

City of Margate DEVELOPMENT REVIEW COMMITTEE **Application for Site Plan**

Submittal Date (official use):

5790 Margate Blvd., Margate, FL 33063 954-972-6454

06-2)-15P12:59 RCV

-	934-972-0434	4			
Project Name Dollar General a					
Address SE Corner of State	Address SE Corner of State Road 7 and SW 7th St				
Acreage 1.8	Paid: \$ 800.00				
Existing Use Vacant Lot	Existing Use Vacant Lot				
Legal Description Serino Park Sec 3 81-46 B Tr B S 67.81 of N 432.62,					
SERINO PARK SEC 3 81-46 B TR B N 300					
SERINO PARK	SEC 3 81-46 B TR B,S 67.81 OF N 367.81				

Describe proposal/request in detail, including non-residential square footage and/or number of dwelling units

Construction of a 7,500 commercial retail store with associated parking lot and additional site improvements.

Agent/Contact Name Hanlex Margate, LLC	
Address 1000 Color Place Apopka, FL 3270	3
	Fax Number 407-410-0351
Email Address jbullard@hanlex.com	

Property Owner Name Tiger Investment Group, Inc. Address 10151 Deerwood Park Blvd 300-110 Jacksonville, FL 32256 Phone Number 904-472-2715 Fax Number 904-472-2701 Email Address ccash@flcb.com or sbaker@flcb.com OWNER'S AFFIDAVIT: I certify that I am the owner of record for the above referenced property and give authorization to file this petition. I understand that I, or a representative on my behalf, must be present at the DRC meeting. I further understand that my petition will be subject to the

regulations of Chapter 16 ½ of the Margate City Code. NUesmen ~ Property Owner's Signature

6 · 23 - 2015 Date

City of Margate *** CUSTOMER RECEIPT ***

Batch ID: CONEILL 6/29/15 00 Receipt no: 137446 Type SvcCd Description Amount ECDV SITE PLAN NON RESID. EL 1.00 Qty \$800.00 HANLEX DEVELOPMENT, LLC 1000 COLOR PLACE APOPKA, FL 32703 RE:SITE PLAN DOLLAR GENERAL SE CORNER OF SR 7 AND SW 7 ST 407-889-4154 Tender detail CK Ref#: \$800.00 3476 Total tendered: \$800.00 Total payment: \$800.00 Trans date: 6/29/15 Time: 13:12:46

HAVE A GREAT DAY!

Nathan Wolfe

From:	Padovan, Jim <jpadovan@wm.com></jpadovan@wm.com>
Sent:	Thursday, June 25, 2015 3:50 PM
То:	Nathan Wolfe
Subject:	RE: City of Margate Waste Management Approval

This is approved, thank you.

Happy Connecting. Sent from my Sprint Samsung Galaxy S® 5

------ Original message ------From: Nathan Wolfe <NWolfe@hanlex.com> Date: 06/25/2015 3:44 PM (GMT-05:00) To: "Padovan, Jim" <JPadovan@wm.com> Subject: RE: City of Margate Waste Management Approval

Good afternoon Mr. Padovan,

I had been working with Kay Hurley to get a letter of approval from Waste Management, and you were copied along with Adrian Moore.

I had sent over the revised plan on Tuesday, but Kay is out of the office until Monday.

I had tried to call Adrian Moore directly, but only received a voicemail.

We are hoping to submit to Margate on Monday morning, and would like to have the letter today or tomorrow at the latest if possible.

Can you help me with the letter? I have attached the revised plan I had sent on Tuesday.

Thank you,

Nathan Wolfe

Hanlex Civil, LLC 1825 S. Orange Blossom Trail Apopka, FL 32703 p.407-889-4154 c.407-383-2587 f. 407-410-0351

WE ARE MOVING! Please note that as of July 1, 2015 our new office will be located at:

1000 Color Place Apopka, FL 32703

From: Nathan Wolfe Sent: Tuesday, June 23, 2015 11:29 AM To: 'Padovan, Jim'; Hurley, Kay **Cc:** 'Amoore@wm.com'; Jason Bullard **Subject:** RE: City of Margate Waste Management Approval

Please see the revised Dumpster Enclosure as requested.

The pad is 24 x 24 as requested, but the enclosure itself is 24 x 12'.

We are planning to submit to the City on Friday. If we can have the approval letter by then, it would be greatly appreciated.

