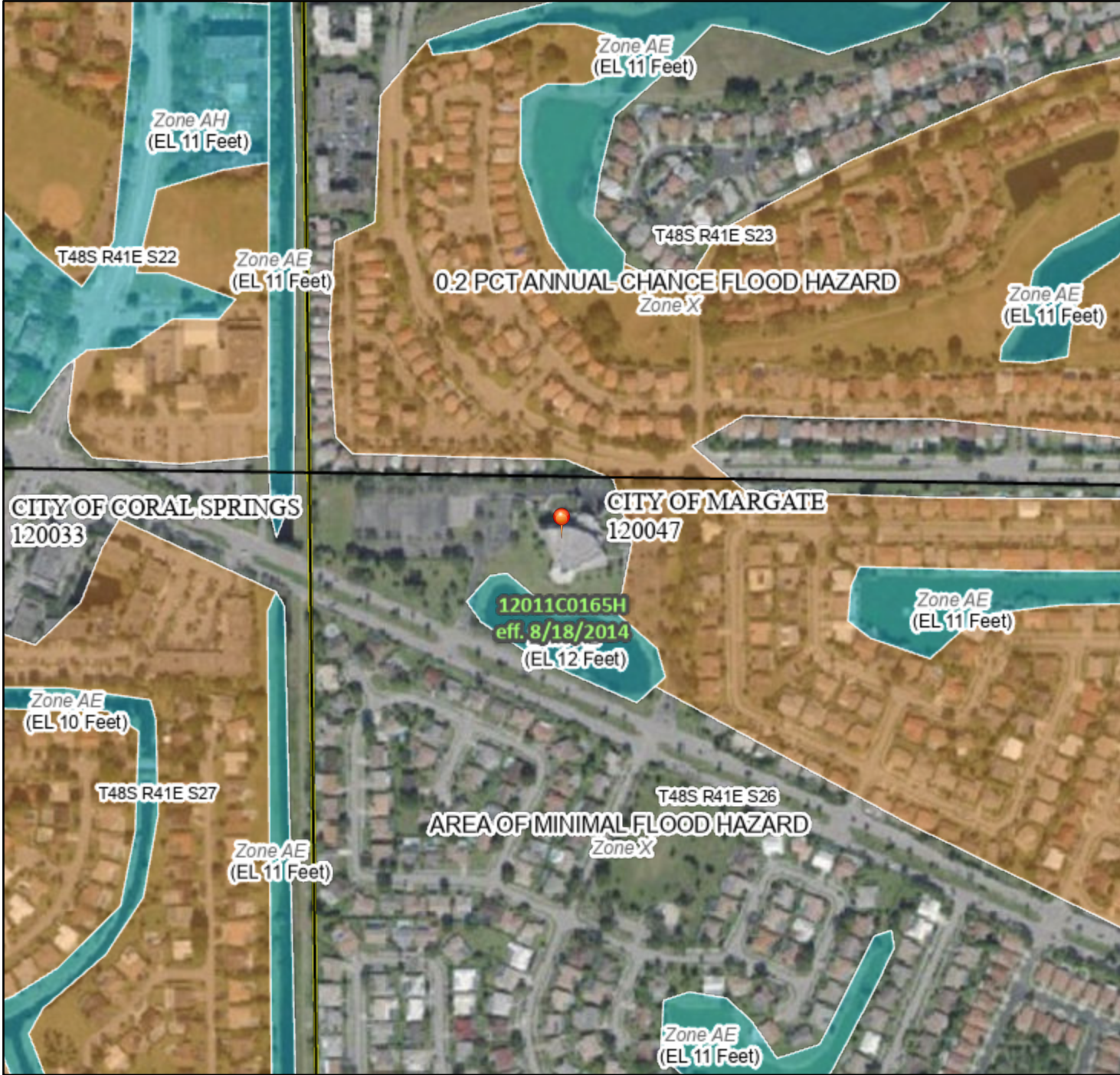
 Basins



National Flood Hazard Layer FIRMette



80°14'12"W 26°15'42"N



1:6,000

80°13'35"W 26°15'10"N

Basemap Imagery Source: USGS National Map 2023

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) Zone A, V, A99
		With BFE or Depth Zone AE, AO, AH, VE, AR
		Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee. See Notes. Zone X
		Area with Flood Risk due to Levee Zone D
OTHER AREAS		NO SCREEN Area of Minimal Flood Hazard Zone X
		Effective LOMRs
GENERAL STRUCTURES		Area of Undetermined Flood Hazard Zone D
		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
OTHER FEATURES		20.2 Cross Sections with 1% Annual Chance Water Surface Elevation
		17.5
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
MAP PANELS		Coastal Transect Baseline
		Profile Baseline
		Hydrographic Feature
		Digital Data Available
		No Digital Data Available
		Unmapped

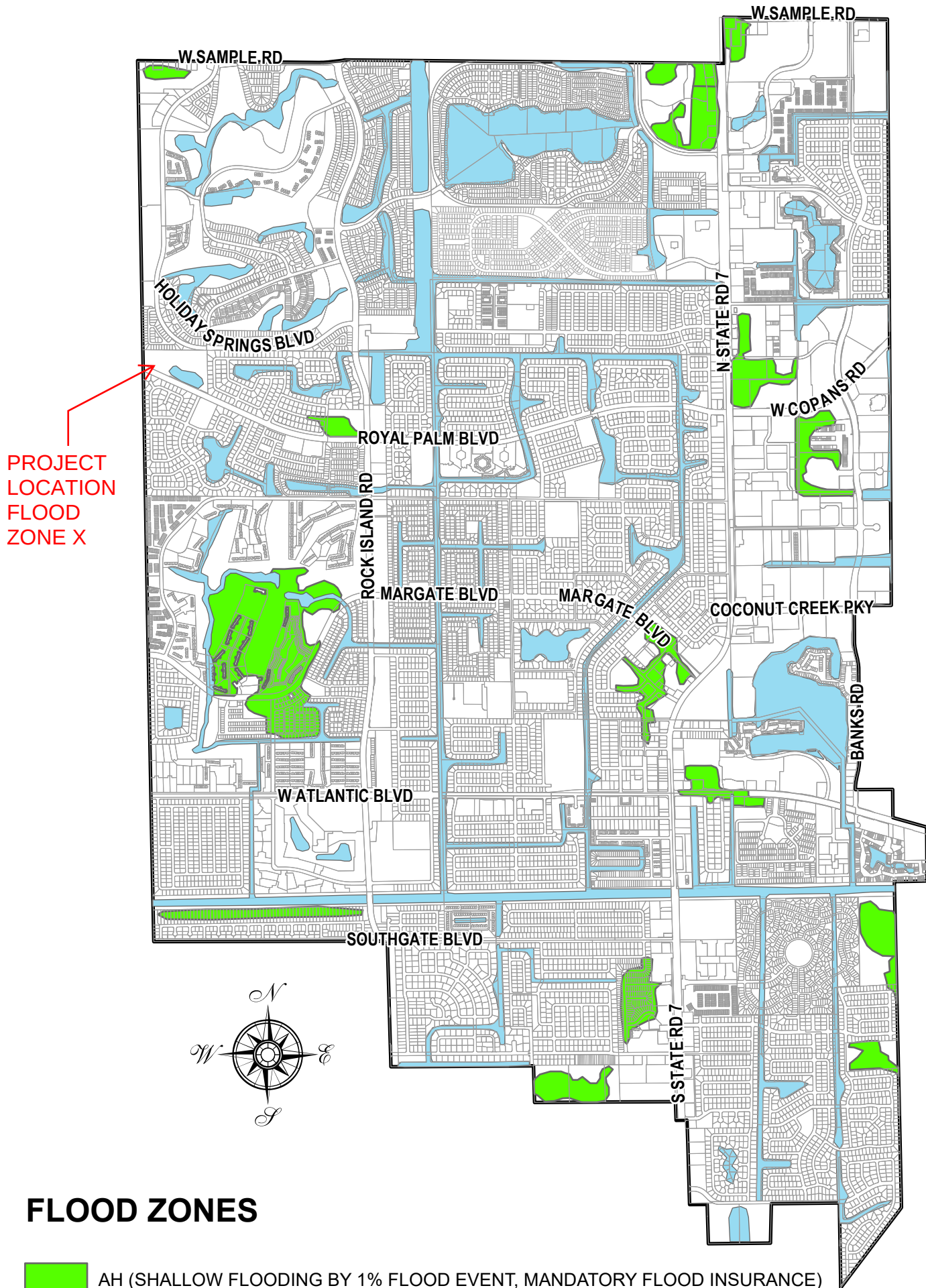


The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **11/20/2024 at 1:36 PM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

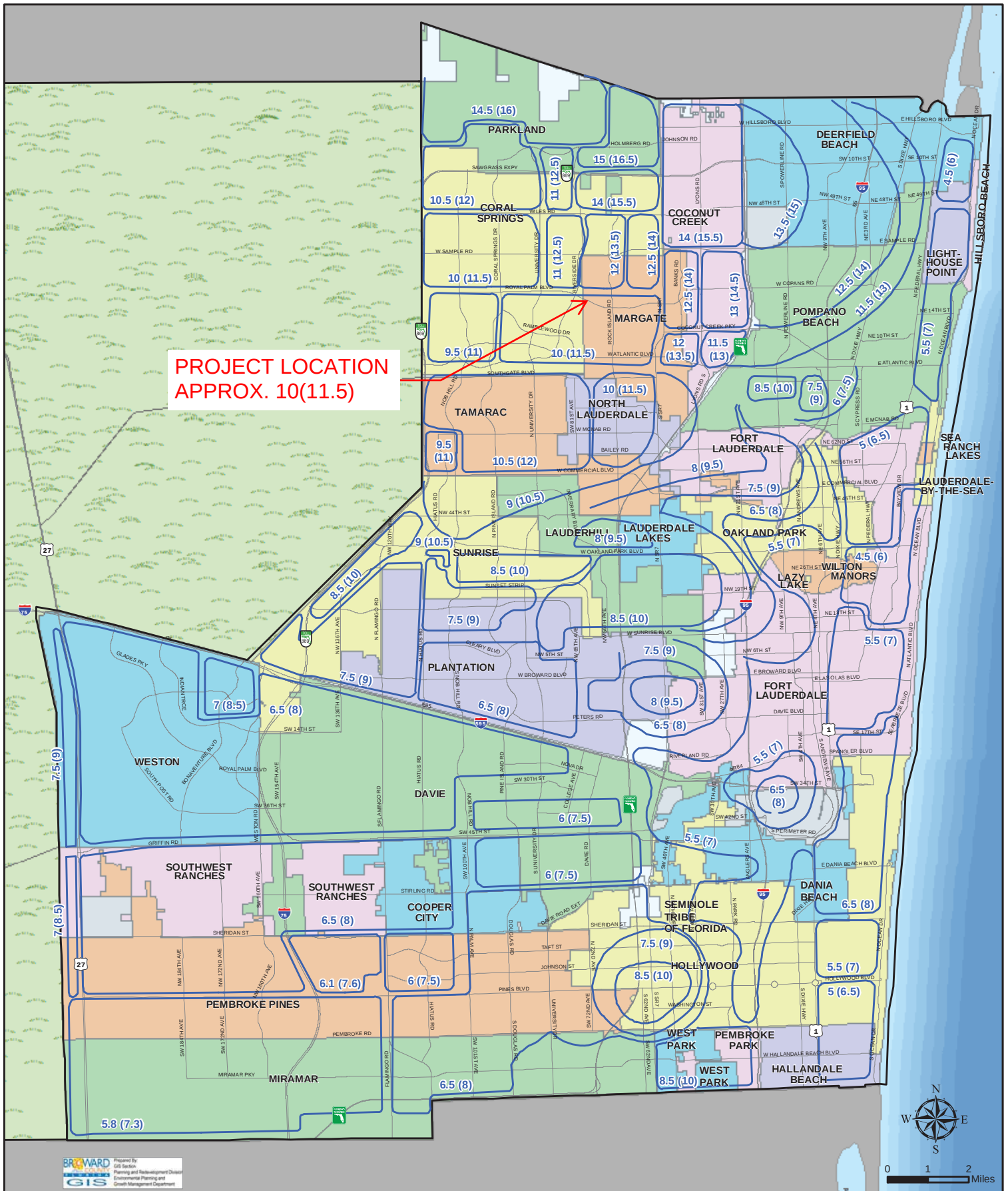
This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.



PROJECT
LOCATION
FLOOD
ZONE X

FLOOD ZONES

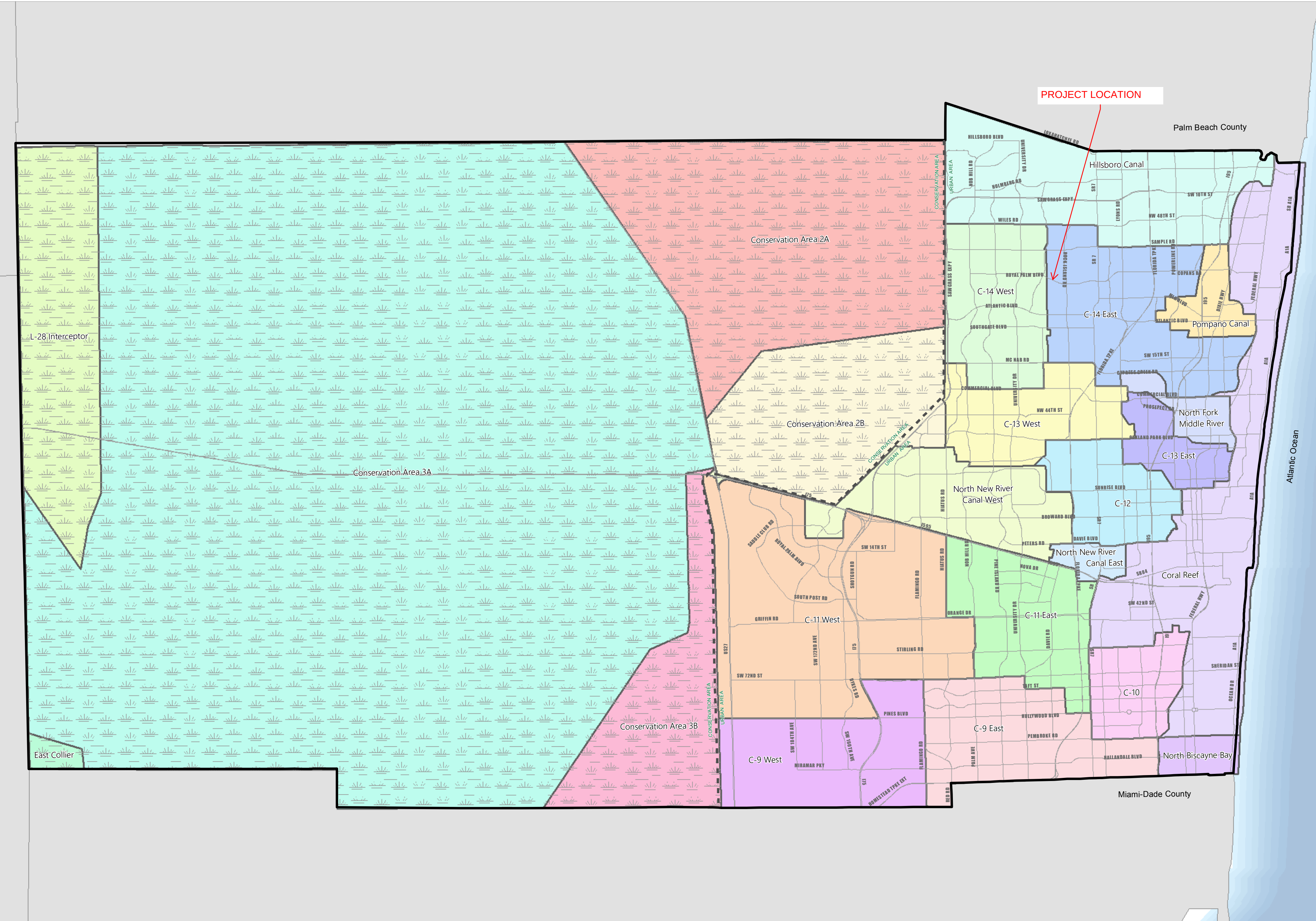
-  AH (SHALLOW FLOODING BY 1% FLOOD EVENT, MANDATORY FLOOD INSURANCE)
-  AE (INUNDATION BY 1% FLOOD EVENT, MANDATORY FLOOD INSURANCE)
-  X (MANDATORY FLOOD INSURANCE PURCHASE REQUIREMENTS DO NOT APPLY)



This map is for conceptual purposes only and should not be used for legal boundary determinations.

Elevations converted from NGVD to NAVD using the FEMA approved conversion factor for Broward County of (-)1.5, based on 1997 FEMA Flood Data

DRAINAGE BASINS



Legend

- County Boundary
- Urban Area Boundary
- Major Roads
- Drainage Basins (with basin names)
- Water Conservation Areas (with names)

Source: South Florida Water Management District
Environmental Protection and Growth Management Department
Environmental Planning and Community Resilience
Broward GIS, 2016

This map is for conceptual purposes only and is not intended for legal boundary determinations.

Environmental Protection & Growth Management Department
Planning and Development Management Division
Prepared by: GIS Section

0 2.5 5 7.5 Miles



MAP NO. WM-4

#14260 Cneira 3/20/18

Appendix B - Calculations

Overall Site**Exfiltration Trench Inputs**

Hydraulic Conductivity (K)	2.22E-04 cfs/ft ² -ft
Pavement Section Thickness	2.0 ft
Factor of Safety (FS)	2.0

Proposed Exfiltration Trench

Length of Trench for Water Quality (Lwq)	801.0 ft
Additional Length of Trench (Ladd)	0.0 ft
Control Elevation (CE)	5.50 NAVD - ft
Lowest Weir/Bleeder Elevations (EL _{inv})	N/A NAVD - ft
Lowest Rim Elevation	11.44 NAVD - ft
Trench Width (W)	6.00 ft
Depth of Trench (D)	4.00 ft
Top of Trench Elevation	9.44 NAVD - ft
Bottom of Trench Elevation	5.44 NAVD - ft
Distance from EL _{inv} and CE (H ₂)	5.94 ft
Unsaturated Trench Depth (Du)	3.94 ft
Saturated Trench Depth (Ds)	0.06 ft
Formula Used	Regular
Volume Provided for Water Quality* (Vwq)	0.611 ac-ft
Additional Volume Provided (Vadd)	0.000 ac-ft

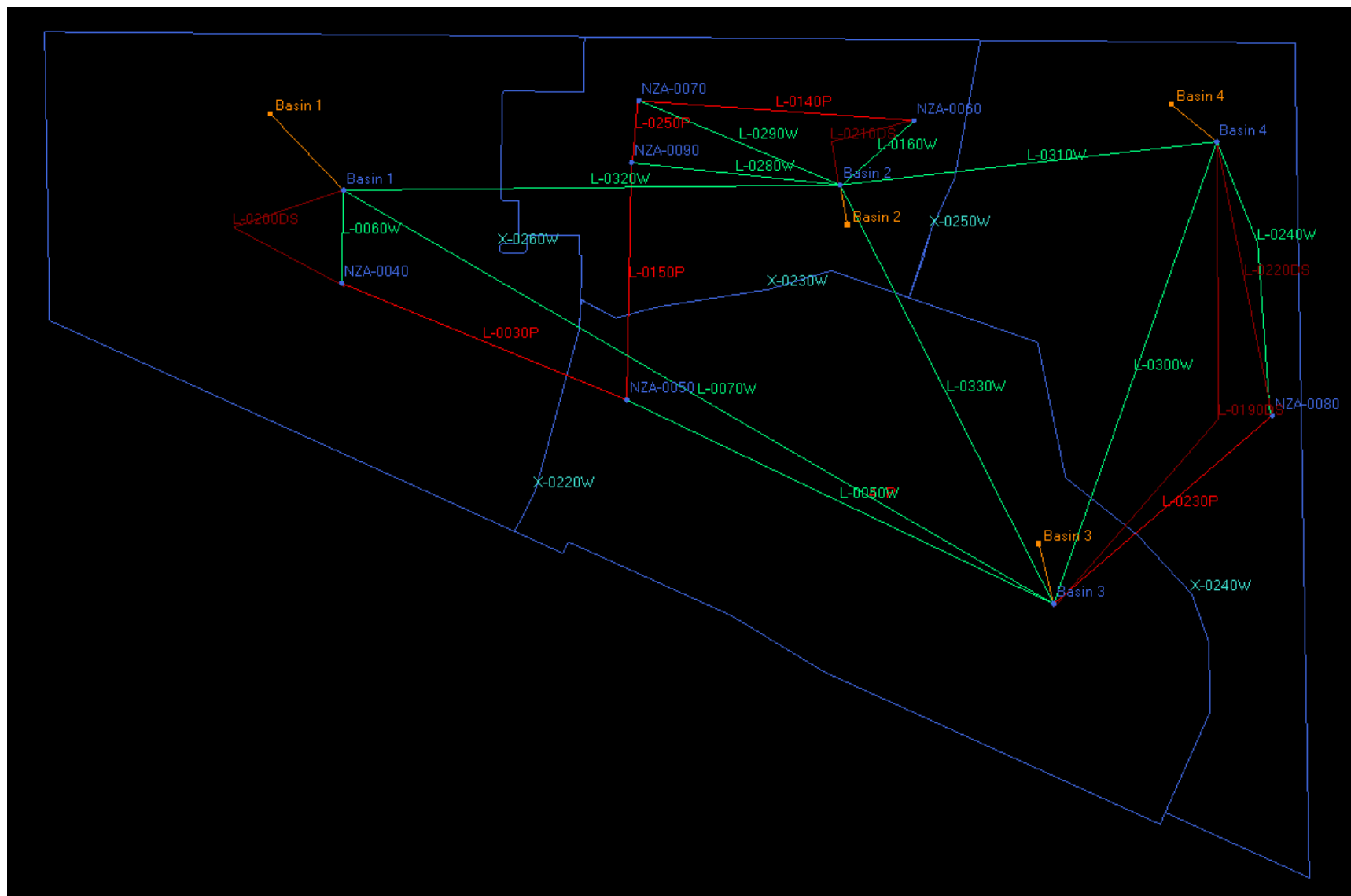
*Does not include credit

	Total Project Area (ac)	Additional Impervious Area (ac)	Total Required Water Quality 2.5" x Impervious Area (ac-ft)	Pre-Treatment 0.5" x Impervious Area (ac-ft)	Additional 50% Water Quality Required for WNAS
	14.08	0.49	0.102	0.020	0.051
TOTAL	14.08	0.49	0.102	0.020	0.051

