

PRELIMINARY STORM WATER MANAGEMENT CALCULATIONS
FOR
NUVO MARGATE SELF STORAGE

MARGATE, FLORIDA
MMA #15-25

June 24, 2016
Revised:
N/A

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TODD N. MCLEOD, P.E.
Florida License No. 69188

6/24/2016

Date



McLeod • McCarthy & Associates, P.A.
Civil Engineers

June 24, 2016

Re: **Drainage Statement for Nuvo Margate Self Storage**
State Road 7, Margate, Florida (MMA #16-010)

Site Description

The Riverside Storage site is located at the northeast corner of State Road 7 and SW 8th Court in the City of Margate, Florida. The subject site is 2.80 acres and is currently vacant. A dry swale providing drainage for State Road 7 occupies the western 0.48 acres of the site and is within an FDOT drainage easement. The site generally slopes from north to south with existing topography ranging between elevations 9.00 and 12.00 NAVD.

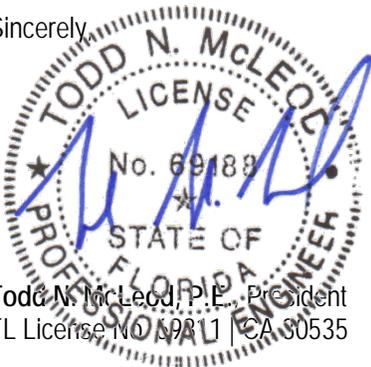
Site Drainage

The site is within the boundaries of the South Florida Water Management District (SFWMD) C-14 Basin. The site will be developed to support a mix of self storage (Phase I) and commercial uses (Phase II). The site development will include excavation of a dry retention area and installation of exfiltration trench. For the purposes of the enclosed stormwater management calculations, the 0.48 acres encompassed by the FDOT drainage ditch has been classified as non-contributing area as it will not connect to the proposed on-site stormwater management system. Drainage design will address the following criteria:

- Collection, conveyance, and treatment of stormwater runoff for the project area via a network of inlets, culverts, exfiltration trench and dry retention.
- Water Quality Treatment in accordance with SFWMD, FDOT, City and County requirements
- Perimeter Berms meeting 25 year – 3 day flood criteria with no increase in post-development stages
- Finished Floor Elevations for all buildings set above the 100 year – 3 day (zero discharge) stage
- Parking Lots set above 5 year – 1 day flood stage

If you have any questions regarding this application, feel free to contact me at 561.689.9500 or todd@mcleodmccarthy.com.

Sincerely,



Todd N. McLeod, P.E., President
FL License No. 69188 | CA 30535

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Project Name: NUVO MARGATE SELF STORAGE

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Engineer: TNM

Date: 06/24/16

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LAND USE BREAKDOWN

PROPOSED

Site Area = 2.80 ac
 Non-Contributing Area = 0.48 ac (SR 7 Swale & Sidewalk)
 Basin Area = 2.32 ac

	Acres	%	Grading	
			From	To
Impervious Area				
Building	0.65 ac	(28%)	12.50	
Pavement	0.68 ac	(29%)	11.00	12.00
Pervious Area				
Green Space	0.67 ac	(29%)	9.50	12.00
Dry Detention	0.21 ac	(9%)	8.50	
Detention Banks	0.11 ac	(5%)	8.50	11.00
Subtotal Impervious Areas	1.33 ac	(57%)		
Subtotal Pervious Areas	0.99 ac	(43%)		

Find Curve Number:

Avg. Pervious Ground El. = 10.16
 Control EL. = 7.50
 Depth to Water Table = 2.66
 Soil Type = Flatwoods

Soil Storage Table

(SFWMD's Vol. IV, Basis of Review, page E-2)

Depth to W.T. (ft)	Coastal Storage (in)	Flatwoods Storage (in)	Depression Storage (in)
1.0	0.6	0.6	0.6
2.0	2.5	2.5	2.1
3.0	6.6	5.4	4.4
4.0	10.9	9.0	6.8

Pervious Area = 0.99 ac
 Storage from Table = 3.31 in (w/ 25% compaction)
 Avail Soil Storage = 0.27 af
 Soil Moisture Storage (S) = 1.41 in
 Curve Number = 88