Thank you

Nathan Wolfe

Hanlex Civil, LLC 1825 S. Orange Blossom Trail Apopka, FL 32703 p.407-889-4154 c.407-383-2587 f. 407-410-0351

WE ARE MOVING! Please note that as of July 1, 2015 our new office will be located at:

1000 Color Place Apopka, FL 32703

From: Padovan, Jim [mailto:JPadovan@wm.com]
Sent: Wednesday, June 17, 2015 10:50 AM
To: Hurley, Kay; Nathan Wolfe
Subject: FW: City of Margate Waste Management Approval

Please see Adrians comment below, and the attachment, thanks.

From: Moore, Adrian
Sent: Wednesday, June 17, 2015 10:46 AM
To: Padovan, Jim
Subject: RE: City of Margate Waste Management Approval

Jim,

The enclosure needs to be 12 by 12 for a combined 24 by 24 for a double. I attached the Builders Guide, maybe they can send it to him? Once corrected and resent to us, we can approve.

Adrian Moore Commercial Supervisor <u>Amoore@wm.com</u>

Waste Management 3831 Nw 21 Ave Pompano Beach, FI 33073 Tel 954-917 0388 Fax 954-917-0261

> Waste Management operates more than 1,600 natural gas trucks across North America, helping to reduce fleet emissions and increasing fuel efficiency by the year 2020. Learn more at <u>www.wm.com</u>.

From: Hurley, Kay Sent: Tuesday, June 16, 2015 5:26 PM To: Marcano, Edelsy; Padovan, Jim Subject: FW: City of Margate Waste Management Approval

I believe the approval is for perm service. Jim Padovan approves them I believe

From: Nathan Wolfe [mailto:NWolfe@hanlex.com] Sent: Tuesday, June 16, 2015 4:09 PM To: Hurley, Kay Subject: City of Margate Waste Management Approval

Good afternoon Kay,

We spoke briefly on the phone regarding a project in the City of Margate.

The project is at the southeast corner of SR 7 and SW7th St. A property card is attached.

The City requires as part of the DRC Submittal that we receive a Letter of Approval from Waste Management regarding dumpster enclosures.

Attached is the property card, a copy of the DRC application listing the requirement, and a preliminary site plan showing the dumpster location.

I am asking for a Letter of Approval accepting the proposed dumpster enclosure.

Please let me know if you need any additional information.

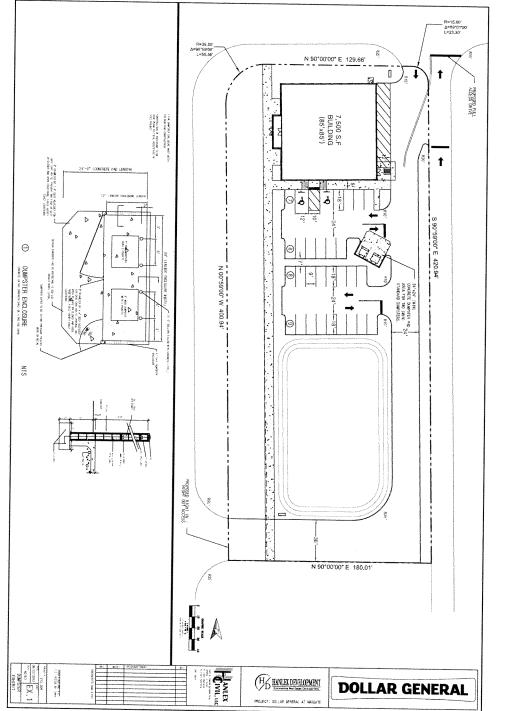
Thank you

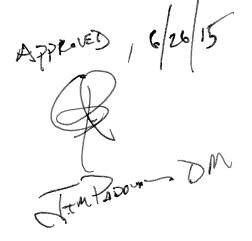
Nathan Wolfe

Hanlex Civil, LLC 1825 S. Orange Blossom Trail Apopka, FL 32703 p.407-889-4154 c.407-383-2587 f. 407-410-0351

WE ARE MOVING! Please note that as of July 1, 2015 our new office will be located at:

1000 Color Place Apopka, FL 32703 Recycling is a good thing. Please recycle any printed emails.







June 26, 2015

City of Margate 5790 Margate Blvd Margate, FL 33063

Reference: Dollar General at Margate TRC 1st Submittal Traffic Impact Statement

This Generalized Traffic Generation Analysis was undertaken in support of the application submitted for the proposed Dollar General located at the Southeast corner of State Road 7 and SW 7th St., in Margate, FL. This analysis assesses the potential traffic generation of the proposed development, in accordance with FDOT Standards. Based on the proposed use and generation figures supplied by the Transportation Engineer's (ITE) *Trip Generation Report, 9th Edition,* the proposed project is expected to generate 475 daily trips.