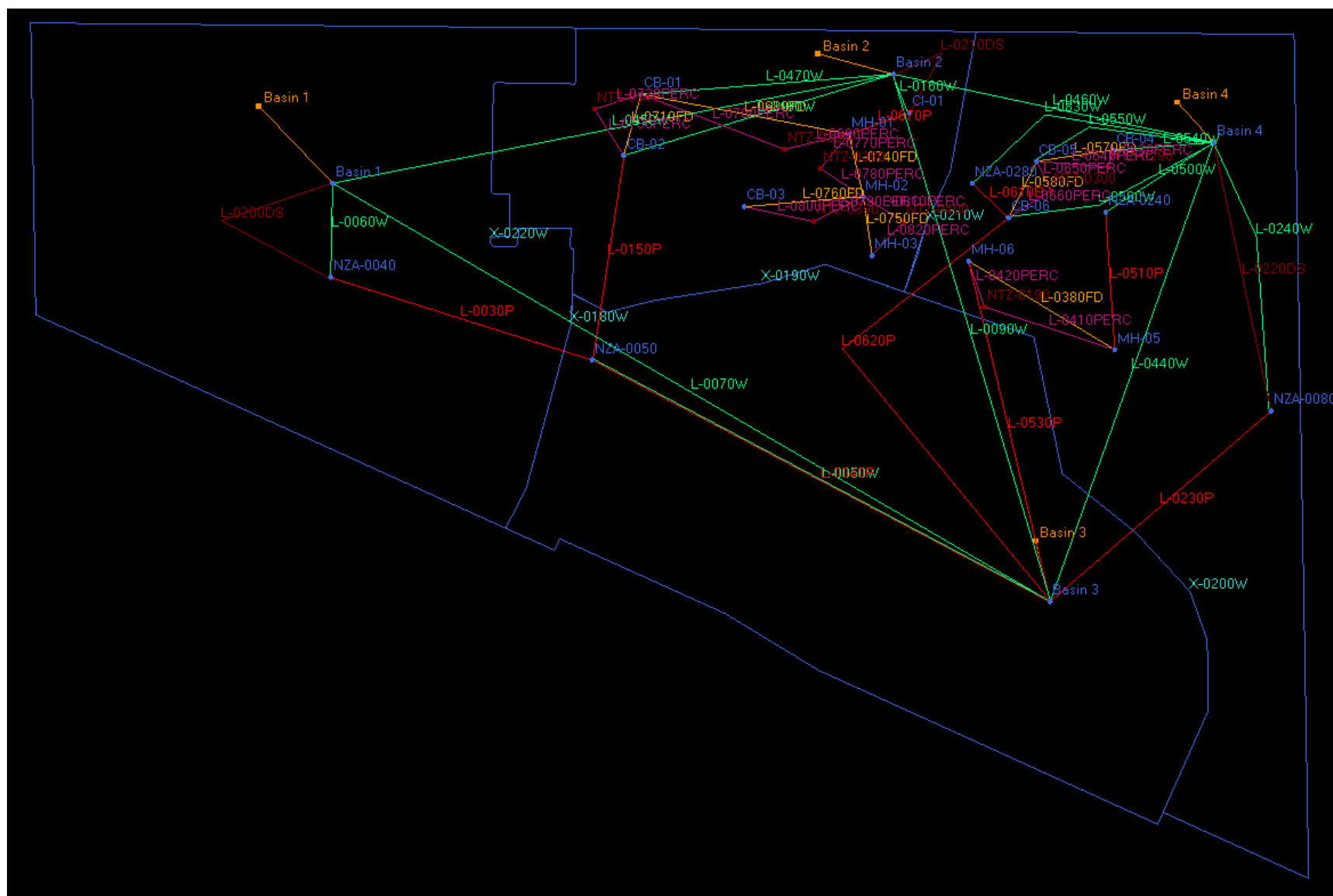
Total Required Water Quality Volume: 0.15 acre-ft

Appendix C – ICPR Modeling

Existing ICPR Model



Proposed ICPR Model



Manual Basin: Basin 1

Scenario: Existing
 Node: Basin 1
 Hydrograph Method: NRCS Unit Hydrograph
 Infiltration Method: Curve Number
 Time of Concentration: 10.0000 min
 Max Allowable Q: 0.00 cfs
 Time Shift: 0.0000 hr
 Unit Hydrograph: UH256
 Peaking Factor: 256.0
 Area: 3.9590 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name	Crop Coefficient Zone	Reference ET Station
2.2781	Pervious	A			
1.6809	Impervious	A			

Comment:

Manual Basin Runoff Summary [Existing]

Basin Name	Sim Name	Max Flow [cfs]	Time to Max Flow [hrs]	Total Rainfall [in]	Total Runoff [in]	Area [ac]	Equivalent Curve Number	% Imperv	% DCIA
Basin 1	100yr-72hr	31.13	60.0167	18.00	16.67	3.9590	89.7	0.00	0.00
Basin 1	25yr-72hr	22.27	60.0167	13.00	11.71	3.9590	89.8	0.00	0.00
Basin 1	5yr-24hr	15.33	12.0500	7.08	5.90	3.9590	90.0	0.00	0.00

Manual Basin Mass Balance Summary [Existing]

Basin Name	Sim Name	Total Rainfall	Total Irrigation	Total Runoff	Total ET	Total Initial Abst	Total Recharge	Change Soil Storage
Basin 1 [in]	100yr-72hr	18.00	0.00	16.67	0.00	0.00	0.00	1.33
Basin 1 [ft3]	100yr-72hr	258681	0	239560	0	0	0	19121
Basin 1 [ac-ft]	100yr-72hr	5.94	0.00	5.50	0.00	0.00	0.00	0.44
Basin 1 [in]	25yr-72hr	13.00	0.00	11.71	0.00	0.00	0.00	1.29
Basin 1 [ft3]	25yr-72hr	186825	0	168321	0	0	0	18505
Basin 1 [ac-ft]	25yr-72hr	4.29	0.00	3.86	0.00	0.00	0.00	0.42
Basin 1 [in]	5yr-24hr	7.08	0.00	5.90	0.00	0.00	0.00	1.18
Basin 1 [ft3]	5yr-24hr	101748	0	84777	0	0	0	16971
Basin 1 [ac-ft]	5yr-24hr	2.34	0.00	1.95	0.00	0.00	0.00	0.39

Manual Basin: Basin 2

Scenario: Existing
 Node: Basin 2
 Hydrograph Method: NRCS Unit Hydrograph
 Infiltration Method: Curve Number
 Time of Concentration: 10.0000 min
 Max Allowable Q: 0.00 cfs
 Time Shift: 0.0000 hr
 Unit Hydrograph: UH256
 Peaking Factor: 256.0
 Area: 2.3425 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name	Crop Coefficient Zone	Reference ET Station
0.7525	Pervious	A			
1.5900	Impervious	A			

Comment:

Manual Basin Runoff Summary [Existing]

Basin Name	Sim Name	Max Flow [cfs]	Time to Max Flow [hrs]	Total Rainfall [in]	Total Runoff [in]	Area [ac]	Equivalent Curve Number	% Imperv	% DCIA
Basin 2	100yr-72hr	18.52	60.0167	18.00	17.14	2.3425	93.3	0.00	0.00
Basin 2	25yr-72hr	13.30	60.0167	13.00	12.17	2.3425	93.3	0.00	0.00
Basin 2	5yr-24hr	9.44	12.0500	7.08	6.31	2.3425	93.6	0.00	0.00

Manual Basin Mass Balance Summary [Existing]

Basin Name	Sim Name	Total Rainfall	Total Irrigation	Total Runoff	Total ET	Total Initial Abst	Total Recharge	Change Soil Storage
Basin 2 [in]	100yr-72hr	18.00	0.00	17.14	0.00	0.00	0.00	0.86
Basin 2 [ft3]	100yr-72hr	153059	0	145755	0	0	0	7304
Basin 2 [ac-ft]	100yr-72hr	3.51	0.00	3.35	0.00	0.00	0.00	0.17
Basin 2 [in]	25yr-72hr	13.00	0.00	12.17	0.00	0.00	0.00	0.83
Basin 2 [ft3]	25yr-72hr	110543	0	103467	0	0	0	7075
Basin 2 [ac-ft]	25yr-72hr	2.54	0.00	2.38	0.00	0.00	0.00	0.16
Basin 2 [in]	5yr-24hr	7.08	0.00	6.31	0.00	0.00	0.00	0.77
Basin 2 [ft3]	5yr-24hr	60203	0	53669	0	0	0	6534
Basin 2 [ac-ft]	5yr-24hr	1.38	0.00	1.23	0.00	0.00	0.00	0.15

Manual Basin: Basin 3

Scenario: Existing
 Node: Basin 3
 Hydrograph Method: NRCS Unit Hydrograph
 Infiltration Method: Curve Number
 Time of Concentration: 10.0000 min
 Max Allowable Q: 0.00 cfs
 Time Shift: 0.0000 hr
 Unit Hydrograph: UH256
 Peaking Factor: 256.0
 Area: 3.6183 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name	Crop Coefficient Zone	Reference ET Station
1.6124	Pervious	A			
2.0059	Impervious	A			

Comment:

Manual Basin Runoff Summary [Existing]

Basin Name	Sim Name	Max Flow [cfs]	Time to Max Flow [hrs]	Total Rainfall [in]	Total Runoff [in]	Area [ac]	Equivalent Curve Number	% Imperv	% DCIA
Basin 3	100yr-72hr	28.53	60.0167	18.00	16.91	3.6183	91.5	0.00	0.00
Basin 3	25yr-72hr	20.45	60.0167	13.00	11.95	3.6183	91.6	0.00	0.00
Basin 3	5yr-24hr	14.30	12.0500	7.08	6.11	3.6183	91.9	0.00	0.00

Manual Basin Mass Balance Summary [Existing]

Basin Name	Sim Name	Total Rainfall	Total Irrigation	Total Runoff	Total ET	Total Initial Abst	Total Recharge	Change Soil Storage
Basin 3 [in]	100yr-72hr	18.00	0.00	16.91	0.00	0.00	0.00	1.09
Basin 3 [ft3]	100yr-72hr	236420	0	222107	0	0	0	14313
Basin 3 [ac-ft]	100yr-72hr	5.43	0.00	5.10	0.00	0.00	0.00	0.33
Basin 3 [in]	25yr-72hr	13.00	0.00	11.95	0.00	0.00	0.00	1.05
Basin 3 [ft3]	25yr-72hr	170748	0	156891	0	0	0	13857
Basin 3 [ac-ft]	25yr-72hr	3.92	0.00	3.60	0.00	0.00	0.00	0.32
Basin 3 [in]	5yr-24hr	7.08	0.00	6.11	0.00	0.00	0.00	0.97
Basin 3 [ft3]	5yr-24hr	92992	0	80248	0	0	0	12744
Basin 3 [ac-ft]	5yr-24hr	2.13	0.00	1.84	0.00	0.00	0.00	0.29

Manual Basin: Basin 4

Scenario: Existing
 Node: Basin 4
 Hydrograph Method: NRCS Unit Hydrograph
 Infiltration Method: Curve Number
 Time of Concentration: 10.0000 min
 Max Allowable Q: 0.00 cfs
 Time Shift: 0.0000 hr
 Unit Hydrograph: UH256
 Peaking Factor: 256.0
 Area: 4.1265 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name	Crop Coefficient Zone	Reference ET Station
2.0372	Pervious	A			
2.0893	Impervious	A			

Comment:

Manual Basin Runoff Summary [Existing]

Basin Name	Sim Name	Max Flow [cfs]	Time to Max Flow [hrs]	Total Rainfall [in]	Total Runoff [in]	Area [ac]	Equivalent Curve Number	% Imperv	% DCIA
Basin 4	100yr-72hr	32.50	60.0167	18.00	16.82	4.1265	90.8	0.00	0.00
Basin 4	25yr-72hr	23.28	60.0167	13.00	11.86	4.1265	90.9	0.00	0.00
Basin 4	5yr-24hr	16.18	12.0500	7.08	6.03	4.1265	91.2	0.00	0.00

Manual Basin Mass Balance Summary [Existing]

Basin Name	Sim Name	Total Rainfall	Total Irrigation	Total Runoff	Total ET	Total Initial Abst	Total Recharge	Change Soil Storage
Basin 4 [in]	100yr-72hr	18.00	0.00	16.82	0.00	0.00	0.00	1.18
Basin 4 [ft3]	100yr-72hr	269626	0	251967	0	0	0	17658
Basin 4 [ac-ft]	100yr-72hr	6.19	0.00	5.78	0.00	0.00	0.00	0.41
Basin 4 [in]	25yr-72hr	13.00	0.00	11.86	0.00	0.00	0.00	1.14
Basin 4 [ft3]	25yr-72hr	194730	0	177636	0	0	0	17093
Basin 4 [ac-ft]	25yr-72hr	4.47	0.00	4.08	0.00	0.00	0.00	0.39
Basin 4 [in]	5yr-24hr	7.08	0.00	6.03	0.00	0.00	0.00	1.05
Basin 4 [ft3]	5yr-24hr	106053	0	90351	0	0	0	15702
Basin 4 [ac-ft]	5yr-24hr	2.43	0.00	2.07	0.00	0.00	0.00	0.36

Manual Basin: Basin 1

Scenario: Proposed
 Node: Basin 1
 Hydrograph Method: NRCS Unit Hydrograph
 Infiltration Method: Curve Number
 Time of Concentration: 10.0000 min
 Max Allowable Q: 0.00 cfs
 Time Shift: 0.0000 hr
 Unit Hydrograph: UH256
 Peaking Factor: 256.0
 Area: 3.9590 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name	Crop Coefficient Zone	Reference ET Station
2.2781	Pervious	A			
1.6809	Impervious	A			

Comment:

Manual Basin Runoff Summary [Proposed]

Basin Name	Sim Name	Max Flow [cfs]	Time to Max Flow [hrs]	Total Rainfall [in]	Total Runoff [in]	Area [ac]	Equivalent Curve Number	% Imperv	% DCIA
Basin 1	100yr-72hr	31.13	60.0167	18.00	16.67	3.9590	89.7	0.00	0.00
Basin 1	25yr-72hr	22.27	60.0167	13.00	11.71	3.9590	89.8	0.00	0.00
Basin 1	5yr-24hr	15.33	12.0500	7.08	5.90	3.9590	90.0	0.00	0.00

Manual Basin Mass Balance Summary [Proposed]

Basin Name	Sim Name	Total Rainfall	Total Irrigation	Total Runoff	Total ET	Total Initial Abst	Total Recharge	Change Soil Storage
Basin 1 [in]	100yr-72hr	18.00	0.00	16.67	0.00	0.00	0.00	1.33
Basin 1 [ft3]	100yr-72hr	258681	0	239560	0	0	0	19121
Basin 1 [ac-ft]	100yr-72hr	5.94	0.00	5.50	0.00	0.00	0.00	0.44
Basin 1 [in]	25yr-72hr	13.00	0.00	11.71	0.00	0.00	0.00	1.29
Basin 1 [ft3]	25yr-72hr	186825	0	168321	0	0	0	18505
Basin 1 [ac-ft]	25yr-72hr	4.29	0.00	3.86	0.00	0.00	0.00	0.42
Basin 1 [in]	5yr-24hr	7.08	0.00	5.90	0.00	0.00	0.00	1.18
Basin 1 [ft3]	5yr-24hr	101748	0	84777	0	0	0	16971
Basin 1 [ac-ft]	5yr-24hr	2.34	0.00	1.95	0.00	0.00	0.00	0.39

Manual Basin: Basin 2

Scenario: Proposed
 Node: Basin 2
 Hydrograph Method: NRCS Unit Hydrograph
 Infiltration Method: Curve Number
 Time of Concentration: 10.0000 min
 Max Allowable Q: 0.00 cfs
 Time Shift: 0.0000 hr
 Unit Hydrograph: UH256
 Peaking Factor: 256.0
 Area: 2.3427 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name	Crop Coefficient Zone	Reference ET Station
0.3699	Pervious	A			
1.9728	Impervious	A			

Comment:

Manual Basin Runoff Summary [Proposed]

Basin Name	Sim Name	Max Flow [cfs]	Time to Max Flow [hrs]	Total Rainfall [in]	Total Runoff [in]	Area [ac]	Equivalent Curve Number	% Imperv	% DCIA
Basin 2	100yr-72hr	18.58	60.0167	18.00	17.44	2.3427	95.6	0.00	0.00
Basin 2	25yr-72hr	13.38	60.0167	13.00	12.46	2.3427	95.7	0.00	0.00
Basin 2	5yr-24hr	9.67	12.0500	7.08	6.58	2.3427	95.8	0.00	0.00

Manual Basin Mass Balance Summary [Proposed]

Basin Name	Sim Name	Total Rainfall	Total Irrigation	Total Runoff	Total ET	Total Initial Abst	Total Recharge	Change Soil Storage
Basin 2 [in]	100yr-72hr	18.00	0.00	17.44	0.00	0.00	0.00	0.56
Basin 2 [ft3]	100yr-72hr	153072	0	148344	0	0	0	4728
Basin 2 [ac-ft]	100yr-72hr	3.51	0.00	3.41	0.00	0.00	0.00	0.11
Basin 2 [in]	25yr-72hr	13.00	0.00	12.46	0.00	0.00	0.00	0.54
Basin 2 [ft3]	25yr-72hr	110552	0	105965	0	0	0	4587
Basin 2 [ac-ft]	25yr-72hr	2.54	0.00	2.43	0.00	0.00	0.00	0.11
Basin 2 [in]	5yr-24hr	7.08	0.00	6.58	0.00	0.00	0.00	0.50
Basin 2 [ft3]	5yr-24hr	60208	0	55929	0	0	0	4280
Basin 2 [ac-ft]	5yr-24hr	1.38	0.00	1.28	0.00	0.00	0.00	0.10

Manual Basin: Basin 3

Scenario: Proposed
 Node: Basin 3
 Hydrograph Method: NRCS Unit Hydrograph
 Infiltration Method: Curve Number
 Time of Concentration: 10.0000 min
 Max Allowable Q: 0.00 cfs
 Time Shift: 0.0000 hr
 Unit Hydrograph: UH256
 Peaking Factor: 256.0
 Area: 3.6183 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name	Crop Coefficient Zone	Reference ET Station
1.5764	Pervious	A			
2.0419	Impervious	A			

Comment:

Manual Basin Runoff Summary [Proposed]

Basin Name	Sim Name	Max Flow [cfs]	Time to Max Flow [hrs]	Total Rainfall [in]	Total Runoff [in]	Area [ac]	Equivalent Curve Number	% Imperv	% DCIA
Basin 3	100yr-72hr	28.53	60.0167	18.00	16.93	3.6183	91.6	0.00	0.00
Basin 3	25yr-72hr	20.46	60.0167	13.00	11.96	3.6183	91.7	0.00	0.00
Basin 3	5yr-24hr	14.32	12.0500	7.08	6.13	3.6183	92.0	0.00	0.00

Manual Basin Mass Balance Summary [Proposed]

Basin Name	Sim Name	Total Rainfall	Total Irrigation	Total Runoff	Total ET	Total Initial Abst	Total Recharge	Change Soil Storage
Basin 3 [in]	100yr-72hr	18.00	0.00	16.93	0.00	0.00	0.00	1.07
Basin 3 [ft3]	100yr-72hr	236420	0	222350	0	0	0	14070
Basin 3 [ac-ft]	100yr-72hr	5.43	0.00	5.10	0.00	0.00	0.00	0.32
Basin 3 [in]	25yr-72hr	13.00	0.00	11.96	0.00	0.00	0.00	1.04
Basin 3 [ft3]	25yr-72hr	170748	0	157125	0	0	0	13623
Basin 3 [ac-ft]	25yr-72hr	3.92	0.00	3.61	0.00	0.00	0.00	0.31
Basin 3 [in]	5yr-24hr	7.08	0.00	6.13	0.00	0.00	0.00	0.95
Basin 3 [ft3]	5yr-24hr	92992	0	80460	0	0	0	12532
Basin 3 [ac-ft]	5yr-24hr	2.13	0.00	1.85	0.00	0.00	0.00	0.29

Manual Basin: Basin 4

Scenario: Proposed
 Node: Basin 4
 Hydrograph Method: NRCS Unit Hydrograph
 Infiltration Method: Curve Number
 Time of Concentration: 10.0000 min
 Max Allowable Q: 0.00 cfs
 Time Shift: 0.0000 hr
 Unit Hydrograph: UH256
 Peaking Factor: 256.0
 Area: 4.1265 ac

Area [ac]	Land Cover Zone	Soil Zone	Rainfall Name	Crop Coefficient Zone	Reference ET Station
2.0019	Pervious	A			
2.1246	Impervious	A			

Comment:

Manual Basin Runoff Summary [Proposed]

Basin Name	Sim Name	Max Flow [cfs]	Time to Max Flow [hrs]	Total Rainfall [in]	Total Runoff [in]	Area [ac]	Equivalent Curve Number	% Imperv	% DCIA
Basin 4	100yr-72hr	32.51	60.0167	18.00	16.84	4.1265	90.9	0.00	0.00
Basin 4	25yr-72hr	23.29	60.0167	13.00	11.87	4.1265	91.0	0.00	0.00
Basin 4	5yr-24hr	16.21	12.0500	7.08	6.05	4.1265	91.3	0.00	0.00

Manual Basin Mass Balance Summary [Proposed]

Basin Name	Sim Name	Total Rainfall	Total Irrigation	Total Runoff	Total ET	Total Initial Abst	Total Recharge	Change Soil Storage
Basin 4 [in]	100yr-72hr	18.00	0.00	16.84	0.00	0.00	0.00	1.16
Basin 4 [ft3]	100yr-72hr	269626	0	252205	0	0	0	17421
Basin 4 [ac-ft]	100yr-72hr	6.19	0.00	5.79	0.00	0.00	0.00	0.40
Basin 4 [in]	25yr-72hr	13.00	0.00	11.87	0.00	0.00	0.00	1.13
Basin 4 [ft3]	25yr-72hr	194730	0	177866	0	0	0	16864
Basin 4 [ac-ft]	25yr-72hr	4.47	0.00	4.08	0.00	0.00	0.00	0.39
Basin 4 [in]	5yr-24hr	7.08	0.00	6.05	0.00	0.00	0.00	1.03
Basin 4 [ft3]	5yr-24hr	106053	0	90558	0	0	0	15494
Basin 4 [ac-ft]	5yr-24hr	2.43	0.00	2.08	0.00	0.00	0.00	0.36

Node: Basin 1

Scenario: Existing
 Type: Stage/Area
 Base Flow: 0.00 cfs
 Initial Stage: 4.80 ft
 Warning Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
9.55	0.0003	12
9.60	0.0006	28
9.70	0.0009	39
9.80	0.0014	61
9.90	0.0022	95
10.00	0.0030	132
10.10	0.0043	188
10.20	0.0064	278
10.30	0.0156	680
10.39	0.0303	1321
10.49	0.0699	3045
10.59	0.1324	5766
10.69	0.1974	8599
10.79	0.2783	12122
10.89	0.3984	17356
10.99	0.5573	24277
11.09	0.7017	30566
11.19	0.8366	36441
11.29	0.9733	42398
11.39	1.1098	48345