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STAGE -STORAGE CALCULATIONS

PROPOSED

Starting Stage	7.50
Ending Stage	13.50
Stage Increment	0.50

Name	Pavement	Green Space	Dry Detention	Detention Banks	Trench	
Area	0.68	0.67	0.21	0.11	0.28 (AF)	
Start Elev	11.00	9.50	8.50	8.50	7.50	
End Elev	12.00	12.00	0.00	11.00	9.50	
Stage Feet	Linear Storage	Linear Storage	Vert Storage	Linear Storage	Linear Storage	Total Storage
NAVD	Ac-ft	Ac-ft	Ac-ft	Ac-ft	Ac-ft	Ac-ft
7.50	0.00	0.00	0.00	0.00	0.00	0.00
8.00	0.00	0.00	0.00	0.00	0.07	0.07
8.50	0.00	0.00	0.00	0.00	0.14	0.14
9.00	0.00	0.00	0.11	0.01	0.21	0.32
9.50	0.00	0.00	0.21	0.02	0.28	0.51
10.00	0.00	0.03	0.32	0.05	0.28	0.68
10.50	0.00	0.13	0.42	0.09	0.28	0.92
11.00	0.00	0.30	0.53	0.14	0.28	1.24
11.50	0.09	0.54	0.63	0.19	0.28	1.72
12.00	0.34	0.84	0.74	0.25	0.28	2.44
12.50	0.68	1.17	0.84	0.30	0.28	3.28
13.00	1.02	1.51	0.95	0.36	0.28	4.11
13.50	1.36	1.84	1.05	0.41	0.28	4.95

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POST-DEVELOPMENT RUNOFF (ZERO DISCHARGE CALCULATIONS)

Soil Moisture Storage (S_{prop}) 1.41 in

5 Year, 1 Day Rainfall Amount (P): 7.0 in Figure C-3

25 Year, 3 Day Rainfall Amount (P): 14.0 in Figure C-8

100 Year, 3 Day Rainfall Amount (P): 18.0 in Figure C-9

PARKING LOT: 5-YEAR, 1-DAY RUNOFF CALCULATIONS (PER SFWMD):

Proposed:

$$Q = (P - (0.2XS))^2 / (P + (0.8*S))$$

$$= 5.5 \text{ in}$$

$$\text{Volume} = Q \times \text{Site Area} \times 1/12''$$

$$= 5.5 \text{ in} \times 2.32 \times 1/12'' = 1.07 \text{ AF}$$

PERIMETER BERM: 25-YEAR, 3-DAY RUNOFF CALCULATIONS:

Proposed:

$$Q = (P - (0.2XS))^2 / (P + (0.8*S))$$

$$= 12.4 \text{ in}$$

$$\text{Volume} = Q \times \text{Site Area} \times 1/12''$$

$$= 12.4 \text{ in} \times 2.32 \times 1/12'' = 2.40 \text{ AF}$$

FINISHED FLOORS: 100-YEAR, 3-DAY RUNOFF CALCULATIONS:

Proposed:

$$Q = (P - (0.2XS))^2 / (P + (0.8*S))$$

$$= 16.4 \text{ in}$$

$$\text{Volume} = Q \times \text{Site Area} \times 1/12''$$

$$= 16.4 \text{ in} \times 2.32 \times 1/12'' = 3.17 \text{ AF}$$

RAINFALL/ ROUTING SUMMARY (See attached CASCADE routings)

Interpolated from Stage vs. Storage Table

Storm Event	Rainfall (in)	Peak Stage (ft-NAVD)	Peak Discharge (cfs)	Design Criteria	Prop Stage (ft-NAVD)
5-yr, 1-day =	7.0	10.75	0.00	Min. Parking Lot Grade	11.00
25-yr, 3-day =	14.0	11.95	0.00	Min. Perimeter Grade	12.00
100-yr, 3-day =	18.0	12.40	0.00	Finished Floors - Zero Discharge	12.50

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WATER QUALITY CALCULATIONS

1-inch Over the Project Area

$$\text{(Treated Volume)} \quad 1\text{-inch} \quad * \quad 1\text{-ft}/12\text{-in} \quad * \quad \frac{2.32}{\text{PROJECT AREA (AC)}} \quad = \quad \boxed{0.19} \text{ ac-ft}$$