The proposed development consists of a 7,500 square foot commercial retail store. Access to the site is provided via driveways on State Road 7 and SW 7th St. A right-turn lane off of State Road 7 has been requested by FDOT and it shown on the submitted plans.

Trip Generation:

Trip generation rates for the proposed development were obtained from the Institute of Transportation Engineer's (ITE) *Trip Generation Report, 9th Edition*. The pass-by rates were obtained from the ITE *Trip Generation Handbook, 2nd Edition*.

Trip Gener	ation Calculati	ion Summary	/	
Description ITE III Code Quantity Dai				
Description	ITE LU Code	Quantity	Rate	Trips
Variety Store	814	7.500 KSF	64.03	475
Pass-by Trips (43%)				
Net New Trips				

From the calculation, the proposed site is projected to generate 475 daily trips. When factoring in the 43% pass-by reduction, the net new trips total 271, which is below the required 500 trips for a De Minimus exception.

Based on this calculation, we do not anticipate that a traffic study is required.

Respectfully, Jason Bullard, MBA P.E.

Hanlex Civil, LLC (407) 889-4154 jbullard@hanlex.com

DOLLAR GENERAL @ MARGATE MARGATE, FLORIDA

STORMWATER DRAINAGE REPORT FOR AN ON-LINE RETENTION SYSTEM

A RETAIL DEVELOPMENT ON A ±1.46 ACRE PARCEL

Prepared for: Hanlex Development, LLC 1825 S Orange Blossom Trail Apopka, Florida 32703

Prepared by: HANLEX CIVIL, LLC 1825 S Orange Blossom Trail Apopka, Florida 32703 (407) 889-4154

> Submitted to: City of Margate Broward County FDOT – District 4

<u>Date:</u> June 26th, 2015

> JASON D. BULLARD, PE HANLEX CIVIL, LLC LICENSE NO: 73800 CERTIFICATE OF AUTHORIZATION: 29910

DOLLAR GENERAL AT PEMBROKE PARK TABLE OF CONTENTS

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Appendix

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1. Project Description

The proposed development consists of a 7,500 square foot Dollar General and associated site improvements. The project is located on a 1.79 acre lot, however there is a 30' Roadway easement along the front of the parcel. The roadway easement makes up 0.33 acres and was excluded from the project area as it drains towards the FDOT ROW. The basin area contributing to the stormwater management system will be approximately, 1.46 acres. The location of this development is on the southeast corner of State Road 7 and SW 7th Street. A driver traveling northbound would encounter this parcel on the right, just before SW 7th Street.

Currently, the 1.46 acre site is vacant. Based upon review of existing topography and site conditions, the site drains towards the rear of the property.

The project is proposed to be constructed in a single phase at which time all the improvements shown on the attached site development drawings will be constructed.

We have effectively modeled this project as a single basin composed of the proposed development area contributing to the proposed stormwater system. There is no evidence of significant offsite stormwater flows to the property. Please refer to the Drainage Basin Exhibits located in Section 4, part A, of this report for a graphic depiction of the contributing area.

The proposed storm water system is an on-line dry retention system. We have modeled the proposed stormwater system using the PONDS software in order to ensure adequate recovery of the treatment volume and have included the design hydraulic calculations for the design storm elevations. We have shown that the proposed stormwater pond is adequate to treat the stormwater from the contributing basin area. Furthermore, we have shown that the pond will not stage higher in the post-development when compared to the existing site condition for the SFWMD design year storm (25-year, 72-hour). Please refer to the discussions and calculations within this report.

Please note all elevations provided in NGVD 1929 vertical datum.

2. Existing Conditions

A. Wetland and Flood Plain Consideration

There are no wetlands onsite or adjacent to the site. Based on review of FEMA maps this site is located partial inside a flood plain. FEMA FIRM describes the area as Zone "X", which are described as an area "outside the 0.2% annual chance flood", with a potion of the site within Zone "AE" marked with a flood elevation of 6. Please reference FIRM Panel No 12011C0355H, dated August 18, 2014. The flood plain exhibit is located within the appendix.

B. Historic Soils

Based on the USDA Soil Survey the property is comprised of Immokalee Fine Sands, nearly level soils. The soil type is categorized as a Type B/D soil and is considered poorly drained.

Please refer to the Geotechnical Report contained within this report for a detailed description of the historic nature of soils found on site and their classification. Please also refer to the USDA Soils map located within the appendix of this report.