Stage [ft]	Area [ac]	Area [ft2]
11.49	1.2110	52749
11.59	1.2788	55706
11.69	1.3654	59475
11.79	1.4948	65114
11.89	1.6091	70094
11.99	1.7321	75450
12.09	1.8762	81727
12.19	2.0053	87353
12.29	2.1244	92538
12.39	2.2246	96902
12.49	2.3364	101773
12.59	2.4469	106588
12.69	2.5656	111756
12.79	2.6942	117358
12.89	2.8441	123887
12.99	2.9773	129691
13.09	3.1382	136701
13.19	3.3849	147446
13.29	3.6017	156892
13.39	3.7596	163766
13.49	3.8323	166933
13.59	3.8778	168916
13.69	3.9020	169971
13.79	3.9216	170826
13.89	3.9355	171429
13.99	3.9461	171894
14.09	3.9538	172228
14.20	3.9590	172454
14.30	3.9590	172454

Comment:

Node: Basin 2

Scenario: Existing
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 4.80 ft
Warning Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
11.44	0.0004	16
11.50	0.0016	70
11.60	0.0054	235
11.70	0.0171	746
11.80	0.0342	1488
11.89	0.0586	2552

Stage [ft]	Area [ac]	Area [ft2]
11.99	0.0961	4188
12.09	0.1414	6161
12.19	0.1912	8329
12.29	0.2362	10288
12.39	0.2786	12135
12.49	0.3270	14242
12.59	0.3872	16868
12.69	0.4475	19492
12.79	0.5294	23060
12.89	0.6017	26211
12.99	0.6657	28999
13.09	0.7394	32209
13.19	0.8355	36394
13.29	0.9052	39429
13.39	0.9432	41087
13.49	0.9915	43190
13.59	1.0272	44746
13.69	1.0518	45816
13.79	1.0693	46578
13.89	1.0881	47398
13.99	1.1096	48335
14.09	1.1280	49137
14.19	1.1449	49871
14.29	1.1656	50772
14.39	1.1796	51385
14.49	1.1945	52034
14.59	1.2086	52647
14.69	1.2226	53255
14.79	1.2352	53807
14.89	1.2477	54348
14.99	1.2617	54958
15.09	1.2694	55297
15.19	1.2823	55858
15.29	1.2967	56484
15.39	1.3040	56804
15.49	1.3100	57062
15.59	1.3186	57437
15.69	1.3268	57795
15.79	1.3337	58094
15.89	1.3396	58352
15.99	1.3481	58724
16.09	1.3540	58979
16.19	1.3610	59285
16.29	1.3658	59495
16.39	1.3734	59824
16.49	1.3808	60146
16.59	1.3905	60568
16.69	1.3956	60791
16.80	1.4009	61023

Stage [ft]	Area [ac]	Area [ft2]
16.89	1.4050	61201
16.99	1.4110	61465
17.09	1.4179	61764
17.20	1.4234	62004
17.30	1.4279	62198
17.40	1.4325	62399
17.50	1.4369	62592
17.60	1.4427	62844
17.70	1.4456	62971
17.80	1.4511	63208
17.90	1.4553	63391
18.00	1.4606	63624
18.10	1.4665	63881
18.20	1.4710	64078
18.30	1.4745	64228
18.40	1.4780	64383
18.50	1.4815	64535
18.60	1.4866	64756
18.70	1.4920	64991
18.80	1.4972	65218
18.90	1.5011	65388
19.00	1.5054	65574
19.10	1.5096	65760
19.20	1.5138	65940
19.30	1.5183	66139
19.40	1.5224	66315
19.50	1.5264	66488
19.60	1.5296	66631
19.70	1.5349	66862
19.80	1.5412	67136
19.90	1.5464	67362
20.00	1.5511	67566
20.10	1.5550	67736
20.20	1.5588	67903
20.30	1.5638	68120
20.40	1.5681	68306
20.50	1.5714	68450
20.60	1.5761	68653
20.70	1.5805	68844
20.80	1.5862	69093
20.90	1.5899	69257
21.00	1.5959	69519
21.10	1.6027	69813
21.20	1.6113	70187
21.30	1.6199	70564
21.40	1.6273	70884
21.50	1.6386	71378
21.60	1.6518	71954
21.70	1.6622	72406

Stage [ft]	Area [ac]	Area [ft2]
21.80	1.6674	72633
21.90	1.6744	72935
22.00	1.6791	73140
22.10	1.6838	73347
22.20	1.6888	73562
22.30	1.6932	73755
22.40	1.6986	73989
22.50	1.7034	74201
22.60	1.7070	74357
22.70	1.7112	74539
22.80	1.7161	74753
22.90	1.7206	74950
23.00	1.7253	75156
23.10	1.7295	75339
23.20	1.7342	75540
23.30	1.7393	75763
23.40	1.7459	76052
23.50	1.7532	76369
23.60	1.7615	76731
23.70	1.7687	77045
23.80	1.7753	77331
23.90	1.7823	77636
24.00	1.7888	77922
24.10	1.7973	78292
24.20	1.8055	78647
24.30	1.8139	79013
24.40	1.8215	79343
24.50	1.8295	79694
24.60	1.8370	80020
24.70	1.8457	80398
24.80	1.8532	80725
24.90	1.8615	81085
25.00	1.8714	81516
25.10	1.8817	81969
25.20	1.8938	82494
25.30	1.9115	83265
25.40	1.9734	85960
25.50	2.0126	87667
25.60	2.0900	91042
25.70	2.1033	91620
25.80	2.1057	91725
25.90	2.1057	91725

Comment:

Scenario: Existing
 Type: Stage/Area
 Base Flow: 0.00 cfs
 Initial Stage: 4.72 ft
 Warning Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
4.72	1.8495	80563
4.80	1.8964	82607
4.90	1.9214	83694
5.00	1.9387	84448
5.09	1.9480	84853
5.19	1.9590	85336
5.29	1.9683	85741
5.39	1.9785	86184
5.49	1.9869	86549
5.60	1.9937	86846
5.69	2.0022	87217
5.79	2.0157	87802
5.89	2.0256	88234
5.99	2.0353	88658
6.09	2.0448	89073
6.19	2.0523	89400
6.29	2.0603	89747
6.39	2.0731	90306
6.49	2.0831	90741
6.60	2.0936	91197
6.69	2.1004	91493
6.79	2.1113	91967
6.89	2.1192	92312
6.99	2.1320	92871
7.09	2.1441	93395
7.19	2.1543	93843
7.29	2.1676	94420
7.39	2.1775	94854
7.49	2.1916	95468
7.59	2.2017	95907
7.69	2.2131	96401
7.79	2.2244	96893
7.89	2.2316	97209
7.99	2.2441	97751
8.09	2.2568	98306
8.19	2.2691	98842
8.29	2.2796	99299
8.39	2.2957	100000
8.49	2.3062	100457
8.59	2.3148	100833
8.69	2.3253	101291
8.79	2.3370	101800
8.90	2.3474	102254

Stage [ft]	Area [ac]	Area [ft2]
8.99	2.3579	102712
9.09	2.3692	103203
9.19	2.3837	103834
9.29	2.3951	104331
9.39	2.4054	104780
9.49	2.4157	105228
9.59	2.4284	105781
9.69	2.4391	106245
9.79	2.4534	106870
9.89	2.4654	107391
9.99	2.4769	107893
10.09	2.4902	108474
10.19	2.5043	109086
10.29	2.5165	109618
10.39	2.5295	110184
10.49	2.5454	110878
10.59	2.5608	111548
10.69	2.5753	112180
10.79	2.5880	112732
10.89	2.6032	113394
10.99	2.6170	113995
11.09	2.6376	114895
11.19	2.6533	115579
11.29	2.6703	116320
11.39	2.6867	117032
11.49	2.7089	117998
11.59	2.7358	119173
11.69	2.7624	120331
11.79	2.7910	121575
11.89	2.8214	122902
11.99	2.8597	124569
12.09	2.9045	126518
12.19	2.9593	128907
12.29	3.0241	131728
12.39	3.0981	134953
12.49	3.1693	138055
12.59	3.2289	140652
12.69	3.2891	143271
12.79	3.3356	145300
12.89	3.3694	146769
12.99	3.3982	148024
13.09	3.4232	149115
13.19	3.4433	149992
13.29	3.4534	150429
13.39	3.4630	150848
13.49	3.4721	151244
13.59	3.4806	151613
13.69	3.4891	151987
13.79	3.4949	152236

Stage [ft]	Area [ac]	Area [ft2]
13.89	3.5035	152612
13.99	3.5096	152878
14.09	3.5152	153123
14.20	3.5222	153426
14.29	3.5316	153838
14.39	3.5376	154097
14.49	3.5423	154303
14.59	3.5489	154589
14.69	3.5539	154807
14.79	3.5629	155201
14.89	3.5684	155441
14.99	3.5762	155780
15.09	3.5818	156025
15.19	3.5872	156260
15.29	3.5920	156467
15.40	3.5994	156790
15.49	3.6026	156931
15.59	3.6085	157187
15.69	3.6165	157536
15.80	3.6217	157760
15.89	3.6257	157935
15.99	3.6311	158169
16.09	3.6368	158420
16.19	3.6410	158602
16.29	3.6449	158770
16.39	3.6499	158990
16.49	3.6579	159337
16.59	3.6615	159497
16.69	3.6651	159651
16.79	3.6712	159918
16.89	3.6750	160082
16.99	3.6808	160337
17.09	3.6848	160511
17.19	3.6895	160714
17.29	3.6949	160948
17.39	3.6997	161158
17.49	3.7030	161301
17.59	3.7070	161476
17.69	3.7108	161641
17.79	3.7161	161873
17.90	3.7208	162080
17.99	3.7255	162281
18.09	3.7315	162545
18.19	3.7365	162761
18.29	3.7399	162911
18.40	3.7444	163105
18.50	3.7498	163341
18.59	3.7538	163516
18.70	3.7584	163718

Stage [ft]	Area [ac]	Area [ft2]
18.80	3.7626	163897
18.90	3.7689	164174
19.00	3.7731	164358
19.10	3.7768	164518
19.20	3.7802	164665
19.30	3.7856	164901
19.40	3.7896	165077
19.50	3.7938	165256
19.60	3.7977	165428
19.70	3.8037	165687
19.80	3.8094	165937
19.90	3.8129	166091
20.00	3.8159	166220
20.10	3.8201	166405
20.20	3.8252	166625
20.30	3.8302	166843
20.40	3.8343	167020
20.50	3.8371	167146
20.60	3.8421	167361
20.70	3.8474	167592
20.80	3.8520	167793
20.90	3.8542	167891
21.00	3.8587	168086
21.10	3.8626	168255
21.20	3.8667	168435
21.30	3.8708	168611
21.40	3.8752	168805
21.50	3.8790	168968
21.60	3.8851	169236
21.70	3.8888	169397
21.80	3.8925	169557
21.90	3.8963	169723
22.00	3.8998	169875
22.10	3.9067	170176
22.20	3.9115	170387
22.30	3.9141	170499
22.40	3.9171	170629
22.50	3.9241	170934
22.60	3.9294	171164
22.70	3.9329	171315
22.80	3.9418	171704
22.90	3.9446	171828
23.00	3.9509	172100
23.10	3.9574	172383
23.20	3.9621	172587
23.30	3.9691	172892
23.40	3.9776	173264
23.50	3.9857	173615
23.60	3.9938	173968

Stage [ft]	Area [ac]	Area [ft2]
23.70	4.0031	174375
23.80	4.0135	174828
23.90	4.0251	175333
24.00	4.0367	175839
24.10	4.0497	176403
24.20	4.0639	177024
24.30	4.0781	177642
24.40	4.0921	178253
24.50	4.1068	178893
24.60	4.1193	179438
24.70	4.1362	180172
24.80	4.1586	181147
24.90	4.1791	182042
25.00	4.2009	182993
25.10	4.2247	184030
25.20	4.2446	184895
25.30	4.2570	185434
25.40	4.2807	186466
25.50	4.3071	187615
25.60	4.3071	187615

Comment:

Node: Basin 4

Scenario: Existing
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 9.76 ft
Warning Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
9.76	0.0004	18
9.80	0.0010	42
9.90	0.0056	243
10.00	0.0168	732
10.10	0.0295	1283
10.19	0.0440	1916
10.30	0.0589	2566
10.39	0.0750	3266
10.49	0.0955	4162
10.59	0.1214	5286
10.69	0.1455	6338
10.79	0.1758	7656
10.89	0.1979	8621
10.99	0.2314	10079
11.09	0.2699	11757

Stage [ft]	Area [ac]	Area [ft2]
11.19	0.3051	13289
11.29	0.3442	14993
11.39	0.3745	16313
11.49	0.3981	17342
11.59	0.4290	18687
11.69	0.4577	19938
11.79	0.5048	21989
11.89	0.5779	25172
11.99	0.6884	29987
12.09	0.8608	37496
12.19	1.0721	46702
12.29	1.2912	56245
12.39	1.5009	65379
12.49	1.7076	74384
12.59	1.8815	81956
12.69	2.0514	89361
12.79	2.1598	94080
12.89	2.2498	98003
12.99	2.3217	101132
13.09	2.3767	103530
13.19	2.4314	105912
13.29	2.4704	107610
13.39	2.5028	109022
13.49	2.5316	110278
13.59	2.5557	111328
13.69	2.5839	112556
13.79	2.6035	113410
13.89	2.6265	114412
13.99	2.6424	115105
14.09	2.6644	116061
14.19	2.6790	116699
14.29	2.6992	117575
14.39	2.7176	118377
14.49	2.7374	119239
14.59	2.7529	119917
14.69	2.7686	120600
14.79	2.7860	121359
14.89	2.7987	121912
14.99	2.8129	122528
15.09	2.8256	123082
15.19	2.8400	123711
15.29	2.8510	124188
15.39	2.8599	124579
15.49	2.8708	125053
15.59	2.8817	125527
15.69	2.8943	126077
15.79	2.9054	126561
15.89	2.9162	127030
15.99	2.9268	127492

Stage [ft]	Area [ac]	Area [ft2]
16.09	2.9371	127940
16.19	2.9456	128309
16.29	2.9547	128705
16.39	2.9637	129099
16.49	2.9719	129455
16.59	2.9801	129814
16.69	2.9886	130185
16.79	2.9944	130436
16.89	3.0048	130888
16.99	3.0113	131171
17.09	3.0172	131431
17.19	3.0233	131695
17.29	3.0306	132013
17.39	3.0403	132436
17.50	3.0456	132665
17.60	3.0513	132914
17.70	3.0573	133178
17.80	3.0634	133443
17.90	3.0697	133718
18.00	3.0761	133994
18.10	3.0826	134279
18.20	3.0899	134597
18.30	3.0952	134827
18.40	3.0998	135028
18.50	3.1062	135306
18.60	3.1096	135453
18.70	3.1169	135772
18.80	3.1221	136001
18.90	3.1264	136187
19.00	3.1328	136465
19.10	3.1410	136822
19.20	3.1463	137054
19.30	3.1517	137289
19.40	3.1563	137487
19.50	3.1632	137790
19.60	3.1693	138055
19.70	3.1743	138272
19.80	3.1821	138614
19.90	3.1876	138853
20.00	3.1952	139182
20.10	3.2021	139482
20.20	3.2091	139789
20.30	3.2172	140142
20.40	3.2243	140451
20.50	3.2326	140811
20.60	3.2407	141167
20.70	3.2503	141581
20.80	3.2620	142093
20.90	3.2759	142697

Stage [ft]	Area [ac]	Area [ft2]
21.00	3.2882	143236
21.10	3.2999	143745
21.20	3.3041	143928
21.30	3.3108	144217
21.40	3.3148	144391
21.50	3.3188	144566
21.60	3.3244	144812
21.70	3.3289	145006
21.80	3.3342	145238
21.90	3.3370	145359
22.00	3.3410	145536
22.10	3.3460	145752
22.20	3.3503	145940
22.30	3.3544	146116
22.40	3.3601	146367
22.50	3.3639	146531
22.60	3.3667	146654
22.70	3.3711	146845
22.80	3.3763	147073
22.90	3.3797	147218
23.00	3.3832	147374
23.10	3.3873	147552
23.20	3.3928	147792
23.30	3.3948	147877
23.40	3.3978	148006
23.50	3.4005	148125
23.60	3.4056	148350
23.70	3.4098	148530
23.80	3.4136	148695
23.90	3.4182	148895
24.00	3.4225	149086
24.10	3.4284	149342
24.20	3.4345	149608
24.30	3.4393	149818
24.40	3.4454	150082
24.50	3.4525	150391
24.60	3.4591	150679
24.70	3.4663	150994
24.80	3.4760	151416
24.90	3.4850	151808
25.00	3.4958	152279
25.10	3.5046	152661
25.20	3.5147	153099
25.30	3.5242	153514
25.40	3.5520	154725
25.50	3.5825	156055
25.60	3.6394	158531
25.70	3.6564	159271
25.80	3.6704	159881

Stage [ft]	Area [ac]	Area [ft2]
25.90	3.6852	160529
26.00	3.6947	160943
26.10	3.6947	160943

Comment:

Node: NZA-0040

Scenario: Existing
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: NZA-0050

Scenario: Existing
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: NZA-0060

Scenario: Existing
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: NZA-0070

Scenario: Existing
Type: Stage/Area

Base Flow: 0.00 cfs
 Initial Stage: 0.00 ft
 Warning Stage: 0.00 ft

Comment:

Node: NZA-0080

Scenario: Existing
 Type: Stage/Area
 Base Flow: 0.00 cfs
 Initial Stage: 0.00 ft
 Warning Stage: 0.00 ft

Comment:

Node: NZA-0090

Scenario: Existing
 Type: Stage/Area
 Base Flow: 0.00 cfs
 Initial Stage: 0.00 ft
 Warning Stage: 0.00 ft

Comment:

Node: Basin 1

Scenario: Proposed
 Type: Stage/Area
 Base Flow: 0.00 cfs
 Initial Stage: 4.80 ft
 Warning Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
9.55	0.0003	12
9.60	0.0006	28
9.70	0.0009	39
9.80	0.0014	61
9.90	0.0022	95
10.00	0.0030	132
10.10	0.0043	188

Stage [ft]	Area [ac]	Area [ft2]
10.20	0.0064	278
10.30	0.0156	680
10.39	0.0303	1321
10.49	0.0699	3045
10.59	0.1324	5766
10.69	0.1974	8599
10.79	0.2783	12122
10.89	0.3984	17356
10.99	0.5573	24277
11.09	0.7017	30566
11.19	0.8366	36441
11.29	0.9733	42398
11.39	1.1098	48345
11.49	1.2110	52749
11.59	1.2788	55706
11.69	1.3654	59475
11.79	1.4948	65114
11.89	1.6091	70094
11.99	1.7321	75450
12.09	1.8762	81727
12.19	2.0053	87353
12.29	2.1244	92538
12.39	2.2246	96902
12.49	2.3364	101773
12.59	2.4469	106588
12.69	2.5656	111756
12.79	2.6942	117358
12.89	2.8441	123887
12.99	2.9773	129691
13.09	3.1382	136701
13.19	3.3849	147446
13.29	3.6017	156892
13.39	3.7596	163766
13.49	3.8323	166933
13.59	3.8778	168916
13.69	3.9020	169971
13.79	3.9216	170826
13.89	3.9355	171429
13.99	3.9461	171894
14.09	3.9538	172228
14.20	3.9590	172454
14.30	3.9590	172454

Comment:

Scenario: Proposed
 Type: Stage/Area
 Base Flow: 0.00 cfs
 Initial Stage: 4.80 ft
 Warning Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
11.44	0.0004	16
11.50	0.0016	70
11.60	0.0044	193
11.70	0.0101	439
11.80	0.0213	929
11.89	0.0408	1778
11.99	0.0711	3098
12.09	0.1126	4904
12.19	0.1785	7776
12.29	0.2384	10385
12.39	0.2979	12976
12.49	0.3599	15679
12.59	0.4216	18363
12.69	0.4943	21530
12.79	0.5961	25967
12.89	0.6982	30415
12.99	0.8008	34881
13.09	1.1236	48943
13.19	1.2138	52874
13.29	1.2826	55872
13.39	1.3284	57865
13.49	1.3800	60111
13.59	1.4076	61316
13.69	1.4261	62120
13.79	1.4408	62760
13.89	1.4554	63396
13.99	1.4695	64013
14.09	1.4818	64548
14.19	1.4947	65111
14.29	1.5089	65729
14.39	1.5250	66429
14.49	1.5393	67052
14.59	1.5522	67612
14.69	1.5643	68141
14.79	1.5752	68616
14.89	1.5843	69014
14.99	1.5933	69403
15.09	1.6017	69768
15.19	1.6102	70141
15.29	1.6190	70523
15.39	1.6271	70878
15.49	1.6344	71193
15.59	1.6418	71515

Stage [ft]	Area [ac]	Area [ft2]
15.69	1.6492	71838
15.79	1.6565	72158
15.89	1.6636	72467
15.99	1.6710	72790
16.09	1.6778	73083
16.19	1.6845	73378
16.29	1.6913	73674
16.39	1.6982	73974
16.49	1.7048	74259
16.59	1.7112	74542
16.69	1.7172	74802
16.79	1.7237	75085
16.89	1.7301	75363
16.99	1.7359	75615
17.09	1.7421	75884
17.20	1.7489	76182
17.30	1.7545	76425
17.40	1.7603	76678
17.50	1.7666	76955
17.60	1.7716	77171
17.70	1.7771	77411
17.80	1.7827	77655
17.90	1.7884	77901
18.00	1.7934	78122
18.10	1.7988	78354
18.20	1.8041	78585
18.30	1.8099	78841
18.40	1.8149	79059
18.50	1.8203	79294
18.60	1.8256	79522
18.70	1.8309	79754
18.80	1.8363	79991
18.90	1.8417	80226
19.00	1.8468	80446
19.10	1.8522	80682
19.20	1.8573	80903
19.30	1.8626	81134
19.40	1.8687	81400
19.50	1.8752	81683
19.60	1.8823	81991
19.70	1.8904	82344
19.80	1.8977	82662
19.90	1.9054	82999
20.00	1.9132	83337
20.10	1.9216	83704
20.20	1.9306	84098
20.30	1.9396	84487
20.40	1.9475	84832
20.50	1.9547	85147

Stage [ft]	Area [ac]	Area [ft2]
20.60	1.9618	85454
20.70	1.9691	85773
20.80	1.9761	86079
20.90	1.9832	86388
21.00	1.9891	86645
21.10	1.9950	86902
21.20	2.0010	87165
21.30	2.0067	87412
21.40	2.0118	87635
21.50	2.0167	87846
21.60	2.0217	88067
21.70	2.0264	88271
21.80	2.0301	88430
21.90	2.0336	88584
22.00	2.0375	88752
22.10	2.0412	88915
22.20	2.0446	89062
22.30	2.0485	89233
22.40	2.0522	89392
22.50	2.0556	89544
22.60	2.0589	89687
22.70	2.0618	89814
22.80	2.0643	89922
22.90	2.0664	90012
23.00	2.0682	90092
23.10	2.0699	90165
23.20	2.0713	90228
23.30	2.0729	90295
23.40	2.0743	90358
23.50	2.0759	90426
23.60	2.0774	90493
23.70	2.0789	90558
23.80	2.0803	90620
23.90	2.0818	90685
24.00	2.0833	90749
24.10	2.0849	90820
24.20	2.0864	90883
24.30	2.0880	90952
24.40	2.0895	91018
24.50	2.0910	91084
24.60	2.0924	91146
24.70	2.0939	91211
24.80	2.0953	91273
24.90	2.0969	91341
25.00	2.0983	91403
25.10	2.0998	91468
25.20	2.1013	91531
25.30	2.1028	91597
25.40	2.1057	91725