TREATED VOLUME

2.5-inches Times the Percent Impervious

$$\begin{aligned} \text{(Site Area)} \quad & \frac{2.32}{\text{PROJECT AREA (AC)}} - \left(\frac{0.00}{\text{LAKES (AC)}} + \frac{0.65}{\text{ROOFS (AC)}} \right) = \frac{1.67}{\text{SITE AREA}} \text{ ac} \\ \text{(Impervious Area)} \quad & \frac{1.67}{\text{SITE AREA (AC)}} - \frac{0.99}{\text{PERVIOUS AREA (AC)}} = \frac{0.68}{\text{IMPERVIOUS AREA}} \text{ ac} \\ \text{(% Impervious)} \quad & \frac{\text{IMPERVIOUS AREA} * 100\%}{\text{SITE AREA (AC)}} = \frac{40.72\%}{\text{SITE AREA (AC)}} \\ \text{(2.5-in * \% Imp.)} \quad & 2.5\text{-inches} * \frac{40.72\%}{\text{PERCENT IMPERVIOUS}} = \frac{1.02}{\text{INCHES TO BE TREATED}} \text{ in} \\ \text{(Treated Volume)} \quad & \frac{1.02}{\text{TREATED (IN)}} * 1\text{-ft}/12\text{-in} * \frac{2.32}{\text{PROJECT AREA - LAKES (AC)}} = \boxed{0.20} \text{ ac-ft} \end{aligned}$$

THEREFORE 2.5-INCHES X %IMP GOVERNS

Required WQ Treatment = $\boxed{0.20}$ ac-ft
Required Retention (50% credit) = $\boxed{0.10}$ ac-ft

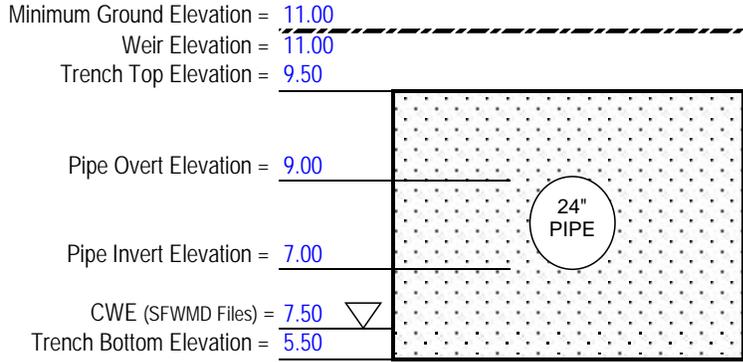
Provided WQ Treatment (Via Retention @ TOB EL. 11.00) = $\boxed{1.24}$ ac-ft

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EXFILTRATION TRENCH DESIGN



Standard Formula

$$L = V / (K(H_2 \cdot W + 2 \cdot H_2^2 \cdot Du - Du^2 + 2 \cdot H_2 \cdot Ds) + (1.39 \cdot 10^{-4}) \cdot W \cdot Du)$$

<=== INPUT ONLY IN GRAY CELLS

V	Volume Treated (acre-in) (3 Yr - Hr Vol.)	7.42
W	Trench Width (feet)	8
K	Hydraulic Conductivity (cfs/ft ² -ft.head)	7.05E-05
H ₂	Depth to Water Table (feet)	3.50
Du	Non Saturated Trench Depth (feet)	2.00
Ds	Saturated Trench Depth (feet)	2.00

(Max. Allow. Vol. = 2.32 x 3.2")

<=== H₂ can extend no lower than the CWE

L Maximum Length of Trench Allowed 1260

$$V = L \cdot (K(H_2 \cdot W + 2 \cdot H_2^2 \cdot Du - Du^2 + 2 \cdot H_2 \cdot Ds) + (1.39 \cdot 10^{-4}) \cdot W \cdot Du)$$

<=== INPUT ONLY IN GRAY CELLS

L	Length of Trench Provided	572
W	Trench Width (feet)	8
K	Hydraulic Conductivity (cfs/ft ² -ft.head)	7.05E-05
H ₂	Depth to Water Table (feet)	3.5
Du	Non Saturated Trench Depth (feet)	2
Ds	Saturated Trench Depth (feet)	2

V Volume Treated (acre-in) 3.37
Volume Treated (acre-ft) 0.28