C. Existing Drainage

Currently, the 1.46 acre site is vacant. Based upon review of existing topography and site conditions, the site drains towards the rear of the property.

Based on FDEP records this site is in the Cypress Creek/Pomano Canal watershed and is presently not impaired for phosphorus or nitrates.

Please refer to sheet C0.2 of the attached construction drawings illustrating the existing conditions.

3. Design Calculations

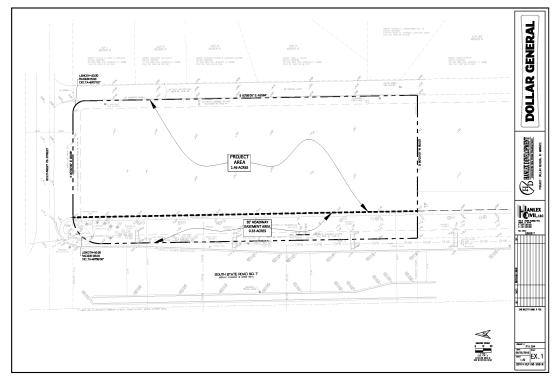
A. Site Areas and Drainage Basin Discussion

The pre-development and post-development basin area is the same and consists of approximately 1.46 acres. The pre-development condition consists of one basin; which is routed through the existing stormwater management system.

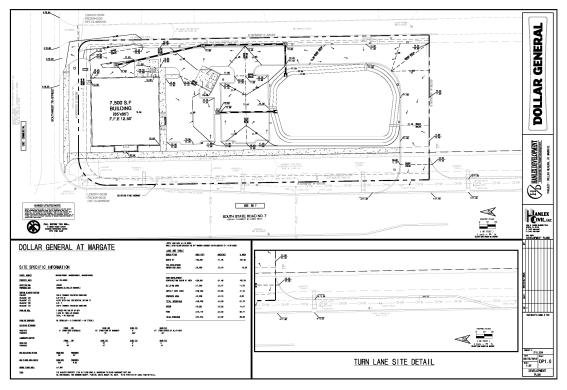
We have modeled the pre-development and post-development system utilizing the contributing basin area and have shown the system will adequately handle the required design criteria. Please refer to the Table 1, listing the areas used in the model, and to the drainage basin exhibits following.

Dollar General at Margate Pre-Development Conditions Curve Number						
		Existing Co		2		
<u>Description</u>	<u>Total</u> <u>Area</u> (sf)	<u>Total</u> <u>Area</u> (sf)	<u>Total</u> <u>Area</u> (ac)	Percent	Runoff Curve Number	Weighted Curve Number
Total Site area	63,601	63,601	1.46	100.00%		
Building Area	0				98	
Asphalt Pavement	0	8,300	0.19	13.05%	98	
Concrete/Sidewalk	8,300				98	55
Green Area	55,301	55,301	1.27	86.95%	49	
	F	Proposed Co	onditions			
<u>Description</u>	<u>Total</u> <u>Area</u> (sf)	<u>Total</u> <u>Area</u> <u>(sf)</u>	<u>Total</u> <u>Area</u> (ac)	<u>Percent</u>	Runoff Curve Number	Weighted Curve Number
Basin Area	63,601	63,601	1.46	100.00%		
Building Area	7,500	_			98	
Asphalt Pavement	26,232	39,258	0.90	61.73%	98	
Concrete/Sidewalk	5,526				98	91
Pond Area	15,116	15,116	0.35	23.77%	100	
Green Area	9,227	9,227	0.21	14.51%	49	

Table 1



Pre-Development Drainage Basin Map



Post-Development Drainage Basin Map

B. Stage Storage Tables

Dollar General at Margate Post-Development Stage-Storage					
Stage	StagePond Area (sf)Incremental Storage (cf)Total Storage 				
9.0	10,766	0.0	0		
10.0	12,430	11,598.0	11,598		
11.0	15,116	13,773.0	25,371		

Tables 2 show the stage and storage available within the proposed pond.

Table 2

C. Curve Number Calculation

In consideration of the Type B/D historic nature of the soils onsite, and upon review of the Geotechincal Report and recommendations by PSI, including a percolation test and results and proximity to seasonal high groundwater recommendation, we believe that a Type "B" soil classification is an accurate description of the soils on site. Based on the Type "B" soils and a site condition described as "good condition" with grass cover > 75%, we used a Curve Number of 61, with a weighted curve number method, to model the existing and proposed conditions.