Stage [ft]	Area [ac]	Area [ft2]
25.50	2.1057	91725

Comment:

Node: Basin 3

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 4.72 ft
Warning Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
4.72	1.8495	80563
4.80	1.8964	82607
4.90	1.9214	83694
5.00	1.9387	84448
5.09	1.9480	84853
5.19	1.9590	85336
5.29	1.9683	85741
5.39	1.9785	86184
5.49	1.9869	86549
5.60	1.9937	86846
5.69	2.0022	87217
5.79	2.0157	87802
5.89	2.0256	88234
5.99	2.0353	88658
6.09	2.0448	89073
6.19	2.0523	89400
6.29	2.0603	89747
6.39	2.0731	90306
6.49	2.0831	90741
6.60	2.0936	91197
6.69	2.1004	91493
6.79	2.1113	91967
6.89	2.1192	92312
6.99	2.1320	92871
7.09	2.1441	93395
7.19	2.1543	93843
7.29	2.1676	94420
7.39	2.1775	94854
7.49	2.1916	95468
7.59	2.2017	95907
7.69	2.2130	96400
7.79	2.2243	96892
7.89	2.2316	97208
7.99	2.2439	97746

Stage [ft]	Area [ac]	Area [ft2]
8.09	2.2567	98301
8.19	2.2689	98833
8.29	2.2794	99290
8.39	2.2953	99985
8.49	2.3057	100435
8.59	2.3140	100799
8.69	2.3245	101257
8.79	2.3362	101766
8.90	2.3461	102194
8.99	2.3562	102634
9.09	2.3672	103116
9.19	2.3817	103747
9.29	2.3927	104226
9.39	2.4030	104675
9.49	2.4130	105111
9.59	2.4253	105644
9.69	2.4359	106108
9.79	2.4498	106712
9.89	2.4606	107184
9.99	2.4716	107663
10.09	2.4847	108232
10.19	2.4985	108835
10.29	2.5101	109338
10.39	2.5226	109886
10.49	2.5383	110568
10.59	2.5537	111238
10.69	2.5674	111837
10.79	2.5792	112350
10.89	2.5942	113003
10.99	2.6070	113560
11.09	2.6270	114430
11.19	2.6424	115105
11.29	2.6590	115826
11.39	2.6751	116526
11.49	2.6971	117486
11.59	2.7225	118592
11.69	2.7475	119680
11.79	2.7756	120906
11.89	2.8057	122215
11.99	2.8435	123861
12.09	2.8870	125757
12.19	2.9428	128187
12.29	3.0024	130786
12.39	3.0712	133783
12.49	3.1366	136631
12.59	3.1874	138841
12.69	3.2402	141141
12.79	3.2801	142879
12.89	3.3104	144200

Stage [ft]	Area [ac]	Area [ft2]
12.99	3.3370	145358
13.09	3.3593	146329
13.19	3.3757	147045
13.29	3.3842	147414
13.39	3.3918	147746
13.49	3.3996	148086
13.59	3.4065	148386
13.69	3.4132	148679
13.79	3.4191	148938
13.89	3.4265	149258
13.99	3.4318	149488
14.09	3.4367	149701
14.19	3.4433	149991
14.29	3.4538	150448
14.39	3.4593	150685
14.49	3.4653	150948
14.59	3.4722	151247
14.69	3.4795	151569
14.79	3.4892	151991
14.89	3.4970	152331
14.99	3.5075	152785
15.09	3.5150	153115
15.19	3.5234	153480
15.29	3.5318	153844
15.39	3.5410	154246
15.49	3.5487	154581
15.59	3.5587	155017
15.69	3.5708	155542
15.79	3.5795	155922
15.89	3.5887	156325
15.99	3.5979	156726
16.09	3.6082	157171
16.19	3.6185	157622
16.29	3.6273	158004
16.39	3.6376	158454
16.49	3.6503	159009
16.59	3.6600	159428
16.69	3.6693	159833
16.79	3.6806	160328
16.89	3.6911	160785
16.99	3.7011	161219
17.09	3.7086	161547
17.19	3.7173	161927
17.29	3.7275	162372
17.39	3.7366	162768
17.49	3.7453	163145
17.59	3.7534	163499
17.69	3.7611	163835
17.79	3.7704	164239

Stage [ft]	Area [ac]	Area [ft2]
17.89	3.7790	164615
17.99	3.7877	164991
18.09	3.7977	165426
18.19	3.8063	165804
18.29	3.8140	166137
18.39	3.8223	166499
18.49	3.8316	166903
18.59	3.8401	167275
18.70	3.8505	167728
18.80	3.8589	168093
18.90	3.8689	168531
19.00	3.8776	168907
19.10	3.8851	169234
19.20	3.8936	169605
19.30	3.9020	169970
19.40	3.9097	170305
19.50	3.9175	170645
19.60	3.9260	171017
19.70	3.9343	171380
19.80	3.9423	171725
19.90	3.9496	172043
20.00	3.9575	172390
20.10	3.9654	172732
20.20	3.9723	173035
20.30	3.9802	173378
20.40	3.9877	173705
20.50	3.9946	174004
20.60	4.0025	174350
20.70	4.0097	174661
20.80	4.0165	174960
20.90	4.0228	175234
21.00	4.0306	175572
21.10	4.0379	175890
21.20	4.0456	176228
21.30	4.0537	176580
21.40	4.0624	176958
21.50	4.0703	177304
21.60	4.0781	177644
21.70	4.0866	178014
21.80	4.0956	178403
21.90	4.1044	178786
22.00	4.1130	179163
22.10	4.1235	179621
22.20	4.1332	180040
22.30	4.1426	180451
22.40	4.1529	180899
22.50	4.1634	181359
22.60	4.1759	181903
22.70	4.1914	182576

Stage [ft]	Area [ac]	Area [ft2]
22.80	4.2111	183435
22.90	4.2342	184443
23.00	4.2629	185692
23.10	4.2809	186478
23.20	4.2902	186880
23.30	4.2975	187199
23.40	4.3049	187522
23.50	4.3071	187615
23.60	4.3071	187615

Comment:

Node: Basin 4

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 9.76 ft
Warning Stage: 0.00 ft

Stage [ft]	Area [ac]	Area [ft2]
9.76	0.0004	18
9.80	0.0010	42
9.90	0.0056	243
10.00	0.0168	732
10.10	0.0295	1283
10.19	0.0440	1916
10.30	0.0589	2566
10.39	0.0750	3266
10.49	0.0955	4162
10.59	0.1214	5286
10.69	0.1455	6338
10.79	0.1758	7656
10.89	0.1979	8621
10.99	0.2314	10079
11.09	0.2699	11757
11.19	0.3051	13289
11.29	0.3442	14993
11.39	0.3745	16313
11.49	0.3981	17342
11.59	0.4287	18675
11.69	0.4571	19910
11.79	0.5034	21928
11.89	0.5758	25084
11.99	0.6844	29814
12.09	0.8547	37231
12.19	1.0652	46401

Stage [ft]	Area [ac]	Area [ft2]
12.29	1.2860	56019
12.39	1.4982	65261
12.49	1.7095	74465
12.59	1.8881	82247
12.69	2.0652	89962
12.79	2.1830	95092
12.89	2.2840	99489
12.99	2.3654	103035
13.09	2.5276	110104
13.19	2.5858	112637
13.29	2.6247	114332
13.39	2.6620	115956
13.49	2.6927	117294
13.59	2.7219	118568
13.69	2.7492	119754
13.79	2.7767	120954
13.89	2.8020	122057
13.99	2.8279	123183
14.09	2.8534	124296
14.19	2.8777	125351
14.29	2.9032	126465
14.39	2.9271	127504
14.49	2.9514	128563
14.59	2.9721	129463
14.69	2.9901	130248
14.79	3.0074	131004
14.89	3.0231	131687
14.99	3.0410	132466
15.09	3.0581	133212
15.19	3.0756	133972
15.29	3.0923	134702
15.39	3.1103	135485
15.49	3.1273	136224
15.59	3.1447	136982
15.69	3.1654	137883
15.79	3.1819	138602
15.89	3.1975	139284
15.99	3.2120	139914
16.09	3.2253	140493
16.19	3.2377	141034
16.29	3.2494	141543
16.39	3.2603	142019
16.49	3.2708	142474
16.59	3.2811	142926
16.69	3.2900	143311
16.79	3.2988	143697
16.89	3.3088	144131
16.99	3.3218	144697
17.09	3.3333	145198

Stage [ft]	Area [ac]	Area [ft2]
17.19	3.3428	145613
17.29	3.3498	145917
17.39	3.3566	146214
17.50	3.3637	146524
17.60	3.3697	146782
17.70	3.3755	147038
17.80	3.3815	147297
17.90	3.3871	147542
18.00	3.3932	147808
18.10	3.3994	148078
18.20	3.4053	148333
18.30	3.4107	148570
18.40	3.4163	148815
18.50	3.4216	149046
18.60	3.4270	149281
18.70	3.4327	149530
18.80	3.4381	149765
18.90	3.4435	150001
19.00	3.4488	150230
19.10	3.4546	150482
19.20	3.4598	150710
19.30	3.4652	150943
19.40	3.4704	151172
19.50	3.4755	151391
19.60	3.4803	151600
19.70	3.4852	151816
19.80	3.4899	152022
19.90	3.4950	152244
20.00	3.5003	152475
20.10	3.5049	152675
20.20	3.5101	152900
20.30	3.5147	153099
20.40	3.5192	153298
20.50	3.5240	153506
20.60	3.5289	153720
20.70	3.5332	153908
20.80	3.5385	154135
20.90	3.5429	154329
21.00	3.5477	154539
21.10	3.5526	154751
21.20	3.5570	154941
21.30	3.5613	155130
21.40	3.5658	155326
21.50	3.5701	155515
21.60	3.5744	155703
21.70	3.5788	155894
21.80	3.5828	156066
21.90	3.5868	156243
22.00	3.5907	156409

Stage [ft]	Area [ac]	Area [ft2]
22.10	3.5944	156571
22.20	3.5984	156745
22.30	3.6024	156920
22.40	3.6062	157086
22.50	3.6105	157272
22.60	3.6143	157441
22.70	3.6184	157616
22.80	3.6225	157794
22.90	3.6263	157960
23.00	3.6291	158082
23.10	3.6317	158198
23.20	3.6344	158315
23.30	3.6371	158430
23.40	3.6396	158539
23.50	3.6423	158658
23.60	3.6449	158770
23.70	3.6474	158882
23.80	3.6500	158994
23.90	3.6525	159104
24.00	3.6550	159212
24.10	3.6574	159317
24.20	3.6599	159424
24.30	3.6624	159534
24.40	3.6649	159644
24.50	3.6673	159746
24.60	3.6697	159851
24.70	3.6720	159954
24.80	3.6744	160057
24.90	3.6767	160159
25.00	3.6791	160262
25.10	3.6815	160365
25.20	3.6838	160465
25.30	3.6860	160562
25.40	3.6947	160943
25.50	3.6947	160943

Comment:

Node: CB-01

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: CB-02

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: CB-03

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: CB-04

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: CB-05

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: CB-06

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: CI-01

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: MH-01

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: MH-02

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: MH-03

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: MH-05

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: MH-06

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: NTZ-0190

Scenario: Proposed
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 4.80 ft
Warning Stage: 0.00 ft
Boundary Stage:

Year	Month	Day	Hour	Stage [ft]
0	0	0	0.0000	4.80

Comment:

Node: NTZ-0290

Scenario: Proposed
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft
Boundary Stage:

Comment:

Node: NTZ-0300

Scenario: Proposed
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft
Boundary Stage:

Comment:

Node: NTZ-0320

Scenario: Proposed
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft
Boundary Stage:

Comment:

Node: NTZ-0340

Scenario: Proposed

Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft
Boundary Stage:

Comment:

Node: NTZ-0380

Scenario: Proposed
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft
Boundary Stage:

Comment:

Node: NTZ-0390

Scenario: Proposed
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft
Boundary Stage:

Comment:

Node: NTZ-0400

Scenario: Proposed
Type: Time/Stage
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft
Boundary Stage:

Comment:

Node: NZA-0040

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: NZA-0050

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: NZA-0080

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: NZA-0240

Scenario: Proposed
Type: Stage/Area
Base Flow: 0.00 cfs
Initial Stage: 0.00 ft
Warning Stage: 0.00 ft

Comment:

Node: NZA-0280

Scenario: Proposed
 Type: Stage/Area
 Base Flow: 0.00 cfs
 Initial Stage: 0.00 ft
 Warning Stage: 0.00 ft

Comment:

Pipe Link: L-0030P

Scenario: Existing
 From Node: NZA-0040
 To Node: NZA-0050
 Link Count: 1
 Flow Direction: Both
 Damping: 0.0000 ft
 Length: 72.29 ft
 FHWA Code: 0
 Entr Loss Coef: 0.50
 Exit Loss Coef: 1.00
 Bend Loss Coef: 0.00
 Bend Location: 0.00 dec
 Energy Switch: Energy

Upstream

Invert: 5.20 ft
 Manning's N: 0.0240
 Geometry: Circular
 Max Depth: 1.50 ft
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000

Downstream

Invert: 5.07 ft
 Manning's N: 0.0240
 Geometry: Circular
 Max Depth: 1.50 ft
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000

Bottom Clip

Top Clip

Comment:

Pipe Link: L-0040P

Scenario: Existing
 From Node: NZA-0050
 To Node: Basin 3
 Link Count: 1
 Flow Direction: Both
 Damping: 0.0000 ft
 Length: 36.33 ft
 FHWA Code: 0
 Entr Loss Coef: 0.50
 Exit Loss Coef: 1.00
 Bend Loss Coef: 0.00
 Bend Location: 0.00 dec
 Energy Switch: Energy

Upstream

Invert: 4.97 ft
 Manning's N: 0.0240
 Geometry: Circular
 Max Depth: 1.25 ft
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000

Downstream

Invert: 3.44 ft
 Manning's N: 0.0240
 Geometry: Circular
 Max Depth: 1.25 ft
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000

Bottom Clip

Top Clip

Comment:

Weir Link: L-0050W

Scenario:	Existing	Bottom Clip
From Node:	Basin 3	Default: 0.00 ft
To Node:	NZA-0050	Op Table:
Link Count:	26	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Horizontal	Op Table:
Geometry Type:	Rectangular	Ref Node:
Invert:	11.87 ft	Discharge Coefficients
Control Elevation:	11.87 ft	Weir Default: 2.800
Max Depth:	0.14 ft	Weir Table:
Max Width:	0.92 ft	Orifice Default: 0.600
Fillet:	0.00 ft	Orifice Table:

Comment:

Weir Link: L-0060W

Scenario:	Existing	Bottom Clip
From Node:	Basin 1	Default: 0.00 ft
To Node:	NZA-0040	Op Table:
Link Count:	26	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Horizontal	Op Table:
Geometry Type:	Rectangular	Ref Node:
Invert:	10.20 ft	Discharge Coefficients
Control Elevation:	10.20 ft	Weir Default: 2.800
Max Depth:	0.14 ft	Weir Table:
Max Width:	0.92 ft	Orifice Default: 0.600
Fillet:	0.00 ft	Orifice Table:

Comment:

Weir Link: L-0070W

Scenario:	Existing	Bottom Clip
From Node:	Basin 1	Default: 0.00 ft
To Node:	Basin 3	Op Table:
Link Count:	1	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Broad Crested Vertical	Op Table:
Geometry Type:	Irregular	Ref Node:
Invert:	12.46 ft	Discharge Coefficients
Control Elevation:	12.46 ft	Weir Default: 2.800
Cross Section:	X-0220W	Weir Table:
		Orifice Default: 0.600

Orifice Table:

Comment:

Pipe Link: L-0140P	Upstream	Downstream
Scenario: Existing	Invert: 6.52 ft	Invert: 6.02 ft
From Node: NZA-0060	Manning's N: 0.0240	Manning's N: 0.0240
To Node: NZA-0070	Geometry: Circular	Geometry: Circular
Link Count: 1	Max Depth: 1.50 ft	Max Depth: 1.50 ft
Flow Direction: Both	Bottom Clip	
Damping: 0.0000 ft	Default: 0.00 ft	Default: 0.00 ft
Length: 184.16 ft	Op Table:	Op Table:
FHWA Code: 0	Ref Node:	Ref Node:
Entr Loss Coef: 0.50	Manning's N: 0.0000	Manning's N: 0.0000
Exit Loss Coef: 1.00	Top Clip	
Bend Loss Coef: 0.00	Default: 0.00 ft	Default: 0.00 ft
Bend Location: 0.00 dec	Op Table:	Op Table:
Energy Switch: Energy	Ref Node:	Ref Node:
	Manning's N: 0.0000	Manning's N: 0.0000

Comment:

Pipe Link: L-0150P	Upstream	Downstream
Scenario: Existing	Invert: 0.50 ft	Invert: 0.13 ft
From Node: NZA-0090	Manning's N: 0.0240	Manning's N: 0.0240
To Node: NZA-0050	Geometry: Circular	Geometry: Circular
Link Count: 1	Max Depth: 2.00 ft	Max Depth: 2.00 ft
Flow Direction: Both	Bottom Clip	
Damping: 0.0000 ft	Default: 0.00 ft	Default: 0.00 ft
Length: 175.39 ft	Op Table:	Op Table:
FHWA Code: 0	Ref Node:	Ref Node:
Entr Loss Coef: 0.50	Manning's N: 0.0000	Manning's N: 0.0000
Exit Loss Coef: 1.00	Top Clip	
Bend Loss Coef: 0.00	Default: 0.00 ft	Default: 0.00 ft
Bend Location: 0.00 dec	Op Table:	Op Table:
Energy Switch: Energy	Ref Node:	Ref Node:
	Manning's N: 0.0000	Manning's N: 0.0000

Comment:

Weir Link: L-0160W	Bottom Clip
Scenario: Existing	Default: 0.00 ft
From Node: Basin 2	Op Table:
To Node: NZA-0060	

Link Count: 26
 Flow Direction: Both
 Damping: 0.0000 ft
 Weir Type: Horizontal
 Geometry Type: Rectangular
 Invert: 11.56 ft
 Control Elevation: 11.56 ft
 Max Depth: 0.14 ft
 Max Width: 0.92 ft
 Fillet: 0.00 ft

Ref Node:

Top Clip

Default: 0.00 ft

Op Table:

Ref Node:

Discharge Coefficients

Weir Default: 2.800

Weir Table:

Orifice Default: 0.600

Orifice Table:

Comment:

Drop Structure Link: L-0190DS

Upstream Pipe

Downstream Pipe

Scenario: Existing
 From Node: Basin 4
 To Node: Basin 3
 Link Count: 1
 Flow Direction: Both
 Solution: Combine
 Increments: 0
 Pipe Count: 1
 Damping: 0.0000 ft
 Length: 40.05 ft
 FHWA Code: 0
 Entr Loss Coef: 0.50
 Exit Loss Coef: 1.00
 Bend Loss Coef: 0.00
 Bend Location: 0.00 dec
 Energy Switch: Energy

Invert: 5.23 ft

Invert: 2.87 ft

Manning's N: 0.0240

Manning's N: 0.0240

Geometry: Circular

Geometry: Circular

Max Depth: 1.25 ft

Max Depth: 1.25 ft

Bottom Clip

Default: 0.00 ft

Default: 0.00 ft

Op Table:

Op Table:

Ref Node:

Ref Node:

Manning's N: 0.0000

Manning's N: 0.0000

Top Clip

Default: 0.00 ft

Default: 0.00 ft

Op Table:

Op Table:

Ref Node:

Ref Node:

Manning's N: 0.0000

Manning's N: 0.0000

Pipe Comment:

Weir Component

Weir: 1

Bottom Clip

Weir Count: 156

Default: 0.00 ft

Weir Flow Direction: Both

Op Table:

Damping: 0.0000 ft

Ref Node:

Weir Type: Horizontal

Top Clip

Geometry Type: Rectangular

Default: 0.00 ft

Invert: 11.85 ft

Op Table:

Control Elevation: 11.85 ft

Ref Node:

Max Depth: 0.14 ft

Discharge Coefficients

Max Width: 0.92 ft

Weir Default: 3.200

Fillet: 0.00 ft

Weir Table:

Orifice Default: 0.600

Orifice Table:

Weir Comment:

Drop Structure Comment:

Drop Structure Link: L-0200DS		Upstream Pipe	Downstream Pipe
Scenario:	Existing	Invert: 5.74 ft	Invert: 5.22 ft
From Node:	Basin 1	Manning's N: 0.0120	Manning's N: 0.0120
To Node:	NZA-0040	Geometry: Circular	Geometry: Circular
Link Count:	1	Max Depth: 1.50 ft	Max Depth: 1.50 ft
Flow Direction:	Both	Bottom Clip	
Solution:	Combine	Default: 0.00 ft	Default: 0.00 ft
Increments:	0	Op Table:	Op Table:
Pipe Count:	1	Ref Node:	Ref Node:
Damping:	0.0000 ft	Manning's N: 0.0000	Manning's N: 0.0000
Length:	227.27 ft	Top Clip	
FHWA Code:	0	Default: 0.00 ft	Default: 0.00 ft
Entr Loss Coef:	0.50	Op Table:	Op Table:
Exit Loss Coef:	1.00	Ref Node:	Ref Node:
Bend Loss Coef:	0.00	Manning's N: 0.0000	Manning's N: 0.0000
Bend Location:	0.00 dec		
Energy Switch:	Energy		

Pipe Comment:

Weir Component		
Weir:	1	Bottom Clip
Weir Count:	26	Default: 0.00 ft
Weir Flow Direction:	Both	Op Table:
Damping:	0.0000 ft	Ref Node:
Weir Type:	Horizontal	Top Clip
Geometry Type:	Rectangular	Default: 0.00 ft
Invert:	9.64 ft	Op Table:
Control Elevation:	9.64 ft	Ref Node:
Max Depth:	0.14 ft	Discharge Coefficients
Max Width:	0.92 ft	Weir Default: 3.200
Fillet:	0.00 ft	Weir Table:
		Orifice Default: 0.600
		Orifice Table:

Weir Comment:

Drop Structure Comment:

Drop Structure Link: L-0210DS		Upstream Pipe	Downstream Pipe
Scenario:	Existing	Invert: 6.61 ft	Invert: 6.61 ft
From Node:	Basin 2	Manning's N: 0.0240	Manning's N: 0.0240
To Node:	NZA-0060	Geometry: Circular	Geometry: Circular
Link Count:	1	Max Depth: 1.25 ft	Max Depth: 1.25 ft