Please refer to Table 1 and the Stormwater Model results in the appendix.

Source: U.S. Department of Agriculture *National Engineering Handbook*, Soil Conservation Service U.S. Department of Agriculture Section 4, Chapter 9, Hydraulic Soil Cover Complexes, 1972.

D. Total Site Storage

Based on Broward County's, FDOT and the City's Drainage Requirements the site must not change the stage elevation based on a non-discharge scenario. Table 3 shows the on-site total storage.

Dollar General at Margate Post-Development Stage-Storage

Stage (NGVD)	Pond Area (sf)	Incremental Storage (cf)	Parking Lot Area (sf)	Incremental Storage (cf)	*Site Area (sf)	Incremental Storage (cf)	Total Storage (Ac-Ft)
9.0	10,766	0.0	0.0	0.0	0.0	353.0	0.01
10.0	12,430	12,430.0	0.0	0.0	0.0	353.0	0.30
11.0	15,116	15,116.0	0.0	0.0	77.1	77.1	0.65
11.5	15,116	7,558.0	18,006.8	18,006.8	3,032.0	1,516.0	1.27
12.00	15,116	7,558.0	26,232.0	13,116.0	9,227.0	4,613.5	1.85
12.50	15,117	7,558.5	26,232.0	13,116.0	10,027.0	5,013.5	2.44

Table 2

*Site (Green Area/Sidewalks)

**Total Storage Starts with total Trench Storage for the storm water pipes system.

E. Water Quality Determination and Calculations

South Florida Water Management District (SFWMD) defines the volumetric requirement for stormwater treatment for an on-line retention system to be $\frac{1}{2}$ " over the drainage basin, for a development under 100 acres, however the City of Margate requires that treatment be 1" over the site.

The pond is designed to hold the required treatment volume within the proposed underground trenches.

We modeled the treatment volume as a "slug load" using PONDS, Version 3.0.

Dollar General at Margate Water Quality Determination for On-Line Retention System					
	Treatment Vol	ume			
Contributing Basin Area 1" Over Basin Area 2.5" Over Impervious Area Impervious Area					
Total Square Feet	63,601				
Acres	1.46				
Impervious Area	39,258				
Cubic Feet		5,300	8,179		
Required Treatment: Larg	est of the Row Above	8,1	79 CF		

Table 5

*Qualifies for 50% Reduction for Dry Retention Systems

F. Attenuation Model

Existing Conditions

Currently, the 1.46 acre site is vacant. Based upon review of existing topography and site conditions, the site drains towards the rear of the property.

Proposed Conditions

It is our intention to treat and attenuate the stormwater which falls onsite before allowing the stormwater discharge to follow its existing pattern, described above. After the treatment volume has been retained, the proposed stormwater system will attenuate the 25 year-72 hour design storm. Once the design elevation has been met the storm water will discharge to the rear of the property as is does in the predevelopment condition.

We used an SFWMD 72 hr hydrograph and a UHG 256 shape factor to define the storms, and the applicable run-off. Please refer to the Summary Tables on the following pages and the model and results found in the appendix.

4. Summary Tables

SFWMD/BROWARD COUNTY Model Results

Perimeter Berm Summary Table (NGVD)						
	Pre-Deve	elopment	Post-Development			
Storm	Volume (Ac-Ft)	Stage (NGVD, Ft)	Volume (Ac-Ft)	Stage (NGVD, Ft)	Post < Pre Stage	
25 Year 72 Hr	0.85	12.19	1.37	11.72	YES	
Post Development Perimeter Berm at Elevation 11.75 (NGVD)						

Finished Floor Elevation Summary Table (NGVD)					
Pre-Development Post-Development					
Storm	Volume (Ac-Ft)	Stage (NGVD, Ft)	Volume (Ac-Ft)	Stage (NGVD, Ft)	FFE Set Above FEMA Flood Elevation of 10
100 Year 72 Hr	1.27	12.47	1.85	12.00	12.5

5. Geotechnical Report and Permeability Results by PSI



REPORT OF GEOTECHNICAL ENGINEERING SERVICES **PROPOSED DOLLAR GENERAL – BROWARD COUNTY, FLORIDA** (S.E.C.) S.R. 7 AND S.W. 7TH STREET, MARGATE, FLORIDA **PSI PROJECT NO. 0397-937**

JUNE 22, 2015



June 22, 2015

Hanlex Development, LLC

1825 S. Orange Blossom Trail Apopka, Florida 32703

Attention:	Mr. Jeremy R. Anderson, P.E. Vice President
Re:	Report of Geotechnical Engineering Services Proposed Dollar General – Broward County, FL (S.E.C.) S.R. 7 and S.W. 7 th Street, Margate, Florida PSI Project No. 397-937

Dear Mr. Anderson:

Professional Service Industries, Inc. (PSI) has completed a geotechnical engineering study in connection with the noted project. Our services were provided and authorized in general accordance with the "Dollar General – Work Order Form", dated May 11, 2015.