Flow Direction:	Both	Bottom Clip	
Solution:	Combine	Default: 0.00 ft	Default: 0.00 ft
Increments:	0	Op Table:	Op Table:
Pipe Count:	1	Ref Node:	Ref Node:
Damping:	0.0000 ft	Manning's N: 0.0000	Manning's N: 0.0000
Length:	18.54 ft	Top Clip	
FHWA Code:	0	Default: 0.00 ft	Default: 0.00 ft
Entr Loss Coef:	0.50	Op Table:	Op Table:
Exit Loss Coef:	1.00	Ref Node:	Ref Node:
Bend Loss Coef:	0.00	Manning's N: 0.0000	Manning's N: 0.0000
Bend Location:	0.00 dec		
Energy Switch:	Energy		
Pipe Comment:			

Weir Component			
Weir:	1	Bottom Clip	
Weir Count:	26	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0000 ft	Ref Node:	
Weir Type:	Horizontal	Top Clip	
Geometry Type:	Rectangular	Default:	0.00 ft
Invert:	11.56 ft	Op Table:	
Control Elevation:	11.56 ft	Ref Node:	
Max Depth:	0.14 ft	Discharge Coefficients	
Max Width:	0.92 ft	Weir Default:	3.200
Fillet:	0.00 ft	Weir Table:	
		Orifice Default:	0.600
		Orifice Table:	
Weir Comment:			

Drop Structure Comment:

Drop Structure Link: L-0220DS		Upstream Pipe	Downstream Pipe
Scenario:	Existing	Invert: 5.55 ft	Invert: 4.83 ft
From Node:	Basin 4	Manning's N: 0.0120	Manning's N: 0.0120
To Node:	NZA-0080	Geometry: Circular	Geometry: Circular
Link Count:	1	Max Depth: 1.25 ft	Max Depth: 1.25 ft
Flow Direction:	Both	Bottom Clip	
Solution:	Combine	Default: 0.00 ft	Default: 0.00 ft
Increments:	0	Op Table:	Op Table:
Pipe Count:	1	Ref Node:	Ref Node:
Damping:	0.0000 ft	Manning's N: 0.0000	Manning's N: 0.0000
Length:	34.07 ft	Top Clip	
FHWA Code:	0	Default: 0.00 ft	Default: 0.00 ft
Entr Loss Coef:	0.50	Op Table:	Op Table:
Exit Loss Coef:	1.00	Ref Node:	Ref Node:
Bend Loss Coef:	0.00	Manning's N: 0.0000	Manning's N: 0.0000

Bend Location: 0.00 dec

Energy Switch: Energy

Pipe Comment:

Weir Component	
Weir: 1	Bottom Clip
Weir Count: 26	Default: 0.00 ft
Weir Flow Direction: Both	Op Table:
Damping: 0.0000 ft	Ref Node:
Weir Type: Horizontal	Top Clip
Geometry Type: Rectangular	Default: 0.00 ft
Invert: 11.93 ft	Op Table:
Control Elevation: 11.93 ft	Ref Node:
Max Depth: 0.14 ft	Discharge Coefficients
Max Width: 0.92 ft	Weir Default: 3.200
Fillet: 0.00 ft	Weir Table:
	Orifice Default: 0.600
	Orifice Table:

Weir Comment:

Drop Structure Comment:

Pipe Link: L-0230P	Upstream	Downstream
Scenario: Existing	Invert: 5.24 ft	Invert: 3.65 ft
From Node: NZA-0080	Manning's N: 0.0120	Manning's N: 0.0120
To Node: Basin 3	Geometry: Circular	Geometry: Circular
Link Count: 1	Max Depth: 1.25 ft	Max Depth: 1.25 ft
Flow Direction: Both	Bottom Clip	
Damping: 0.0000 ft	Default: 0.00 ft	Default: 0.00 ft
Length: 43.28 ft	Op Table:	Op Table:
FHWA Code: 0	Ref Node:	Ref Node:
Entr Loss Coef: 0.50	Manning's N: 0.0000	Manning's N: 0.0000
Exit Loss Coef: 1.00	Top Clip	
Bend Loss Coef: 0.00	Default: 0.00 ft	Default: 0.00 ft
Bend Location: 0.00 dec	Op Table:	Op Table:
Energy Switch: Energy	Ref Node:	Ref Node:
	Manning's N: 0.0000	Manning's N: 0.0000

Comment:

Weir Link: L-0240W	
Scenario: Existing	Bottom Clip
From Node: Basin 4	Default: 0.00 ft
To Node: NZA-0080	Op Table:
Link Count: 26	Ref Node:
Flow Direction: Both	Top Clip

Damping: 0.0000 ft
 Weir Type: Horizontal
 Geometry Type: Rectangular
 Invert: 11.78 ft
 Control Elevation: 11.78 ft
 Max Depth: 0.14 ft
 Max Width: 0.92 ft
 Fillet: 0.00 ft

Default: 0.00 ft
 Op Table:
 Ref Node:
 Discharge Coefficients
 Weir Default: 2.800
 Weir Table:
 Orifice Default: 0.600
 Orifice Table:

Comment:

Pipe Link: L-0250P

Scenario: Existing
 From Node: NZA-0070
 To Node: NZA-0090
 Link Count: 1
 Flow Direction: Both
 Damping: 0.0000 ft
 Length: 173.25 ft
 FHWA Code: 0
 Entr Loss Coef: 0.50
 Exit Loss Coef: 1.00
 Bend Loss Coef: 0.00
 Bend Location: 0.00 dec
 Energy Switch: Energy

Upstream

Invert: 6.52 ft
 Manning's N: 0.0240
 Geometry: Circular
 Max Depth: 1.50 ft
 Bottom Clip
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000
 Top Clip
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000

Downstream

Invert: 6.02 ft
 Manning's N: 0.0240
 Geometry: Circular
 Max Depth: 1.50 ft
 Bottom Clip
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000
 Top Clip
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000

Comment:

Weir Link: L-0280W

Scenario: Existing
 From Node: Basin 2
 To Node: NZA-0090
 Link Count: 26
 Flow Direction: Both
 Damping: 0.0000 ft
 Weir Type: Horizontal
 Geometry Type: Rectangular
 Invert: 11.40 ft
 Control Elevation: 11.40 ft
 Max Depth: 0.14 ft
 Max Width: 0.92 ft
 Fillet: 0.00 ft

Bottom Clip
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Top Clip
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Discharge Coefficients
 Weir Default: 2.800
 Weir Table:
 Orifice Default: 0.600
 Orifice Table:

Comment:

Weir Link: L-0290W

Scenario:	Existing	Bottom Clip
From Node:	Basin 2	Default: 0.00 ft
To Node:	NZA-0070	Op Table:
Link Count:	26	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Horizontal	Op Table:
Geometry Type:	Rectangular	Ref Node:
Invert:	11.44 ft	Discharge Coefficients
Control Elevation:	11.44 ft	Weir Default: 2.800
Max Depth:	0.14 ft	Weir Table:
Max Width:	0.92 ft	Orifice Default: 0.600
Fillet:	0.00 ft	Orifice Table:

Comment:

Weir Link: L-0300W

Scenario:	Existing	Bottom Clip
From Node:	Basin 3	Default: 0.00 ft
To Node:	Basin 4	Op Table:
Link Count:	1	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Broad Crested Vertical	Op Table:
Geometry Type:	Irregular	Ref Node:
Invert:	12.14 ft	Discharge Coefficients
Control Elevation:	12.14 ft	Weir Default: 2.800
Cross Section:	X-0240W	Weir Table:
		Orifice Default: 0.600
		Orifice Table:

Comment:

Weir Link: L-0310W

Scenario:	Existing	Bottom Clip
From Node:	Basin 2	Default: 0.00 ft
To Node:	Basin 4	Op Table:
Link Count:	1	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Broad Crested Vertical	Op Table:
Geometry Type:	Irregular	Ref Node:
Invert:	12.54 ft	Discharge Coefficients
Control Elevation:	12.54 ft	Weir Default: 2.800
Cross Section:	X-0250W	Weir Table:
		Orifice Default: 0.600

Orifice Table:

Comment:

Weir Link: L-0320W

Scenario:	Existing	Bottom Clip
From Node:	Basin 2	Default: 0.00 ft
To Node:	Basin 1	Op Table:
Link Count:	1	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Broad Crested Vertical	Op Table:
Geometry Type:	Irregular	Ref Node:
Invert:	11.59 ft	Discharge Coefficients
Control Elevation:	11.59 ft	Weir Default: 2.800
Cross Section:	X-0260W	Weir Table:
		Orifice Default: 0.600
		Orifice Table:

Comment:

Weir Link: L-0330W

Scenario:	Existing	Bottom Clip
From Node:	Basin 2	Default: 0.00 ft
To Node:	Basin 3	Op Table:
Link Count:	1	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Broad Crested Vertical	Op Table:
Geometry Type:	Irregular	Ref Node:
Invert:	12.23 ft	Discharge Coefficients
Control Elevation:	12.23 ft	Weir Default: 2.800
Cross Section:	X-0230W	Weir Table:
		Orifice Default: 0.600
		Orifice Table:

Comment:

Pipe Link: L-0030P

	Upstream	Downstream
Scenario:	Proposed	
From Node:	NZA-0040	
To Node:	NZA-0050	
Link Count:	1	
Flow Direction:	Both	
	Invert: 5.20 ft	Invert: 5.07 ft
	Manning's N: 0.0240	Manning's N: 0.0240
	Geometry: Circular	Geometry: Circular
	Max Depth: 1.50 ft	Max Depth: 1.50 ft
	Bottom Clip	

Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	72.29 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0.50	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	1.00	Top Clip			
Bend Loss Coef:	0.00	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: L-0040P		Upstream	Downstream
Scenario:	Proposed	Invert: 4.97 ft	Invert: 3.44 ft
From Node:	NZA-0050	Manning's N: 0.0240	Manning's N: 0.0240
To Node:	Basin 3	Geometry: Circular	Geometry: Circular
Link Count:	1	Max Depth: 1.25 ft	Max Depth: 1.25 ft
Flow Direction:	Both	Bottom Clip	
Damping:	0.0000 ft	Default:	0.00 ft
Length:	36.33 ft	Op Table:	
FHWA Code:	0	Ref Node:	
Entr Loss Coef:	0.50	Manning's N:	0.0000
Exit Loss Coef:	1.00	Top Clip	
Bend Loss Coef:	0.00	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:	
Energy Switch:	Energy	Ref Node:	
		Manning's N:	0.0000

Comment:

Weir Link: L-0050W			
Scenario:	Proposed	Bottom Clip	
From Node:	Basin 3	Default:	0.00 ft
To Node:	NZA-0050	Op Table:	
Link Count:	26	Ref Node:	
Flow Direction:	Both	Top Clip	
Damping:	0.0000 ft	Default:	0.00 ft
Weir Type:	Horizontal	Op Table:	
Geometry Type:	Rectangular	Ref Node:	
Invert:	11.87 ft	Discharge Coefficients	
Control Elevation:	11.87 ft	Weir Default:	2.800
Max Depth:	0.14 ft	Weir Table:	
Max Width:	0.92 ft	Orifice Default:	0.600
Fillet:	0.00 ft	Orifice Table:	

Comment:

Weir Link: L-0060W

Scenario:	Proposed	Bottom Clip
From Node:	Basin 1	Default: 0.00 ft
To Node:	NZA-0040	Op Table:
Link Count:	26	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Horizontal	Op Table:
Geometry Type:	Rectangular	Ref Node:
Invert:	10.20 ft	Discharge Coefficients
Control Elevation:	10.20 ft	Weir Default: 2.800
Max Depth:	0.14 ft	Weir Table:
Max Width:	0.92 ft	Orifice Default: 0.600
Fillet:	0.00 ft	Orifice Table:

Comment:

Weir Link: L-0070W

Scenario:	Proposed	Bottom Clip
From Node:	Basin 1	Default: 0.00 ft
To Node:	Basin 3	Op Table:
Link Count:	1	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Broad Crested Vertical	Op Table:
Geometry Type:	Irregular	Ref Node:
Invert:	12.46 ft	Discharge Coefficients
Control Elevation:	12.46 ft	Weir Default: 2.800
Cross Section:	X-0180W	Weir Table:
		Orifice Default: 0.600
		Orifice Table:

Comment:

Weir Link: L-0090W

Scenario:	Proposed	Bottom Clip
From Node:	Basin 2	Default: 0.00 ft
To Node:	Basin 3	Op Table:
Link Count:	1	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Broad Crested Vertical	Op Table:
Geometry Type:	Irregular	Ref Node:
Invert:	12.54 ft	Discharge Coefficients
Control Elevation:	12.54 ft	Weir Default: 2.800
Cross Section:	X-0210W	Weir Table:
		Orifice Default: 0.600

Orifice Table:

Comment:

Pipe Link: L-0150P	Upstream	Downstream
Scenario: Proposed	Invert: 0.50 ft	Invert: 0.13 ft
From Node: CB-02	Manning's N: 0.0240	Manning's N: 0.0240
To Node: NZA-0050	Geometry: Circular	Geometry: Circular
Link Count: 1	Max Depth: 2.00 ft	Max Depth: 2.00 ft
Flow Direction: Both	Bottom Clip	
Damping: 0.0000 ft	Default: 0.00 ft	Default: 0.00 ft
Length: 175.39 ft	Op Table:	Op Table:
FHWA Code: 0	Ref Node:	Ref Node:
Entr Loss Coef: 0.50	Manning's N: 0.0000	Manning's N: 0.0000
Exit Loss Coef: 1.00	Top Clip	
Bend Loss Coef: 0.00	Default: 0.00 ft	Default: 0.00 ft
Bend Location: 0.00 dec	Op Table:	Op Table:
Energy Switch: Energy	Ref Node:	Ref Node:
	Manning's N: 0.0000	Manning's N: 0.0000

Comment:

Weir Link: L-0160W		
Scenario:	Proposed	Bottom Clip
From Node:	Basin 2	Default: 0.00 ft
To Node:	CI-01	Op Table:
Link Count:	26	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Horizontal	Op Table:
Geometry Type:	Rectangular	Ref Node:
Invert:	11.56 ft	Discharge Coefficients
Control Elevation:	11.56 ft	Weir Default: 2.800
Max Depth:	0.14 ft	Weir Table:
Max Width:	0.92 ft	Orifice Default: 0.600
Fillet:	0.00 ft	Orifice Table:

Comment:

Drop Structure Link: L-0200DS	Upstream Pipe	Downstream Pipe
Scenario: Proposed	Invert: 5.74 ft	Invert: 5.22 ft
From Node: Basin 1	Manning's N: 0.0120	Manning's N: 0.0120
To Node: NZA-0040	Geometry: Circular	Geometry: Circular
Link Count: 1	Max Depth: 1.50 ft	Max Depth: 1.50 ft

Flow Direction:	Both	Bottom Clip	
Solution:	Combine	Default: 0.00 ft	Default: 0.00 ft
Increments:	0	Op Table:	Op Table:
Pipe Count:	1	Ref Node:	Ref Node:
Damping:	0.0000 ft	Manning's N: 0.0000	Manning's N: 0.0000
Length:	227.27 ft	Top Clip	
FHWA Code:	0	Default: 0.00 ft	Default: 0.00 ft
Entr Loss Coef:	0.50	Op Table:	Op Table:
Exit Loss Coef:	1.00	Ref Node:	Ref Node:
Bend Loss Coef:	0.00	Manning's N: 0.0000	Manning's N: 0.0000
Bend Location:	0.00 dec		
Energy Switch:	Energy		
Pipe Comment:			

Weir Component			
Weir:	1	Bottom Clip	
Weir Count:	26	Default:	0.00 ft
Weir Flow Direction:	Both	Op Table:	
Damping:	0.0000 ft	Ref Node:	
Weir Type:	Horizontal	Top Clip	
Geometry Type:	Rectangular	Default:	0.00 ft
Invert:	9.64 ft	Op Table:	
Control Elevation:	9.64 ft	Ref Node:	
Max Depth:	0.14 ft	Discharge Coefficients	
Max Width:	0.92 ft	Weir Default:	3.200
Fillet:	0.00 ft	Weir Table:	
		Orifice Default:	0.600
		Orifice Table:	
Weir Comment:			

Drop Structure Comment:

Drop Structure Link: L-0210DS		Upstream Pipe	Downstream Pipe
Scenario:	Proposed	Invert: 6.61 ft	Invert: 6.61 ft
From Node:	Basin 2	Manning's N: 0.0240	Manning's N: 0.0240
To Node:	CI-01	Geometry: Circular	Geometry: Circular
Link Count:	1	Max Depth: 1.25 ft	Max Depth: 1.25 ft
Flow Direction:	Both	Bottom Clip	
Solution:	Combine	Default: 0.00 ft	Default: 0.00 ft
Increments:	0	Op Table:	Op Table:
Pipe Count:	1	Ref Node:	Ref Node:
Damping:	0.0000 ft	Manning's N: 0.0000	Manning's N: 0.0000
Length:	18.54 ft	Top Clip	
FHWA Code:	0	Default: 0.00 ft	Default: 0.00 ft
Entr Loss Coef:	0.50	Op Table:	Op Table:
Exit Loss Coef:	1.00	Ref Node:	Ref Node:
Bend Loss Coef:	0.00	Manning's N: 0.0000	Manning's N: 0.0000

Bend Location: 0.00 dec

Energy Switch: Energy

Pipe Comment:

Weir Component	
Weir: 1	Bottom Clip
Weir Count: 26	Default: 0.00 ft
Weir Flow Direction: Both	Op Table:
Damping: 0.0000	Ref Node:
Weir Type: Horizontal	Top Clip
Geometry Type: Rectangular	Default: 0.00 ft
Invert: 11.56 ft	Op Table:
Control Elevation: 11.56 ft	Ref Node:
Max Depth: 0.14 ft	Discharge Coefficients
Max Width: 0.92 ft	Weir Default: 3.200
Fillet: 0.00 ft	Weir Table:
	Orifice Default: 0.600
	Orifice Table:

Weir Comment:

Drop Structure Comment:

Drop Structure Link: L-0220DS		Upstream Pipe	Downstream Pipe
Scenario: Proposed		Invert: 5.55 ft	Invert: 4.83 ft
From Node: Basin 4		Manning's N: 0.0120	Manning's N: 0.0120
To Node: NZA-0080		Geometry: Circular	Geometry: Circular
Link Count: 1		Max Depth: 1.25 ft	Max Depth: 1.25 ft
Flow Direction: Both		Bottom Clip	
Solution: Combine		Default: 0.00 ft	Default: 0.00 ft
Increments: 0		Op Table:	Op Table:
Pipe Count: 1		Ref Node:	Ref Node:
Damping: 0.0000		Manning's N: 0.0000	Manning's N: 0.0000
Length: 34.07 ft		Top Clip	
FHWA Code: 0		Default: 0.00 ft	Default: 0.00 ft
Entr Loss Coef: 0.50		Op Table:	Op Table:
Exit Loss Coef: 1.00		Ref Node:	Ref Node:
Bend Loss Coef: 0.00		Manning's N: 0.0000	Manning's N: 0.0000
Bend Location: 0.00 dec			
Energy Switch: Energy			
Pipe Comment:			

Weir Component	
Weir: 1	Bottom Clip
Weir Count: 26	Default: 0.00 ft
Weir Flow Direction: Both	Op Table:
Damping: 0.0000	Ref Node:
Weir Type: Horizontal	Top Clip

Geometry Type: Rectangular
 Invert: 11.93 ft
 Control Elevation: 11.93 ft
 Max Depth: 0.14 ft
 Max Width: 0.92 ft
 Fillet: 0.00 ft

Default: 0.00 ft
 Op Table:
 Ref Node:
 Discharge Coefficients
 Weir Default: 3.200
 Weir Table:
 Orifice Default: 0.600
 Orifice Table:

Weir Comment:

Drop Structure Comment:

Pipe Link: L-0230P

Scenario: Proposed
 From Node: NZA-0080
 To Node: Basin 3
 Link Count: 1
 Flow Direction: Both
 Damping: 0.0000 ft
 Length: 43.28 ft
 FHWA Code: 0
 Entr Loss Coef: 0.50
 Exit Loss Coef: 1.00
 Bend Loss Coef: 0.00
 Bend Location: 0.00 dec
 Energy Switch: Energy

Upstream

Invert: 5.24 ft
 Manning's N: 0.0120
 Geometry: Circular
 Max Depth: 1.25 ft
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000

Downstream

Invert: 3.65 ft
 Manning's N: 0.0120
 Geometry: Circular
 Max Depth: 1.25 ft
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000

Bottom Clip

Top Clip

Comment:

Weir Link: L-0240W

Scenario: Proposed
 From Node: Basin 4
 To Node: NZA-0080
 Link Count: 26
 Flow Direction: Both
 Damping: 0.0000 ft
 Weir Type: Horizontal
 Geometry Type: Rectangular
 Invert: 11.78 ft
 Control Elevation: 11.78 ft
 Max Depth: 0.14 ft
 Max Width: 0.92 ft
 Fillet: 0.00 ft

Bottom Clip

Default: 0.00 ft
 Op Table:
 Ref Node:

Top Clip

Default: 0.00 ft
 Op Table:
 Ref Node:

Discharge Coefficients

Weir Default: 2.800
 Weir Table:
 Orifice Default: 0.600
 Orifice Table:

Comment:

French Drain Link: L-0380FD

Pipe Data

Scenario:	Proposed	Damping:	0.0000 ft
From Node:	MH-05	FHWA Code:	1
To Node:	MH-06	Entr Loss Coef:	0.50
Link Count:	1	Exit Loss Coef:	0.10
Flow Direction:	Both	Bend Loss Coef:	0.00
OF Region:		Bend Location:	0.00 dec
GW Region:		Energy Switch:	Energy
Mesh Scaling Factor:	1.0	Pipe Length:	186.00 ft
Trench Length:	153.00 ft	Pipe Invert:	6.00 ft
Trench Width:	6.00 ft	Pipe Invert:	6.00 ft
Trench Height:	4.00 ft	Manning's N:	0.0120
Trench Depth Below Invert:	1.50 ft	Geometry Type:	Circular
Trench Gravel Porosity:	0.500	Pipe Max Depth:	1.50 ft

Comment:

Percolation Link: L-0410PERC

Scenario:	Proposed	Surface Area Option:	User Specified
From Node:	MH-05	Bottom Elevation:	5.44 ft
To Node:	NTZ-0190	Surface Area:	0.0202 ac
Link Count:	1	Vertical Flow Termination:	Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1:	153.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2:	153.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3:	153.00 ft
Annual Recharge Rate:	0 ipy	Distance P1 to P2:	5.00 ft
Horizontal Conductivity:	3.000 fpd	Distance P2 to P3:	5.00 ft
Vertical Conductivity:	1.500 fpd	# of Cells P1 to P2:	10
Fillable Porosity:	0.250	# of Cells P2 to P3:	15
Layer Thickness:	0.00 ft		

Comment:

Percolation Link: L-0420PERC

Scenario:	Proposed	Surface Area Option:	User Specified
From Node:	NTZ-0190	Bottom Elevation:	5.44 ft
To Node:	MH-06	Surface Area:	0.0202 ac
Link Count:	1	Vertical Flow Termination:	Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1:	153.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2:	153.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3:	153.00 ft
Annual Recharge Rate:	0 ipy	Distance P1 to P2:	5.00 ft
Horizontal Conductivity:	3.000 fpd	Distance P2 to P3:	5.00 ft
Vertical Conductivity:	1.500 fpd	# of Cells P1 to P2:	10
Fillable Porosity:	0.250	# of Cells P2 to P3:	15
Layer Thickness:	0.00 ft		