PSI has also conducted a Phase I Environmental Site Assessment for the property, the results of which have been presented under separate cover. The environmental report should be reviewed to assess the potential impact on site/subgrade preparation requirements for this project.

1.0 PROJECT INFORMATION

The area of study is located at the southeast corner of S.R. 7 and S.W. 7th Street in Margate, Broward County, Florida. A site vicinity map identifying the project location with respect to existing streets and features is presented on **Sheet 1** of the **Appendix**.

As we understand, the project will consist of the construction of a Dollar General facility (Standard DG Prototype "D"). The facility will include a one-story, 7,225 square feet (sf) (85 feet by 85 feet) concrete block structure. Structural loads were not provided at this time. However, based on our experience with similar type developments, we have assumed maximum wall and column loads of 3 kips/foot and 100 kips, respectively. Additionally, we understand that the development will include paved parking/drive areas, stormwater management system and associated landscaping.

At the time of our study, the site was a vacant parcel of land with a ground cover consisting of light vegetation. Our truck mounted drilling equipment used for the subsurface exploration did not experience any difficulty in moving around the site. Photographs from our site visit are presented on **Sheet 2** of the **Appendix**.

If any of the noted information is incorrect or has changed, please notify PSI so that we may amend the recommendations presented in this report, if appropriate.

2.0 FIELD EXPLORATION AND SUBSURFACE CONDITIONS

2.1 STANDARD PENETRATION TEST (SPT) BORINGS

To evaluate the subsurface conditions at the site, we drilled/sampled nine SPT borings that were advanced to depths of 10 and 20 feet below grade. The SPT borings were performed using a CME-75 truck mounted drill rig equipped with an automatic hammer. The approximate locations at which the borings where drilled are presented on **Sheet 3** of the **Appendix**.

After seating the sample spoon six inches, the number of successive blows required to drive the sampler twelve inches into the soil constitutes the test result commonly referred to as the "N" value. The "N" value has been empirically correlated with various soil properties and is considered to be indicative of the relative density of cohesionless soils and the consistency of cohesive materials. The recovered split spoon samples were visually classified in the field and transported to the laboratory for further review. Following completion of our field services, all boreholes were backfilled with excavated soil/rock and the site generally cleaned, as required.

2.2 PERCOLATION TESTS

PSI performed two percolation tests at depths of 10 feet below grade within SPT borings PB-1 and PB-3. The percolation tests were performed in general accordance with the South Florida Water Management District (SFWMD) procedures for the "Usual Condition Constant Head" Percolation Test. SPT sampling was performed simultaneously as the boreholes were advanced using a 6-inch diameter casing. A 4-inch diameter perforated PVC pipe was placed in the borehole prior to retrieving the casing. Water was then pumped into the borehole in order to raise the water level as close to the ground surface as possible. Once the inflow equalized with the outflow rate, the average pumping rate and level of the water for this stabilized flow rate was recorded.

The hydraulic conductivity values determined from the tests are presented in **Table 1** of the **Appendix**. The values are in units of cubic feet of flow per second, per square foot of seepage area, per foot of head (cfs/ft²-ft). The tabulated values are ultimate values. The designer should apply an appropriate factor of safety to the reported values.

2.3 SUBSURFACE CONDITIONS

The soil types encountered at the boring locations are presented on individual logs included in the **Appendix**. The stratification presented is based on visual observation of the recovered soil samples and the interpretation of the field logs by a geotechnical engineer. Included with the logs are the N-values and groundwater levels measured at the time the borings were drilled.

The results of our borings performed for the study generally revealed a surficial cover of topsoil, followed by granular fill material comprising fine sand and limerock that persisted to depths ranging from 0.5 to 2.0 feet below grade. Below the granular fill material, the borings encountered medium dense to dense fine sand to the termination depth of the explorations at 20 feet below grade.

Also encountered in the borings in the 5.5 to 12.5 feet depth interval was a medium dense to dense layer of "hard pan" type material. The hard pan consisted of a 1.0 to 6.5 foot thick stratum of organic stained fine sand. The hard pan soils had percentage organic matter by dry weight and moisture content ranging from 3 to 7% and 15 to 20%, respectively. The organics in the soil are colloidal sized.