Comment:

Weir Link: L-0440W

Scenario: Proposed
 From Node: Basin 3
 To Node: Basin 4
 Link Count: 1
 Flow Direction: Both
 Damping: 0.0000 ft
 Weir Type: Broad Crested Vertical
 Geometry Type: Irregular
 Invert: 12.14 ft
 Control Elevation: 12.14 ft
 Cross Section: X-0200W

Bottom Clip

Default: 0.00 ft
 Op Table:
 Ref Node:

Top Clip

Default: 0.00 ft
 Op Table:
 Ref Node:

Discharge Coefficients

Weir Default: 2.800
 Weir Table:
 Orifice Default: 0.600
 Orifice Table:

Comment:

Weir Link: L-0450W

Scenario: Proposed
 From Node: Basin 2
 To Node: Basin 1
 Link Count: 1
 Flow Direction: Both
 Damping: 0.0000 ft
 Weir Type: Broad Crested Vertical
 Geometry Type: Irregular
 Invert: 11.59 ft
 Control Elevation: 11.59 ft
 Cross Section: X-0220W

Bottom Clip

Default: 0.00 ft
 Op Table:
 Ref Node:

Top Clip

Default: 0.00 ft
 Op Table:
 Ref Node:

Discharge Coefficients

Weir Default: 2.800
 Weir Table:
 Orifice Default: 0.600
 Orifice Table:

Comment:

Weir Link: L-0460W

Scenario: Proposed
 From Node: Basin 2
 To Node: Basin 4
 Link Count: 1
 Flow Direction: Both
 Damping: 0.0000 ft
 Weir Type: Broad Crested Vertical
 Geometry Type: Irregular
 Invert: 12.54 ft
 Control Elevation: 12.54 ft
 Cross Section: X-0210W

Bottom Clip

Default: 0.00 ft
 Op Table:
 Ref Node:

Top Clip

Default: 0.00 ft
 Op Table:
 Ref Node:

Discharge Coefficients

Weir Default: 2.800
 Weir Table:
 Orifice Default: 0.600

Orifice Table:

Comment:

Weir Link: L-0470W

Scenario:	Proposed	Bottom Clip
From Node:	Basin 2	Default: 0.00 ft
To Node:	CB-01	Op Table:
Link Count:	26	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Horizontal	Op Table:
Geometry Type:	Rectangular	Ref Node:
Invert:	11.44 ft	Discharge Coefficients
Control Elevation:	11.44 ft	Weir Default: 2.800
Max Depth:	0.14 ft	Weir Table:
Max Width:	0.92 ft	Orifice Default: 0.600
Fillet:	0.00 ft	Orifice Table:

Comment:

Weir Link: L-0480W

Scenario:	Proposed	Bottom Clip
From Node:	Basin 2	Default: 0.00 ft
To Node:	CB-02	Op Table:
Link Count:	26	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Horizontal	Op Table:
Geometry Type:	Rectangular	Ref Node:
Invert:	11.40 ft	Discharge Coefficients
Control Elevation:	11.40 ft	Weir Default: 2.800
Max Depth:	0.14 ft	Weir Table:
Max Width:	0.92 ft	Orifice Default: 0.600
Fillet:	0.00 ft	Orifice Table:

Comment:

Weir Link: L-0500W

Scenario:	Proposed	Bottom Clip
From Node:	Basin 4	Default: 0.00 ft
To Node:	NZA-0240	Op Table:
Link Count:	26	Ref Node:
Flow Direction:	Both	Top Clip

Damping: 0.0000 ft
 Weir Type: Horizontal
 Geometry Type: Rectangular
 Invert: 11.85 ft
 Control Elevation: 11.85 ft
 Max Depth: 0.14 ft
 Max Width: 0.92 ft
 Fillet: 0.00 ft

Default: 0.00 ft
 Op Table:
 Ref Node:
 Discharge Coefficients
 Weir Default: 2.800
 Weir Table:
 Orifice Default: 0.600
 Orifice Table:

Comment:

Pipe Link: L-0510P

Scenario: Proposed
 From Node: MH-05
 To Node: NZA-0240
 Link Count: 1
 Flow Direction: Both
 Damping: 0.0000 ft
 Length: 46.00 ft
 FHWA Code: 0
 Entr Loss Coef: 0.50
 Exit Loss Coef: 1.00
 Bend Loss Coef: 0.00
 Bend Location: 0.00 dec
 Energy Switch: Energy

Upstream

Invert: 6.90 ft
 Manning's N: 0.0240
 Geometry: Circular
 Max Depth: 1.50 ft
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000

Downstream

Invert: 6.90 ft
 Manning's N: 0.0240
 Geometry: Circular
 Max Depth: 1.50 ft
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000

Comment:

Pipe Link: L-0530P

Scenario: Proposed
 From Node: MH-06
 To Node: Basin 3
 Link Count: 1
 Flow Direction: Positive
 Damping: 0.0000 ft
 Length: 76.79 ft
 FHWA Code: 0
 Entr Loss Coef: 0.50
 Exit Loss Coef: 1.00
 Bend Loss Coef: 0.00
 Bend Location: 0.00 dec
 Energy Switch: Energy

Upstream

Invert: 9.00 ft
 Manning's N: 0.0240
 Geometry: Circular
 Max Depth: 1.50 ft
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000

Downstream

Invert: 9.00 ft
 Manning's N: 0.0240
 Geometry: Circular
 Max Depth: 1.50 ft
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000
 Default: 0.00 ft
 Op Table:
 Ref Node:
 Manning's N: 0.0000

Comment:

Weir Link: L-0540W

Scenario:	Proposed	Bottom Clip
From Node:	Basin 4	Default: 0.00 ft
To Node:	CB-04	Op Table:
Link Count:	26	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Horizontal	Op Table:
Geometry Type:	Rectangular	Ref Node:
Invert:	11.85 ft	Discharge Coefficients
Control Elevation:	11.85 ft	Weir Default: 2.800
Max Depth:	0.14 ft	Weir Table:
Max Width:	0.92 ft	Orifice Default: 0.600
Fillet:	0.00 ft	Orifice Table:

Comment:

Weir Link: L-0550W

Scenario:	Proposed	Bottom Clip
From Node:	Basin 4	Default: 0.00 ft
To Node:	CB-05	Op Table:
Link Count:	26	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Horizontal	Op Table:
Geometry Type:	Rectangular	Ref Node:
Invert:	11.85 ft	Discharge Coefficients
Control Elevation:	11.85 ft	Weir Default: 2.800
Max Depth:	0.14 ft	Weir Table:
Max Width:	0.92 ft	Orifice Default: 0.600
Fillet:	0.00 ft	Orifice Table:

Comment:

Weir Link: L-0560W

Scenario:	Proposed	Bottom Clip
From Node:	Basin 4	Default: 0.00 ft
To Node:	CB-06	Op Table:
Link Count:	26	Ref Node:
Flow Direction:	Both	Top Clip
Damping:	0.0000 ft	Default: 0.00 ft
Weir Type:	Horizontal	Op Table:
Geometry Type:	Rectangular	Ref Node:
Invert:	11.85 ft	Discharge Coefficients
Control Elevation:	11.85 ft	Weir Default: 2.800
Max Depth:	0.14 ft	Weir Table:
Max Width:	0.92 ft	Orifice Default: 0.600

Fillet: 0.00 ft

Orifice Table:

Comment:

French Drain Link: L-0570FD

Pipe Data

Scenario:	Proposed	Damping:	0.0000 ft
From Node:	CB-04	FHWA Code:	1
To Node:	CB-05	Entr Loss Coef:	0.50
Link Count:	1	Exit Loss Coef:	0.10
Flow Direction:	Both	Bend Loss Coef:	0.00
OF Region:		Bend Location:	0.00 dec
GW Region:		Energy Switch:	Energy
Mesh Scaling Factor:	1.0	Pipe Length:	157.00 ft
Trench Length:	120.00 ft	Pipe Invert:	6.99 ft
Trench Width:	6.00 ft	Pipe Invert:	6.56 ft
Trench Height:	4.00 ft	Manning's N:	0.0120
Trench Depth Below Invert:	1.50 ft	Geometry Type:	Circular
Trench Gravel Porosity:	0.500	Pipe Max Depth:	1.50 ft

Comment:

French Drain Link: L-0580FD

Pipe Data

Scenario:	Proposed	Damping:	0.0000 ft
From Node:	CB-05	FHWA Code:	1
To Node:	CB-06	Entr Loss Coef:	0.50
Link Count:	1	Exit Loss Coef:	0.10
Flow Direction:	Both	Bend Loss Coef:	0.00
OF Region:		Bend Location:	0.00 dec
GW Region:		Energy Switch:	Energy
Mesh Scaling Factor:	1.0	Pipe Length:	302.00 ft
Trench Length:	243.00 ft	Pipe Invert:	6.50 ft
Trench Width:	6.00 ft	Pipe Invert:	6.00 ft
Trench Height:	4.00 ft	Manning's N:	0.0120
Trench Depth Below Invert:	1.50 ft	Geometry Type:	Circular
Trench Gravel Porosity:	0.500	Pipe Max Depth:	1.50 ft

Comment:

Pipe Link: L-0610P

Upstream

Downstream

Scenario:	Proposed	Invert:	5.37 ft	Invert:	5.55 ft
From Node:	CB-06	Manning's N:	0.0240	Manning's N:	0.0240
To Node:	NZA-0280	Geometry:	Circular	Geometry:	Circular
Link Count:	1	Max Depth:	2.00 ft	Max Depth:	2.00 ft
Flow Direction:	Both	Bottom Clip			

Damping:	0.0000 ft	Default:	0.00 ft	Default:	0.00 ft
Length:	31.00 ft	Op Table:		Op Table:	
FHWA Code:	0	Ref Node:		Ref Node:	
Entr Loss Coef:	0.50	Manning's N:	0.0000	Manning's N:	0.0000
Exit Loss Coef:	1.00	Top Clip			
Bend Loss Coef:	0.00	Default:	0.00 ft	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:		Op Table:	
Energy Switch:	Energy	Ref Node:		Ref Node:	
		Manning's N:	0.0000	Manning's N:	0.0000

Comment:

Pipe Link: L-0620P		Upstream	Downstream
Scenario:	Proposed	Invert: 5.23 ft	Invert: 2.87 ft
From Node:	CB-06	Manning's N: 0.0240	Manning's N: 0.0240
To Node:	Basin 3	Geometry: Circular	Geometry: Circular
Link Count:	1	Max Depth: 2.00 ft	Max Depth: 2.00 ft
Flow Direction:	Both	Bottom Clip	
Damping:	0.0000 ft	Default:	0.00 ft
Length:	42.00 ft	Op Table:	
FHWA Code:	0	Ref Node:	
Entr Loss Coef:	0.50	Manning's N:	0.0000
Exit Loss Coef:	1.00	Top Clip	
Bend Loss Coef:	0.00	Default:	0.00 ft
Bend Location:	0.00 dec	Op Table:	
Energy Switch:	Energy	Ref Node:	
		Manning's N:	0.0000

Comment:

Percolation Link: L-0630PERC			
Scenario:	Proposed	Surface Area Option:	User Specified
From Node:	CB-04	Bottom Elevation:	5.44 ft
To Node:	NTZ-0290	Surface Area:	0.0165 ac
Link Count:	1	Vertical Flow Termination:	Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1:	120.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2:	120.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3:	120.00 ft
Annual Recharge Rate:	0 ipy	Distance P1 to P2:	5.00 ft
Horizontal Conductivity:	3.000 fpd	Distance P2 to P3:	5.00 ft
Vertical Conductivity:	1.500 fpd	# of Cells P1 to P2:	10
Fillable Porosity:	0.250	# of Cells P2 to P3:	15
Layer Thickness:	0.00 ft		

Comment:

Percolation Link: L-0640PERC

Scenario:	Proposed	Surface Area Option:	User Specified
From Node:	NTZ-0290	Bottom Elevation:	5.44 ft
To Node:	CB-05	Surface Area:	0.0165 ac
Link Count:	1	Vertical Flow Termination:	Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1:	120.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2:	120.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3:	120.00 ft
Annual Recharge Rate:	0 ipy	Distance P1 to P2:	5.00 ft
Horizontal Conductivity:	3.000 fpd	Distance P2 to P3:	5.00 ft
Vertical Conductivity:	1.500 fpd	# of Cells P1 to P2:	10
Fillable Porosity:	0.250	# of Cells P2 to P3:	15
Layer Thickness:	0.00 ft		

Comment:

Percolation Link: L-0650PERC

Scenario:	Proposed	Surface Area Option:	User Specified
From Node:	CB-05	Bottom Elevation:	5.44 ft
To Node:	NTZ-0300	Surface Area:	0.0334 ac
Link Count:	1	Vertical Flow Termination:	Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1:	243.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2:	243.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3:	243.00 ft
Annual Recharge Rate:	0 ipy	Distance P1 to P2:	5.00 ft
Horizontal Conductivity:	3.000 fpd	Distance P2 to P3:	5.00 ft
Vertical Conductivity:	1.500 fpd	# of Cells P1 to P2:	10
Fillable Porosity:	0.250	# of Cells P2 to P3:	15
Layer Thickness:	0.00 ft		

Comment:

Percolation Link: L-0660PERC

Scenario:	Proposed	Surface Area Option:	User Specified
From Node:	NTZ-0300	Bottom Elevation:	5.44 ft
To Node:	CB-06	Surface Area:	0.0334 ac
Link Count:	1	Vertical Flow Termination:	Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1:	243.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2:	243.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3:	243.00 ft
Annual Recharge Rate:	0 ipy	Distance P1 to P2:	5.00 ft
Horizontal Conductivity:	3.000 fpd	Distance P2 to P3:	5.00 ft
Vertical Conductivity:	1.500 fpd	# of Cells P1 to P2:	10
Fillable Porosity:	0.250	# of Cells P2 to P3:	15
Layer Thickness:	0.00 ft		

Comment:

Pipe Link: L-0670P		Upstream	Downstream
Scenario:	Proposed	Invert: 0.00 ft	Invert: 0.00 ft
From Node:	CI-01	Manning's N: 0.0240	Manning's N: 0.0240
To Node:	MH-01	Geometry: Circular	Geometry: Circular
Link Count:	1	Max Depth: 1.50 ft	Max Depth: 1.50 ft
Flow Direction:	Both	Bottom Clip	
Damping:	0.0000 ft	Default: 0.00 ft	Default: 0.00 ft
Length:	96.00 ft	Op Table:	Op Table:
FHWA Code:	0	Ref Node:	Ref Node:
Entr Loss Coef:	0.50	Manning's N: 0.0000	Manning's N: 0.0000
Exit Loss Coef:	1.00	Top Clip	
Bend Loss Coef:	0.00	Default: 0.00 ft	Default: 0.00 ft
Bend Location:	0.00 dec	Op Table:	Op Table:
Energy Switch:	Energy	Ref Node:	Ref Node:
		Manning's N: 0.0000	Manning's N: 0.0000
Comment:			

French Drain Link: L-0680FD		Pipe Data
Scenario:	Proposed	Damping: 0.0000 ft
From Node:	MH-01	FHWA Code: 1
To Node:	CB-01	Entr Loss Coef: 0.50
Link Count:	1	Exit Loss Coef: 0.10
Flow Direction:	Both	Bend Loss Coef: 0.00
OF Region:		Bend Location: 0.00 dec
GW Region:		Energy Switch: Energy
Mesh Scaling Factor:	1.0	Pipe Length: 62.00 ft
Trench Length:	47.00 ft	Pipe Invert: 7.50 ft
Trench Width:	6.00 ft	Pipe Invert: 7.50 ft
Trench Height:	4.00 ft	Manning's N: 0.0120
Trench Depth Below Invert:	1.50 ft	Geometry Type: Circular
Trench Gravel Porosity:	0.500	Pipe Max Depth: 1.50 ft
Comment:		

Percolation Link: L-0690PERC		
Scenario:	Proposed	Surface Area Option: User Specified
From Node:	MH-01	Bottom Elevation: 5.44 ft
To Node:	NTZ-0320	Surface Area: 0.0065 ac
Link Count:	1	Vertical Flow Termination: Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1: 47.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2: 47.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3: 47.00 ft
Annual Recharge Rate:	0 ipy	Distance P1 to P2: 5.00 ft
Horizontal Conductivity:	3.000 fpd	Distance P2 to P3: 5.00 ft
Vertical Conductivity:	1.500 fpd	# of Cells P1 to P2: 10
Fillable Porosity:	0.250	# of Cells P2 to P3: 15

Layer Thickness: 0.00 ft

Comment:

Percolation Link: L-0700PERC

Scenario:	Proposed	Surface Area Option:	User Specified
From Node:	NTZ-0320	Bottom Elevation:	5.44 ft
To Node:	CB-01	Surface Area:	0.0065 ac
Link Count:	1	Vertical Flow Termination:	Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1:	47.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2:	47.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3:	47.00 ft
Annual Recharge Rate:	0 ipy	Distance P1 to P2:	5.00 ft
Horizontal Conductivity:	3.000 fpd	Distance P2 to P3:	5.00 ft
Vertical Conductivity:	1.500 fpd	# of Cells P1 to P2:	10
Fillable Porosity:	0.250	# of Cells P2 to P3:	15
Layer Thickness:	0.00 ft		

Comment:

French Drain Link: L-0710FD

Pipe Data

Scenario:	Proposed	Damping:	0.0000 ft
From Node:	CB-01	FHWA Code:	1
To Node:	CB-02	Entr Loss Coef:	0.50
Link Count:	1	Exit Loss Coef:	0.10
Flow Direction:	Both	Bend Loss Coef:	0.00
OF Region:		Bend Location:	0.00 dec
GW Region:		Energy Switch:	Energy
Mesh Scaling Factor:	1.0	Pipe Length:	173.00 ft
Trench Length:	158.00 ft	Pipe Invert:	7.50 ft
Trench Width:	6.00 ft	Pipe Invert:	7.50 ft
Trench Height:	4.00 ft	Manning's N:	0.0120
Trench Depth Below Invert:	1.50 ft	Geometry Type:	Circular
Trench Gravel Porosity:	0.500	Pipe Max Depth:	1.50 ft

Comment:

Percolation Link: L-0720PERC

Scenario:	Proposed	Surface Area Option:	User Specified
From Node:	CB-01	Bottom Elevation:	5.44 ft
To Node:	NTZ-0340	Surface Area:	0.0230 ac
Link Count:	1	Vertical Flow Termination:	Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1:	173.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2:	173.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3:	173.00 ft

Annual Recharge Rate:	0 ipy	
Horizontal Conductivity:	3.000 fpd	Distance P1 to P2: 5.00 ft
Vertical Conductivity:	1.500 fpd	Distance P2 to P3: 5.00 ft
Fillable Porosity:	0.250	# of Cells P1 to P2: 10
Layer Thickness:	0.00 ft	# of Cells P2 to P3: 15

Comment:

Percolation Link: L-0730PERC

Scenario:	Proposed	Surface Area Option:	User Specified
From Node:	NTZ-0340	Bottom Elevation:	5.44 ft
To Node:	CB-02	Surface Area:	0.0230 ac
Link Count:	1	Vertical Flow Termination:	Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1:	173.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2:	173.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3:	173.00 ft
Annual Recharge Rate:	0 ipy	Distance P1 to P2:	5.00 ft
Horizontal Conductivity:	3.000 fpd	Distance P2 to P3:	5.00 ft
Vertical Conductivity:	1.500 fpd	# of Cells P1 to P2:	10
Fillable Porosity:	0.250	# of Cells P2 to P3:	15
Layer Thickness:	0.00 ft		

Comment:

French Drain Link: L-0740FD

Pipe Data

Scenario:	Proposed	Damping:	0.0000 ft
From Node:	MH-01	FHWA Code:	1
To Node:	MH-02	Entr Loss Coef:	0.50
Link Count:	1	Exit Loss Coef:	0.10
Flow Direction:	Both	Bend Loss Coef:	0.00
OF Region:		Bend Location:	0.00 dec
GW Region:		Energy Switch:	Energy
Mesh Scaling Factor:	1.0	Pipe Length:	58.00 ft
Trench Length:	44.00 ft	Pipe Invert:	7.50 ft
Trench Width:	6.00 ft	Pipe Invert:	7.50 ft
Trench Height:	4.00 ft	Manning's N:	0.0120
Trench Depth Below Invert:	1.50 ft	Geometry Type:	Circular
Trench Gravel Porosity:	0.500	Pipe Max Depth:	1.50 ft

Comment:

French Drain Link: L-0750FD

Pipe Data

Scenario:	Proposed	Damping:	0.0000 ft
From Node:	MH-02	FHWA Code:	1
To Node:	MH-03	Entr Loss Coef:	0.50

Link Count:	1	Pipe Data	
Flow Direction:	Both	Exit Loss Coef:	0.10
OF Region:		Bend Loss Coef:	0.00
GW Region:		Bend Location:	0.00 dec
Mesh Scaling Factor:	1.0	Energy Switch:	Energy
Trench Length:	36.00 ft	Pipe Length:	51.00 ft
Trench Width:	6.00 ft	Pipe Invert:	7.50 ft
Trench Height:	4.00 ft	Pipe Invert:	7.50 ft
Trench Depth Below Invert:	1.50 ft	Manning's N:	0.0120
Trench Gravel Porosity:	0.500	Geometry Type:	Circular
		Pipe Max Depth:	1.50 ft

Comment:

French Drain Link: L-0760FD		Pipe Data	
Scenario:	Proposed	Damping:	0.0000 ft
From Node:	MH-02	FHWA Code:	1
To Node:	CB-03	Entr Loss Coef:	0.50
Link Count:	1	Exit Loss Coef:	0.10
Flow Direction:	Both	Bend Loss Coef:	0.00
OF Region:		Bend Location:	0.00 dec
GW Region:		Energy Switch:	Energy
Mesh Scaling Factor:	1.0	Pipe Length:	31.00 ft
Trench Length:	16.00 ft	Pipe Invert:	7.50 ft
Trench Width:	6.00 ft	Pipe Invert:	7.50 ft
Trench Height:	4.00 ft	Manning's N:	0.0120
Trench Depth Below Invert:	1.50 ft	Geometry Type:	Circular
Trench Gravel Porosity:	0.500	Pipe Max Depth:	1.50 ft

Comment:

Percolation Link: L-0770PERC			
Scenario:	Proposed	Surface Area Option:	User Specified
From Node:	MH-01	Bottom Elevation:	5.44 ft
To Node:	NTZ-0380	Surface Area:	0.0060 ac
Link Count:	1	Vertical Flow Termination:	Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1:	44.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2:	44.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3:	44.00 ft
Annual Recharge Rate:	0 ipy	Distance P1 to P2:	5.00 ft
Horizontal Conductivity:	3.000 fpd	Distance P2 to P3:	5.00 ft
Vertical Conductivity:	1.500 fpd	# of Cells P1 to P2:	10
Fillable Porosity:	0.250	# of Cells P2 to P3:	15
Layer Thickness:	0.00 ft		

Comment:

Percolation Link: L-0780PERC

Scenario:	Proposed	Surface Area Option:	User Specified
From Node:	NTZ-0380	Bottom Elevation:	5.44 ft
To Node:	MH-02	Surface Area:	0.0060 ac
Link Count:	1	Vertical Flow Termination:	Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1:	44.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2:	44.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3:	44.00 ft
Annual Recharge Rate:	0 ipy	Distance P1 to P2:	5.00 ft
Horizontal Conductivity:	3.000 fpd	Distance P2 to P3:	5.00 ft
Vertical Conductivity:	1.500 fpd	# of Cells P1 to P2:	10
Fillable Porosity:	0.250	# of Cells P2 to P3:	15
Layer Thickness:	0.00 ft		

Comment:

Percolation Link: L-0790PERC

Scenario:	Proposed	Surface Area Option:	User Specified
From Node:	MH-02	Bottom Elevation:	5.44 ft
To Node:	NTZ-0390	Surface Area:	0.0042 ac
Link Count:	1	Vertical Flow Termination:	Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1:	31.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2:	31.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3:	31.00 ft
Annual Recharge Rate:	0 ipy	Distance P1 to P2:	5.00 ft
Horizontal Conductivity:	3.000 fpd	Distance P2 to P3:	5.00 ft
Vertical Conductivity:	1.500 fpd	# of Cells P1 to P2:	10
Fillable Porosity:	0.250	# of Cells P2 to P3:	15
Layer Thickness:	0.00 ft		

Comment:

Percolation Link: L-0800PERC

Scenario:	Proposed	Surface Area Option:	User Specified
From Node:	NTZ-0390	Bottom Elevation:	5.44 ft
To Node:	CB-03	Surface Area:	0.0042 ac
Link Count:	1	Vertical Flow Termination:	Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1:	31.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2:	31.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3:	31.00 ft
Annual Recharge Rate:	0 ipy	Distance P1 to P2:	5.00 ft
Horizontal Conductivity:	3.000 fpd	Distance P2 to P3:	5.00 ft
Vertical Conductivity:	1.500 fpd	# of Cells P1 to P2:	10
Fillable Porosity:	0.250	# of Cells P2 to P3:	15
Layer Thickness:	0.00 ft		

Comment:

Percolation Link: L-0810PERC

Scenario:	Proposed	Surface Area Option:	User Specified
From Node:	MH-02	Bottom Elevation:	5.44 ft
To Node:	NTZ-0400	Surface Area:	0.0049 ac
Link Count:	1	Vertical Flow Termination:	Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1:	36.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2:	36.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3:	36.00 ft
Annual Recharge Rate:	0 ipy	Distance P1 to P2:	5.00 ft
Horizontal Conductivity:	3.000 fpd	Distance P2 to P3:	5.00 ft
Vertical Conductivity:	1.500 fpd	# of Cells P1 to P2:	10
Fillable Porosity:	0.250	# of Cells P2 to P3:	15
Layer Thickness:	0.00 ft		

Comment:

Percolation Link: L-0820PERC

Scenario:	Proposed	Surface Area Option:	User Specified
From Node:	NTZ-0400	Bottom Elevation:	5.44 ft
To Node:	MH-03	Surface Area:	0.0049 ac
Link Count:	1	Vertical Flow Termination:	Horizontal Flow Algorithm
Flow Direction:	Both	Perimeter 1:	36.00 ft
Aquifer Base Elevation:	-46.00 ft	Perimeter 2:	36.00 ft
Water Table Elevation:	5.50 ft	Perimeter 3:	36.00 ft
Annual Recharge Rate:	0 ipy	Distance P1 to P2:	5.00 ft
Horizontal Conductivity:	3.000 fpd	Distance P2 to P3:	5.00 ft
Vertical Conductivity:	1.500 fpd	# of Cells P1 to P2:	10
Fillable Porosity:	0.250	# of Cells P2 to P3:	15
Layer Thickness:	0.00 ft		

Comment:

Weir Link: L-0830W

Scenario:	Proposed	Bottom Clip	
From Node:	Basin 4	Default:	0.00 ft
To Node:	NZA-0280	Op Table:	
Link Count:	26	Ref Node:	
Flow Direction:	Both	Top Clip	
Damping:	0.0000 ft	Default:	0.00 ft
Weir Type:	Horizontal	Op Table:	
Geometry Type:	Rectangular	Ref Node:	
Invert:	11.85 ft	Discharge Coefficients	
Control Elevation:	11.85 ft	Weir Default:	2.800
Max Depth:	0.14 ft	Weir Table:	
Max Width:	0.92 ft	Orifice Default:	0.600
Fillet:	0.00 ft	Orifice Table:	

Comment:

Simulation: 100yr-72hr

Scenario: Existing
 Run Date/Time: 5/13/2025 2:11:25 PM
 Program Version: ICPR4 4.07.04

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	96.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
 Reference ET Folder:
 Unit Hydrograph
 Folder:

Lookup Tables

Boundary Stage Set:
 Extern Hydrograph Set:
 Curve Number Set: Curve Number

Green-Ampt Set:
 Vertical Layers Set:
 Impervious Set: Impervious
 Roughness Set:
 Crop Coef Set:
 Fillable Porosity Set:

Conductivity Set:

Leakage Set:

Tolerances & Options

Time Marching:	SAOR	IA Recovery Time:	24.0000 hr
Max Iterations:	6	ET for Manual Basins:	False
Over-Relax Weight	0.5 dec		
Fact:			
dZ Tolerance:	0.0010 ft	Smp/Man Basin Rain	Global
		Opt:	
Max dZ:	1.0000 ft	OF Region Rain Opt:	Global
Link Optimizer Tol:	0.0001 ft	Rainfall Name:	~SFWMD-72
		Rainfall Amount:	18.00 in
Edge Length Option:	Automatic	Storm Duration:	72.0000 hr
Dflt Damping (2D):	0.0050 ft	Dflt Damping (1D):	0.0050 ft
Min Node Srf Area	100 ft2	Min Node Srf Area	100 ft2
(2D):		(1D):	
Energy Switch (2D):	Energy	Energy Switch (1D):	Energy

Comment:

Simulation: 25yr-72hr

Scenario: Existing
 Run Date/Time: 5/13/2025 2:12:51 PM
 Program Version: ICPR4 4.07.04

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	96.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Reference ET Folder:
Unit Hydrograph
Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Curve Number

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Impervious
Roughness Set:
Crop Coef Set:
Fillable Porosity Set:
Conductivity Set:
Leakage Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight: 0.5 dec
Fact:
dZ Tolerance: 0.0010 ft

Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

Edge Length Option: Automatic

Dflt Damping (2D): 0.0050 ft
Min Node Srf Area: 100 ft2
(2D):
Energy Switch (2D): Energy

IA Recovery Time: 24.0000 hr
ET for Manual Basins: False

Smp/Man Basin Rain: Global
Opt:
OF Region Rain Opt: Global
Rainfall Name: ~SFWMD-72
Rainfall Amount: 13.00 in
Storm Duration: 72.0000 hr

Dflt Damping (1D): 0.0050 ft
Min Node Srf Area: 100 ft2
(1D):
Energy Switch (1D): Energy

Comment:

Simulation: 5yr-24hr

Scenario: Existing
 Run Date/Time: 5/13/2025 2:14:18 PM
 Program Version: ICPR4 4.07.04

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	36.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
 Reference ET Folder:
 Unit Hydrograph
 Folder:

Lookup Tables

Boundary Stage Set:
 Extern Hydrograph Set:
 Curve Number Set: Curve Number

Green-Ampt Set:
 Vertical Layers Set:
 Impervious Set: Impervious
 Roughness Set:
 Crop Coef Set:
 Fillable Porosity Set:

Conductivity Set:

Leakage Set:

Tolerances & Options

Time Marching: SAOR	IA Recovery Time: 24.0000 hr
Max Iterations: 6	ET for Manual Basins: False
Over-Relax Weight 0.5 dec	
Fact:	
dZ Tolerance: 0.0010 ft	Smp/Man Basin Rain Global
	Opt:
Max dZ: 1.0000 ft	OF Region Rain Opt: Global
Link Optimizer Tol: 0.0001 ft	Rainfall Name: ~FLMOD
	Rainfall Amount: 7.08 in
Edge Length Option: Automatic	Storm Duration: 24.0000 hr
Dflt Damping (2D): 0.0050 ft	Dflt Damping (1D): 0.0050 ft
Min Node Srf Area 100 ft2	Min Node Srf Area 100 ft2
(2D):	(1D):
Energy Switch (2D): Energy	Energy Switch (1D): Energy

Comment:

Simulation: 100yr-72hr

Scenario: Proposed
 Run Date/Time: 5/13/2025 2:14:54 PM
 Program Version: ICPR4 4.07.04

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	96.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Reference ET Folder:
Unit Hydrograph
Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Curve Number

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Impervious
Roughness Set:
Crop Coef Set:
Fillable Porosity Set:
Conductivity Set:
Leakage Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight: 0.5 dec
Fact:
dZ Tolerance: 0.0010 ft

Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

Edge Length Option: Automatic

Dflt Damping (2D): 0.0050 ft
Min Node Srf Area: 100 ft2
(2D):
Energy Switch (2D): Energy

IA Recovery Time: 24.0000 hr
ET for Manual Basins: False

Smp/Man Basin Rain: Global
Opt:
OF Region Rain Opt: Global
Rainfall Name: ~SFWMD-72
Rainfall Amount: 18.00 in
Storm Duration: 72.0000 hr

Dflt Damping (1D): 0.0050 ft
Min Node Srf Area: 100 ft2
(1D):
Energy Switch (1D): Energy

Comment:

Simulation: 25yr-72hr

Scenario: Proposed
 Run Date/Time: 5/13/2025 2:17:38 PM
 Program Version: ICPR4 4.07.04

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	96.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
 Reference ET Folder:
 Unit Hydrograph
 Folder:

Lookup Tables

Boundary Stage Set:
 Extern Hydrograph Set:
 Curve Number Set: Curve Number

Green-Ampt Set:
 Vertical Layers Set:
 Impervious Set: Impervious
 Roughness Set:
 Crop Coef Set:
 Fillable Porosity Set:

Conductivity Set:

Leakage Set:

Tolerances & Options

Time Marching: SAOR	IA Recovery Time: 24.0000 hr
Max Iterations: 6	ET for Manual Basins: False
Over-Relax Weight 0.5 dec	
Fact:	
dZ Tolerance: 0.0010 ft	Smp/Man Basin Rain Global
	Opt:
Max dZ: 1.0000 ft	OF Region Rain Opt: Global
Link Optimizer Tol: 0.0001 ft	Rainfall Name: ~SFWMMD-72
	Rainfall Amount: 13.00 in
Edge Length Option: Automatic	Storm Duration: 72.0000 hr
Dflt Damping (2D): 0.0050 ft	Dflt Damping (1D): 0.0050 ft
Min Node Srf Area 100 ft2	Min Node Srf Area 100 ft2
(2D):	(1D):
Energy Switch (2D): Energy	Energy Switch (1D): Energy

Comment:

Simulation: 5yr-24hr

Scenario: Proposed
 Run Date/Time: 5/13/2025 2:20:42 PM
 Program Version: ICPR4 4.07.04

General

Run Mode: Normal

	Year	Month	Day	Hour [hr]
Start Time:	0	0	0	0.0000
End Time:	0	0	0	36.0000

	Hydrology [sec]	Surface Hydraulics [sec]	Groundwater [sec]
Min Calculation Time:	60.0000	0.1000	900.0000
Max Calculation Time:		30.0000	

Output Time Increments

Hydrology

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Surface Hydraulics

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	15.0000

Groundwater

Year	Month	Day	Hour [hr]	Time Increment [min]
0	0	0	0.0000	60.0000

Restart File

Save Restart: False

Resources & Lookup Tables

Resources

Rainfall Folder:
Reference ET Folder:
Unit Hydrograph
Folder:

Lookup Tables

Boundary Stage Set:
Extern Hydrograph Set:
Curve Number Set: Curve Number

Green-Ampt Set:
Vertical Layers Set:
Impervious Set: Impervious
Roughness Set:
Crop Coef Set:
Fillable Porosity Set:
Conductivity Set:
Leakage Set:

Tolerances & Options

Time Marching: SAOR
Max Iterations: 6
Over-Relax Weight: 0.5 dec
Fact:
dZ Tolerance: 0.0010 ft

Max dZ: 1.0000 ft
Link Optimizer Tol: 0.0001 ft

Edge Length Option: Automatic

Dflt Damping (2D): 0.0050 ft
Min Node Srf Area: 100 ft2
(2D):
Energy Switch (2D): Energy

IA Recovery Time: 24.0000 hr
ET for Manual Basins: False

Smp/Man Basin Rain: Global
Opt:
OF Region Rain Opt: Global
Rainfall Name: ~FLMOD
Rainfall Amount: 7.08 in
Storm Duration: 24.0000 hr

Dflt Damping (1D): 0.0050 ft
Min Node Srf Area: 100 ft2
(1D):
Energy Switch (1D): Energy

Comment:

Scenario	Sim	Node Name	Maximum Stage [ft]
Existing	100yr-72hr	Basin 1	12.36
Existing	100yr-72hr	Basin 2	12.36
Existing	100yr-72hr	Basin 3	12.16
Existing	100yr-72hr	Basin 4	12.35
Proposed	100yr-72hr	Basin 1	12.34
Proposed	100yr-72hr	Basin 2	12.34
Proposed	100yr-72hr	Basin 3	11.89
Proposed	100yr-72hr	Basin 4	11.97
Existing	25yr-72hr	Basin 1	11.95
Existing	25yr-72hr	Basin 2	11.95
Existing	25yr-72hr	Basin 3	10.73
Existing	25yr-72hr	Basin 4	12.03
Proposed	25yr-72hr	Basin 1	11.85
Proposed	25yr-72hr	Basin 2	11.93
Proposed	25yr-72hr	Basin 3	10.33
Proposed	25yr-72hr	Basin 4	11.93
Existing	5yr-24hr	Basin 1	11.41
Existing	5yr-24hr	Basin 2	11.81
Existing	5yr-24hr	Basin 3	7.93
Existing	5yr-24hr	Basin 4	11.90
Proposed	5yr-24hr	Basin 1	11.28
Proposed	5yr-24hr	Basin 2	11.52
Proposed	5yr-24hr	Basin 3	7.72
Proposed	5yr-24hr	Basin 4	11.91

Scenario	Sim	Link Name	Maximum Flow Rate [cfs]	Minimum Flow Rate [cfs]
Existing	100yr-72hr	L-0030P	3.65	-0.04
Existing	100yr-72hr	L-0040P	7.18	-0.02
Existing	100yr-72hr	L-0050W	0.03	-3.14
Existing	100yr-72hr	L-0060W	2.71	-0.03
Existing	100yr-72hr	L-0070W	0.00	0.00
Existing	100yr-72hr	L-0140P	0.26	-0.51
Existing	100yr-72hr	L-0150P	7.07	-0.02
Existing	100yr-72hr	L-0160W	0.43	-0.03
Existing	100yr-72hr	L-0190DS	8.00	0.00
Existing	100yr-72hr	L-0200DS	3.24	-0.02
Existing	100yr-72hr	L-0210DS	0.25	-0.02
Existing	100yr-72hr	L-0220DS	3.54	-0.02
Existing	100yr-72hr	L-0230P	10.71	-0.02
Existing	100yr-72hr	L-0240W	7.97	-0.03
Existing	100yr-72hr	L-0250P	1.37	-0.09
Existing	100yr-72hr	L-0280W	5.79	-0.03
Existing	100yr-72hr	L-0290W	2.39	-0.03
Existing	100yr-72hr	L-0300W	0.00	-0.62
Existing	100yr-72hr	L-0310W	0.00	0.00
Existing	100yr-72hr	L-0320W	13.71	-3.38
Existing	100yr-72hr	L-0330W	1.84	0.00
Proposed	100yr-72hr	L-0030P	4.25	0.00
Proposed	100yr-72hr	L-0040P	7.38	-0.23
Proposed	100yr-72hr	L-0050W	0.03	-2.73
Proposed	100yr-72hr	L-0060W	3.16	0.00
Proposed	100yr-72hr	L-0070W	0.00	0.00
Proposed	100yr-72hr	L-0090W	0.00	0.00
Proposed	100yr-72hr	L-0150P	7.13	-0.65
Proposed	100yr-72hr	L-0160W	1.15	0.00
Proposed	100yr-72hr	L-0200DS	3.76	0.00
Proposed	100yr-72hr	L-0210DS	0.80	0.00
Proposed	100yr-72hr	L-0220DS	1.23	0.00
Proposed	100yr-72hr	L-0230P	8.34	-0.21
Proposed	100yr-72hr	L-0240W	7.18	-0.21
Proposed	100yr-72hr	L-0380FD	4.34	-0.04
Proposed	100yr-72hr	L-0410PERC	0.73	0.00
Proposed	100yr-72hr	L-0420PERC	0.00	0.00
Proposed	100yr-72hr	L-0440W	0.00	0.00
Proposed	100yr-72hr	L-0450W	13.06	-2.92
Proposed	100yr-72hr	L-0460W	0.00	0.00
Proposed	100yr-72hr	L-0470W	4.60	-0.03
Proposed	100yr-72hr	L-0480W	5.92	-0.42

Scenario	Sim	Link Name	Maximum Flow Rate [cfs]	Minimum Flow Rate [cfs]
Proposed	100yr-72hr	L-0500W	5.35	0.00
Proposed	100yr-72hr	L-0510P	0.00	-5.07
Proposed	100yr-72hr	L-0530P	4.34	0.00
Proposed	100yr-72hr	L-0540W	4.45	0.00
Proposed	100yr-72hr	L-0550W	5.16	0.00
Proposed	100yr-72hr	L-0560W	5.67	-0.28
Proposed	100yr-72hr	L-0570FD	3.76	-0.30
Proposed	100yr-72hr	L-0580FD	7.22	-0.54
Proposed	100yr-72hr	L-0610P	0.24	-5.63
Proposed	100yr-72hr	L-0620P	17.33	-0.76
Proposed	100yr-72hr	L-0630PERC	0.39	-0.01
Proposed	100yr-72hr	L-0640PERC	0.00	0.00
Proposed	100yr-72hr	L-0650PERC	0.74	0.00
Proposed	100yr-72hr	L-0660PERC	0.00	0.00
Proposed	100yr-72hr	L-0670P	1.05	-0.41
Proposed	100yr-72hr	L-0680FD	0.40	-2.48
Proposed	100yr-72hr	L-0690PERC	0.14	0.00
Proposed	100yr-72hr	L-0700PERC	0.00	0.00
Proposed	100yr-72hr	L-0710FD	2.08	-1.28
Proposed	100yr-72hr	L-0720PERC	0.49	0.00
Proposed	100yr-72hr	L-0730PERC	0.00	0.00
Proposed	100yr-72hr	L-0740FD	1.50	-0.01
Proposed	100yr-72hr	L-0750FD	0.53	-0.04
Proposed	100yr-72hr	L-0760FD	0.49	-0.04
Proposed	100yr-72hr	L-0770PERC	0.13	0.00
Proposed	100yr-72hr	L-0780PERC	0.00	0.00
Proposed	100yr-72hr	L-0790PERC	0.09	0.00
Proposed	100yr-72hr	L-0800PERC	0.00	0.00
Proposed	100yr-72hr	L-0810PERC	0.10	0.00
Proposed	100yr-72hr	L-0820PERC	0.00	0.00
Proposed	100yr-72hr	L-0830W	5.67	-0.12
Existing	25yr-72hr	L-0030P	5.01	0.00
Existing	25yr-72hr	L-0040P	8.50	-0.01
Existing	25yr-72hr	L-0050W	0.00	0.00
Existing	25yr-72hr	L-0060W	4.17	-0.03
Existing	25yr-72hr	L-0070W	0.00	0.00
Existing	25yr-72hr	L-0140P	0.26	-0.63
Existing	25yr-72hr	L-0150P	7.38	-0.01
Existing	25yr-72hr	L-0160W	0.41	-0.03
Existing	25yr-72hr	L-0190DS	9.32	0.00
Existing	25yr-72hr	L-0200DS	4.40	-0.01
Existing	25yr-72hr	L-0210DS	0.22	0.00

Scenario	Sim	Link Name	Maximum Flow Rate [cfs]	Minimum Flow Rate [cfs]
Existing	25yr-72hr	L-0220DS	4.21	0.00
Existing	25yr-72hr	L-0230P	11.92	-0.01
Existing	25yr-72hr	L-0240W	8.25	0.00
Existing	25yr-72hr	L-0250P	1.65	-0.27
Existing	25yr-72hr	L-0280W	6.12	0.00
Existing	25yr-72hr	L-0290W	2.63	0.00
Existing	25yr-72hr	L-0300W	0.00	0.00
Existing	25yr-72hr	L-0310W	0.00	0.00
Existing	25yr-72hr	L-0320W	7.20	-2.61
Existing	25yr-72hr	L-0330W	0.00	0.00
Proposed	25yr-72hr	L-0030P	6.77	-0.09
Proposed	25yr-72hr	L-0040P	8.50	-0.35
Proposed	25yr-72hr	L-0050W	0.00	0.00
Proposed	25yr-72hr	L-0060W	5.57	0.00
Proposed	25yr-72hr	L-0070W	0.00	0.00
Proposed	25yr-72hr	L-0090W	0.00	0.00
Proposed	25yr-72hr	L-0150P	6.98	-0.90
Proposed	25yr-72hr	L-0160W	1.32	0.00
Proposed	25yr-72hr	L-0200DS	5.46	-0.01
Proposed	25yr-72hr	L-0210DS	1.05	0.00
Proposed	25yr-72hr	L-0220DS	0.00	0.00
Proposed	25yr-72hr	L-0230P	6.35	0.00
Proposed	25yr-72hr	L-0240W	6.40	0.00
Proposed	25yr-72hr	L-0380FD	2.63	-0.03
Proposed	25yr-72hr	L-0410PERC	0.64	0.00
Proposed	25yr-72hr	L-0420PERC	0.00	0.00
Proposed	25yr-72hr	L-0440W	0.00	0.00
Proposed	25yr-72hr	L-0450W	6.12	-2.40
Proposed	25yr-72hr	L-0460W	0.00	0.00
Proposed	25yr-72hr	L-0470W	5.58	0.00
Proposed	25yr-72hr	L-0480W	6.33	0.00
Proposed	25yr-72hr	L-0500W	3.36	0.00
Proposed	25yr-72hr	L-0510P	0.00	-3.16
Proposed	25yr-72hr	L-0530P	2.62	0.00
Proposed	25yr-72hr	L-0540W	3.35	0.00
Proposed	25yr-72hr	L-0550W	3.35	0.00
Proposed	25yr-72hr	L-0560W	3.35	0.00
Proposed	25yr-72hr	L-0570FD	3.02	-0.42
Proposed	25yr-72hr	L-0580FD	5.84	-0.57
Proposed	25yr-72hr	L-0610P	0.13	-3.28
Proposed	25yr-72hr	L-0620P	12.37	-0.61
Proposed	25yr-72hr	L-0630PERC	0.27	-0.01