Hard pan soils are suitable for supporting shallow foundations of low to mid rise buildings and for typical pavement sections (flexible and rigid).

The Soil Survey of Broward County Florida, identifies the site to consist of Immokalee fine sand as the primary mapping unit. The findings of our field explorations are generally consistent with that of this mapping unit.

The groundwater was noted at depths ranging from 5.9 to 7.5 feet below grade at the time of drilling. It should be noted that groundwater levels fluctuate seasonally in response to rainfall and the infiltration rate of the soil. At a time of the year different from the time of drilling, there is a possibility of a change in the recorded levels. We estimate that during the peak of the wet hydroperiod, with rainfall and recharge at a maximum, groundwater levels at the site could be two feet higher than the values reported herein.

We recommend that the contractor determine the actual groundwater level at the time of construction to assess groundwater impact on the construction procedure.

Please refer to the individual boring logs presented in the **Appendix** for details on the subsurface conditions at specific exploration locations.

3.0 RECOMMENDATIONS

3.1 FOUNDATIONS

After completion of site preparation procedures as noted in **Section 4.0** of the report, the proposed structure can be supported on shallow foundations that are designed using a maximum net allowable bearing pressure of 3,000 pounds per square foot (psf), resting on compacted approved structural fill material or densified native soils.

The bottom of the footings should be at least 18 inches below the finished exterior grade in order to provide confinement. We further recommend that the footings supporting isolated columns have a minimum width of 36 inches and that continuous footings have a minimum width of at least 18 inches, even if those dimensions produce a bearing pressure less than the allowable. The purpose of limiting the minimum footing size is to prevent a "punching" shear failure and to reduce the possibility of bearing on an isolated weak zone.

Foundations subject to transient lateral loads will resist these forces through a combination of base shearing resistance mobilized at the footing-subgrade interface and earth pressure acting on the vertical faces of the footings at right angles to the direction of applied load. Base shearing resistance may be determined using a friction factor of 0.55. Passive earth pressure resistance should be computed using an equivalent fluid pressure of 180 pounds per square foot per foot of depth, for granular backfill material. Resistance to sliding determined in accordance with the noted parameters should be considered available/ultimate resistance. Accordingly, the design for sliding resistance should include a factor of safety. We suggest that a factor of safety of at least 1.5 be used.

To calculate the resistance of a footing to uplift forces, a prismatic failure block with vertical faces should be assumed above the footing base. The resisting forces will be provided by the combination of footing weight, overburden soil weight in the failure block, and shearing resistance along the faces of the soil block. The weight of the soil above the water table should be taken as



110 pounds per cubic foot (pcf). For submerged soil, a buoyant weight of 48 pcf should be used. The factor of safety against uplift should not be less than 1.5.

The amount of settlement of a foundation on top of granular soils is primarily governed by the elastic compressibility of the material, the size and depth of its foundations, and the pressure imposed on the supporting materials by the foundations. Based on the field test data obtained, our experience with similar structures and empirical relationships for bearing capacity and settlement, we have estimated that the maximum total settlement of the foundations will be less than one inch. Differential settlement, between adjacent foundations, should be approximately one-half of the total settlement movement. The majority of the settlement will occur during the construction period.

3.2 FLOOR SLAB RECOMMENDATIONS

We recommend that the procedures described in **Section 4.0** of this report be used to prepare the floor slab subgrade. Ground floor slabs can bear directly on top of compacted structural fill material. A modulus of subgrade reaction value of 150 pounds per cubic inch (pci) may be used for design.

To avoid potential moisture problems, we recommend that floor slab subgrade soils be covered with a vapor barrier (such as visqueen, normally 6 mil thick) prior to constructing the slab-ongrade floors. The floor slabs should be reinforced to make them as rigid as practical. Proper joints should be provided at the junctions of the slabs and foundation system so that a small amount of independent movement can occur without causing structural damage. An ultimate friction factor of 0.21 should be used for the vapor barrier-soil interface with an appropriate factor of safety.

3.3 STORMWATER POND

A stormwater pond is proposed for the subject project. The pond is planned at the south end of the property. The proposed pond should be designed and constructed in accordance with applicable Water Management District criteria.

Based on the borings performed in the vicinity of the proposed stormwater system and the results of our field permeability testing, we are recommending the parameters listed in **Table A** below be used to model the shallow effective aquifer in the system recovery analysis.