Scenario	Sim	Link Name	Maximum Flow Rate [cfs]	Minimum Flow Rate [cfs]
Proposed	25yr-72hr	L-0640PERC	0.00	0.00
Proposed	25yr-72hr	L-0650PERC	0.50	0.00
Proposed	25yr-72hr	L-0660PERC	0.00	0.00
Proposed	25yr-72hr	L-0670P	1.32	-1.93
Proposed	25yr-72hr	L-0680FD	0.47	-7.28
Proposed	25yr-72hr	L-0690PERC	0.36	0.00
Proposed	25yr-72hr	L-0700PERC	0.00	0.00
Proposed	25yr-72hr	L-0710FD	2.00	-4.64
Proposed	25yr-72hr	L-0720PERC	0.69	0.00
Proposed	25yr-72hr	L-0730PERC	0.00	0.00
Proposed	25yr-72hr	L-0740FD	6.55	-0.42
Proposed	25yr-72hr	L-0750FD	2.88	-0.15
Proposed	25yr-72hr	L-0760FD	3.03	-0.16
Proposed	25yr-72hr	L-0770PERC	0.33	0.00
Proposed	25yr-72hr	L-0780PERC	0.00	0.00
Proposed	25yr-72hr	L-0790PERC	0.15	0.00
Proposed	25yr-72hr	L-0800PERC	0.00	0.00
Proposed	25yr-72hr	L-0810PERC	0.18	0.00
Proposed	25yr-72hr	L-0820PERC	0.00	0.00
Proposed	25yr-72hr	L-0830W	3.35	0.00
Existing	5yr-24hr	L-0030P	6.86	-0.02
Existing	5yr-24hr	L-0040P	10.55	0.00
Existing	5yr-24hr	L-0050W	0.00	0.00
Existing	5yr-24hr	L-0060W	5.23	0.00
Existing	5yr-24hr	L-0070W	0.00	0.00
Existing	5yr-24hr	L-0140P	0.69	-0.76
Existing	5yr-24hr	L-0150P	7.85	-0.02
Existing	5yr-24hr	L-0160W	0.42	-0.03
Existing	5yr-24hr	L-0190DS	10.42	0.00
Existing	5yr-24hr	L-0200DS	6.38	0.00
Existing	5yr-24hr	L-0210DS	0.15	0.00
Existing	5yr-24hr	L-0220DS	0.00	0.00
Existing	5yr-24hr	L-0230P	5.62	0.00
Existing	5yr-24hr	L-0240W	5.66	0.00
Existing	5yr-24hr	L-0250P	2.01	-0.86
Existing	5yr-24hr	L-0280W	6.47	0.00
Existing	5yr-24hr	L-0290W	2.67	0.00
Existing	5yr-24hr	L-0300W	0.00	0.00
Existing	5yr-24hr	L-0310W	0.00	0.00
Existing	5yr-24hr	L-0320W	1.64	0.00
Existing	5yr-24hr	L-0330W	0.00	0.00
Proposed	5yr-24hr	L-0030P	7.50	0.00

Scenario	Sim	Link Name	Maximum Flow Rate [cfs]	Minimum Flow Rate [cfs]
Proposed	5yr-24hr	L-0040P	10.17	-0.12
Proposed	5yr-24hr	L-0050W	0.00	0.00
Proposed	5yr-24hr	L-0060W	5.70	0.00
Proposed	5yr-24hr	L-0070W	0.00	0.00
Proposed	5yr-24hr	L-0090W	0.00	0.00
Proposed	5yr-24hr	L-0150P	6.27	-0.16
Proposed	5yr-24hr	L-0160W	0.00	0.00
Proposed	5yr-24hr	L-0200DS	6.36	0.00
Proposed	5yr-24hr	L-0210DS	0.00	0.00
Proposed	5yr-24hr	L-0220DS	0.00	0.00
Proposed	5yr-24hr	L-0230P	5.86	0.00
Proposed	5yr-24hr	L-0240W	5.90	0.00
Proposed	5yr-24hr	L-0380FD	1.80	-0.07
Proposed	5yr-24hr	L-0410PERC	0.35	0.00
Proposed	5yr-24hr	L-0420PERC	0.00	0.00
Proposed	5yr-24hr	L-0440W	0.00	0.00
Proposed	5yr-24hr	L-0450W	0.00	0.00
Proposed	5yr-24hr	L-0460W	0.00	0.00
Proposed	5yr-24hr	L-0470W	3.80	0.00
Proposed	5yr-24hr	L-0480W	5.87	0.00
Proposed	5yr-24hr	L-0500W	2.03	0.00
Proposed	5yr-24hr	L-0510P	0.00	-2.03
Proposed	5yr-24hr	L-0530P	0.41	0.00
Proposed	5yr-24hr	L-0540W	2.03	0.00
Proposed	5yr-24hr	L-0550W	2.03	0.00
Proposed	5yr-24hr	L-0560W	2.03	0.00
Proposed	5yr-24hr	L-0570FD	1.91	-0.15
Proposed	5yr-24hr	L-0580FD	3.36	-0.25
Proposed	5yr-24hr	L-0610P	0.01	-2.01
Proposed	5yr-24hr	L-0620P	7.02	-0.27
Proposed	5yr-24hr	L-0630PERC	0.20	-0.01
Proposed	5yr-24hr	L-0640PERC	0.00	0.00
Proposed	5yr-24hr	L-0650PERC	0.40	0.00
Proposed	5yr-24hr	L-0660PERC	0.00	0.00
Proposed	5yr-24hr	L-0670P	0.26	-1.77
Proposed	5yr-24hr	L-0680FD	1.79	-6.34
Proposed	5yr-24hr	L-0690PERC	0.36	-0.01
Proposed	5yr-24hr	L-0700PERC	0.00	0.00
Proposed	5yr-24hr	L-0710FD	2.70	-3.49
Proposed	5yr-24hr	L-0720PERC	0.54	-0.06
Proposed	5yr-24hr	L-0730PERC	0.00	0.00
Proposed	5yr-24hr	L-0740FD	5.72	-1.06

Scenario	Sim	Link Name	Maximum Flow Rate [cfs]	Minimum Flow Rate [cfs]
Proposed	5yr-24hr	L-0750FD	2.55	-0.26
Proposed	5yr-24hr	L-0760FD	2.69	-0.26
Proposed	5yr-24hr	L-0770PERC	0.33	-0.01
Proposed	5yr-24hr	L-0780PERC	0.00	0.00
Proposed	5yr-24hr	L-0790PERC	0.14	-0.01
Proposed	5yr-24hr	L-0800PERC	0.00	0.00
Proposed	5yr-24hr	L-0810PERC	0.16	-0.01
Proposed	5yr-24hr	L-0820PERC	0.00	0.00
Proposed	5yr-24hr	L-0830W	2.03	0.00

87311143

AGREEMENT FOR AMENDMENT
OF NOTATION ON PLAT

THIS IS an AGREEMENT between BROWARD COUNTY, a political subdivision of the State of Florida, its successors and assigns hereinafter referred to as "COUNTY";

AND

THE FIRST PRESBYTERIAN CONTINUING CHURCH OF CORAL SPRINGS, INC. its successors and assigns, hereinafter referred to as "DEVELOPER."

WHEREAS, DEVELOPER is the owner of _____,
~~22-01000000~~ the LUPTON Plat, which Plat was recorded
PLAT
in ~~Official Records~~ Book 122, Page 30, in the Public Records
of Broward County on March 15, 1985; and

WHEREAS, the LUPTON Plat contains the following
notation on the face of the Plat:

This Plat is restricted to 148 Townhouse units

_____;

and

WHEREAS, DEVELOPER has determined there exists a need for an amendment to said notation; and

WHEREAS, the COUNTY has no objection to amending said notation and the Board of County Commissioners approved such an amendment at its meeting of January 7, 1985;

NOW, THEREFORE, in consideration of the mutual terms, conditions and promises hereinafter set forth, the COUNTY and DEVELOPER agree as follows:

1. The above recitals and representations are true and correct.
2. COUNTY and DEVELOPER hereby agree that the notation shown on the face of the LUPTON Plat for the purpose of clarifying and limiting the use of the LUPTON Plat property which states:

This Plat is limited to 148 Townhouse units

_____.

JUL 22 3 17 AM '97

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is hereby amended to read as follows:

This Plat is limited to use as a Church. This
note is required by Chapter 5, Article 9,
Broward County Code of Ordinances and may be
amended by agreement by Broward County .

3. This Agreement shall be recorded in the Public Records of Broward County, Florida, to evidence the agreements and representations of the parties as set forth herein.

IN WITNESS WHEREOF, the parties hereto have made and executed this Agreement on the respective dates under each signature: BROWARD COUNTY through its BOARD OF COUNTY COMMISSIONERS, signing by and through its Chairman, authorized to execute same by Board action on the 7th day of January, 1986, and THE FIRST PRESBYTERIAN CONTINUING CHURCH OF CORAL SPRINGS, INC., signing by and through its PRESIDENT, duly authorized to execute same.

COUNTY

ATTEST:

Z. A. Hester
County Administrator and Ex-
Officio Clerk of the Board of
County Commissioners of
Broward County, Florida

BROWARD COUNTY, through its
BOARD OF COUNTY COMMISSIONERS

By [Signature]
Chairman

21st day of July, 19__

Approved as to form and legality by
Office of General Counsel
for Broward County, Florida
SUSAN F. DELEGAL, General Counsel
Governmental Center, Suite 423
115 South Andrews Avenue
Fort Lauderdale, Florida 33301
Telephone: (305) 357-7600

By [Signature]
Assistant General Counsel

STATE OF FLORIDA)
COUNTY OF) SS.

I HEREBY CERTIFY that on this day before me, an officer duly qualified to take acknowledgments, personally appeared Harold C. Toman, to me well known and known to me to be the Chairman of the Board of County Commissioners of Broward County, Florida, and who executed the foregoing instrument, and acknowledged to and before me that he executed said instrument for the purposes therein expressed.

1987 WITNESS my hand and official seal, this 21st day of July

My commission expires:

NOTARY PUBLIC, STATE OF FLORIDA
MY COMMISSION EXPIRES JAN. 23, 1990.
BONDED THRU NOTARY PUBLIC UNDERWRITERS

BK14640P60653

DEVELOPER

THE FIRST PRESBYTERIAN CONTINUING
CHURCH OF CORAL SPRINGS, INC.

WITNESS:

B Y X

J. FRANK BEAUCHAMP, PRES

13th day of September, 1986.

STATE OF FLORIDA

)

SS.

COUNTY OF BROWARD

;

I HEREBY CERTIFY that on this day before me, an officer duly qualified to take acknowledgments, personally appeared J. Frank Beauchamp, to me well known and known to me to be the President of THE FIRST PRESBYTERIAN CONTINUING CHURCH OF CORAL SPRINGS, INC, and who executed the foregoing instrument, and acknowledged to and before me that HE executed said instrument for the purposes therein expressed.

1986.

WITNESS my hand and official seal, this 18th day of September,

Notary Public

My commission expires:

10-3-89

4540654 BK14540654

MORTGAGEE'S CONSENT AND ACKNOWLEDGMENT

The undersigned, FIRST UNION NATIONAL BANK OF FLORIDA hereby certifies that it is the holder of a mortgage upon the parcel as shown on the LUPTON Plat, and does hereby consent to the foregoing Agreement for Amendment of Notation on the LUPTON Plat.

WITNESS:

Christopher P.
Deane B. Schyske

FIRST UNION NATIONAL BANK OF FLORIDA

By Sally B. Robinson, V.P.

28th day of January, 1987.

(SEAL)

STATE OF FLORIDA)
) SS.
COUNTY OF BROWARD)

I HEREBY CERTIFY that on this day before me, an officer duly qualified to take acknowledgments, personally appeared Sally B. Robinson, to me well known and known to me to be the Vice President of FIRST UNION NATIONAL BANK OF FLORIDA, and who executed the foregoing instrument, and acknowledged to and before me that she executed said instrument for the purposes therein expressed.

WITNESS my hand and official seal, this 28th day of January, 1987.

Virginia L. Smith
Notary Public

My commission expires:

Notary Public, State of Florida
My Commission Expires March 18, 1987
Bonded thru Troy Fair Insurance, Inc.

RECORDED IN THE OFFICIAL RECORDS BOOK
OF BROWARD COUNTY, FLORIDA
L. A. HESTER
COUNTY ADMINISTRATOR

ASL:ibl
A138-N
4/4/85
#85-109

BK14640PG0655



Rodrigo Torres, P.E.
Kimley-Horn
8201 Peters Rd, Suite 2200
Plantation, FL 33324
D: 954.519.2432

Matt Garde
Olsson, Inc.
11600 Broadway Extension, Suite 300
Oklahoma City, OK 73114
O: 405.242.6600

Rodrigo:

Please see information below for our expected water consumption regarding this project.

References

Per Margate, FL – Code of Ordinances, 1 ERC = 335 GPD.

https://library.municode.com/fl/margate/codes/code_of_ordinances?nodeId=PTIICOOR_CH39WASEEPENENSE_AR_TVIIUTRURERA_S39-73COCH

Design Water Supply Fixture Units: 221 WSFU (Refer to Appendix A for additional information)

Design Peak Flowrate: 80 GPM

Restroom Daily Usage and Durations:

The occupancy fluctuations of a church make the daily usage of restrooms vary based on when there are church services being provided. For a typical week, the highest usage will be on Sunday morning's when there will be a large amount of people coming and going to services. The other days of the week will be minimal use due to a decreased capacity and act more like an office schedule rather than a public assembly space. For these calculations, it's assumed that there are 20 minutes between services where occupants will fill the restrooms. Assuming there are 6 services, that equates to 120 minutes (2 hr). The other days of the week are assumed to have about 30 minutes of usage total per day (i.e. there most likely will not be 20 consecutive minutes of people flushing toilets, washing hands, etc.; the usage will be more intermittent). The other days of the week also won't see the same peak flowrate due to decreased occupancy count and simultaneous usage. For the purpose of this calculation, a diversity factor of 25% has been applied for Monday – Saturday. Taking the product of the duration and flowrate results in the design usage per day.

<i>Day</i>	<i>Usage Duration</i>	<i>Usage Flowrate</i>	<i>Usage per day</i>	<i>Equivalent Residential Connection (ERC)</i>
Sunday	120 min (2 hr)	80 GPM	9600 GPD	29
Monday	30 min (0.5 hr)	20 GPM	600 GPD	2
Tuesday	30 min (0.5 hr)	20 GPM	600 GPD	2
Wednesday	30 min (0.5 hr)	20 GPM	600 GPD	2
Thursday	30 min (0.5 hr)	20 GPM	600 GPD	2
Friday	30 min (0.5 hr)	20 GPM	600 GPD	2
Saturday	30 min (0.5 hr)	20 GPM	600 GPD	2

Table 1: Daily ERC Summary

With the design usage per day, we can then determine the ERC count by taking the quotient of the usage per day divided by 335 (from the city code). It's important to note that in these calculations the usage duration is not based on how much time occupants spend in the facilities but instead how much time the water is actually running. As an example, a person might spend 10 minutes in the restroom but the water might only run for 1 minute (while the watercloset is flushing / refilling & when the lavatory faucet is running). These calculations also exclude any equipment / fixtures outside the building such as yard hydrants, fountains, irrigation, etc.

Using these numbers, a weekly water consumption can also be determined by summing all of the Daily ERC values together. This leads to a weekly usage of 41 ERC.

APPENDIX A

EXISTING WSFU								
<u>Fixture Type</u>	<u>Qty.</u>	<u>Hot</u>	<u>Cold</u>	<u>Total</u>	<u>Sub-Total Hot</u>	<u>Sub-Total Cold</u>	<u>Sub-Total</u>	<u>Notes</u>
Water Closet (Tank)	35	0	2	2	0	70	70	WSFU updated to current code requirements
Urinal	9	0	5	5	0	45	45	WSFU updated to current code requirements
Lavatory	24	1.5	1.5	2	36	36	48	WSFU updated to current code requirements
Mop Sink	1	2.25	2.25	3	2.25	2.25	3	WSFU updated to current code requirements
Electric Water Cooler	4	0	0.25	0.25	0	1	1	WSFU updated to current code requirements
Sub-Total	73	3.75	11	12.25	38.25	154.25	167	
<u>Fixture Type (Fixed Flowrates)</u>	<u>Qty.</u>	<u>GPM</u>						<u>Notes</u>
3 Comp Sink	1	10						Estimate
WSFU Count	167	FU						
WSFU Flowrate	60	GPM						
Fixed Flowrate	10	GPM						
Total Flowrate	70	GPM						

APPENDIX A

RENOVATION & ADDITION WSFU								
<u>Demolition</u>	<u>Qty</u>							
Water Closet (Tank)	2							
Urinal	2							
Lavatory	3							
Mop Sink	1							
Electric Water Cooler	1							
<u>Added</u>	<u>Qty</u>							
Water Closet (Tank)	10							
Urinal	4							
Lavatory	7							
Mop Sink	2							
Electric Water Cooler	1							
Sinks (Bar/Prep)	9							
Sinks (Kitchen)	1							
<u>Fixture Type</u>	<u>Qty.</u>	<u>Hot</u>	<u>Cold</u>	<u>Total</u>	<u>Sub-Total Hot</u>	<u>Sub-Total Cold</u>	<u>Sub-Total</u>	<u>Notes</u>
Water Closet (Tank)	43	0	2	2	0	86	86	
Urinal	11	0	5	5	0	55	55	
Lavatory	28	1.5	1.5	2	42	42	56	
Mop Sink	2	2.25	2.25	3	4.5	4.5	6	
Electric Water Cooler	4	0	0.25	0.25	0	1	1	
Sinks (Bar/Prep)	9	1	1	1.4	9	9	12.6	
Sinks (Kitchen)	1	3	3	4	3	3	4	
Sub-Total	98	7.75	15	17.65	58.5	200.5	220.6	
<u>Fixture Type (Fixed Flowrates)</u>	<u>Qty.</u>	<u>GPM</u>						<u>Notes</u>
3 Comp Sink	1	10						Estimate
WSFU Count	220.6	FU						
WSFU Flowrate	70	GPM						
Fixed Flowrate	10	GPM						
Total Flowrate	80	GPM						

PROPERTY SUMMARY

Tax Year: 2024	Property Use: 71-01 Churches	Deputy Appraiser: Commercial Department
Property ID: 484126130010	Millage Code: 1212	Appraisers Number: 954-357-6835
Property Owner(s): ABUNDANT LIFE CHRISTIAN CENTRE INC	Adj. Bldg. S.F: 42710	Email: commercialtrim@bcpa.net
Mailing Address: 1811 BANKS RD MARGATE, FL 33063-7702	Bldg Under Air S.F:	Zoning : R-3 - MULTIPLE FAMILY DWELLING
Physical Address: 7955 ROYAL PALM BOULEVARD MARGATE, 33063	Effective Year: 2002	Abbr. Legal Des.: THE LUPTON PLAT 122-30 B PARCELS A & B
	Year Built: 1992	
	Units/Beds/Baths: 0 / /	

PROPERTY ASSESSMENT

Year	Land	Building / Improvement	Agricultural Saving	Just / Market Value	Assessed / SOH Value	Tax
2024	\$3,779,510	\$2,551,990	0	\$6,331,500	\$6,331,500	
2023	\$3,779,510	\$2,551,990	0	\$6,331,500	\$6,331,500	
2022	\$3,779,510	\$2,066,800	0	\$5,846,310	\$5,846,310	

EXEMPTIONS AND TAXING AUTHORITY INFORMATION

	County	School Board	Municipal	Independent
Just Value	\$6,331,500	\$6,331,500	\$6,331,500	\$6,331,500
Portability	0	0	0	0
Assessed / SOH	\$6,331,500	\$6,331,500	\$6,331,500	\$6,331,500
Granny Flat				
Homestead	0	0	0	0
Add. Homestead	0	0	0	0
Wid/Vet/Dis	0	0	0	0
Senior	0	0	0	0
Exemption Type	\$6,331,500	\$6,331,500	\$6,331,500	\$6,331,500
Affordable Housing	0	0	0	0
Taxable	0	0	0	0

SALES HISTORY FOR THIS PARCEL					LAND CALCULATIONS		
Date	Type	Price	Book/Page or Cin		Unit Price	Units	Type
07/28/2014	Warranty Deed	\$5,650,000	112439393		\$7.00	538,876 SqFt	Square Foot
	Disqualified Sale				\$0.10	73,802 SqFt	Square Foot
11/18/1985	Warranty Deed	\$1,100,000	12981 / 64				

RECENT SALES IN THIS SUBDIVISION

Property ID	Date	Type	Qualified/ Disqualified	Price	CIN	Property Address
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SPECIAL ASSESSMENTS									SCHOOL
Fire	Garb	Light	Drain	Impr	Safe	Storm	Clean	Misc	Margate Elementary: A Margate Middle: C Coral Springs High: I

ELECTED OFFICIALS

Property Appraiser Marty Kiar	County Comm. District 2	County Comm. Name Mark David Bogen	US House Rep. District 23	US House Rep. Name Jared Moskowitz
Florida House Rep. District 95	Florida House Rep. Name Christine Hunschofsky	Florida Senator District 30	Florida Senator Name Tina Scott Polsky	School Board Member Lori Alhadeff



[Department of State](#) / [Division of Corporations](#) / [Search Records](#) / [Search by Entity Name](#) /

Detail by Entity Name

Florida Not For Profit Corporation

ABUNDANT LIFE CHRISTIAN CENTRE, INC.

Filing Information

Document Number	748058
FEI/EIN Number	59-1937119
Date Filed	07/12/1979
State	FL
Status	ACTIVE
Last Event	EVENT CONVERTED TO NOTES
Event Date Filed	09/16/1980
Event Effective Date	NONE

Principal Address

7955 Royal Palm Blvd
MARGATE, FL 33063

Changed: 04/26/2024

Mailing Address

1855 Northwest 124th Avenue
Coral Springs, FL 33071

Changed: 04/26/2024

Registered Agent Name & Address

THOMAS, RICHARD W
1855 NW 124 AVE
CORAL SPRINGS, FL 33071

Name Changed: 04/21/2010

Address Changed: 01/27/2000

Officer/Director Detail

Name & Address

Title PRESIDENT

THOMAS, RICHARD W, BISHOP
1855 NW 124 AVE
CORAL SPRINGS, FL 33071

Title VP, Secretary

THOMAS, KATHY A, REV
1855 NW 124th Avenue
Coral Springs, FL 33071

Title DIRECTOR

THOMAS, Richard S
2700 NW 83 Terrace
CORAL SPRINGS, FL 33065

Title DIRECTOR

SWAD, KEVIN
12085 NW 50TH DRIVE
Coral Springs, FL 33076

Annual Reports

Report Year	Filed Date
2023	04/24/2023
2024	04/26/2024
2025	02/03/2025

Document Images

02/03/2025 -- ANNUAL REPORT	View image in PDF format
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