Table A:	Parameters	for Ponc	l Design

Parameter	Value	Unit
Coefficient of Horizontal Permeability	6	feet per day
Depth to SHWT	4	feet
Depth of Confining Layer	8	feet
Porosity of Shallow Aquifer	20	percent

Note: Depths referenced from existing grade at the time of PSI's field exploration program.



The recovery of a given stormwater system is dependent on the soil permeability as well as the groundwater table, pond bottom elevation, pond geometry, confining layer and water level in the system. We recommend a commercially available computer program such as PONDS or MODRET be used by an engineer experienced in groundwater modeling to evaluate the recovery potential of the proposed stormwater system. The system should be designed and constructed in accordance with Water Management District requirements. We recommend an appropriate safety factor be applied to the stormwater model.

4.0 SITE PREPARATION

Based on the results of our field exploration, we anticipate site preparation procedures to include the steps listed below. All work should be carried out in accordance with current regulatory criteria with the site preparation work and construction activities being carried out with care so as not to impact the adjacent existing construction that is to remain. The earthwork observation and testing required herein should be performed under the supervision of PSI personnel.

- 1. If not already done, provisions should be made to relocate any interfering utility lines within the construction area. Excavations resulting from the removal of unwanted pipes should be infilled with suitable granular soils that are thoroughly compacted.
- 2. Site preparation for the proposed development should include clearing the vegetation and stripping and removal of the topsoil to expose clean granular soils. All unwanted ground cover should be completely removed from the site and properly disposed of. This work should be carried out in accordance with current regulatory criteria.
- 3. The cleared exposed subgrade should be densified using a self-propelled vibratory roller which imparts a dynamic force of not less than 10 tons. Densification of the soils should be performed within the proposed development areas plus a 5-foot wide perimeter extending beyond the outside edge of the same, where practical. Densification operations should continue until the subgrade soils are firm and unyielding. Any area of the exposed surface that deflects excessively under the weight of the compaction equipment should be excavated approximately 24 inches and be replaced with compacted structural fill. Soils in this interval should be compacted to at least 95 percent of the Modified Proctor maximum dry density determined per ASTM D-1557.
- 4. Structural fill material may be composed of either clean sands or limerock. The fill should consist of an inorganic, non-plastic material, free of any man-made debris and limerock with a three inch maximum particle size. Proper control of the placement and compaction of new fills for the project should be exercised by a representative of the geotechnical engineer. The fill materials should be placed in lifts not exceeding 12 inches in loose thickness. Each lift should be compacted to at least 95 percent of the Modified Proctor maximum dry density near the optimum moisture content as determined by ASTM D-1557. Fill to be compacted with a vibratory plate tamper or a small walk behind vibratory roller should be placed in lifts not exceeding six inches in loose thickness.



- A) The structural fill to be used above the water table should have a Unified Soil Classification System designation of GP, GW, SP, SW, GP-GM, GW-GM, SW-SM, or SP-SM containing less than 12 percent material passing the No. 200 sieve.
- B) The structural fill or backfill to be placed below the water table and to a height of one foot above it should consist of a combination of FDOT 57 Stone and structural fill material mixed in an approximate 50% proportion by volume. Once above the water table, the exposed FDOT 57 Stone layer should be systematically stabilized using a plate compactor. Density testing will not be required within this layer, however the subgrade preparation work should be observed by a representative from our office to confirm that the material is in a stable and unyielding condition.
- C) The use of a commercially available fill material by the name "Cyclone sand" should not be permitted for the project. Cyclone sand contains large amounts of fines and is therefore very sensitive to moisture. The moisture sensitivity of the material makes it difficult to compact and achieve the desired densities.
- 5. The footings for the proposed structure should be placed on compacted approved structural fill material or densified native soils.
 - A) It is recommended that the soils exposed at the bottom of the footing excavations be compacted to at least 95 percent of the Modified Proctor maximum dry density just before pouring concrete. If the footing bearing materials become disturbed due to surface water resulting from precipitation and runoff, the disturbed soils should be overexcavated and replaced with compacted limerock which is densified to at least 95 percent of the materials Modified Proctor maximum dry density as determined by ASTM designation D-1557.
 - B) All open foundation excavations should be observed and approved by a licensed geotechnical engineer or his representative prior to pouring concrete.
- 6. Groundwater control may be required at this site, if required for either excavation dewatering or removal of temporarily perched water from a rain event. Such water can be controlled by pumping from sumps located in ditches or pits. Groundwater should be maintained at least one foot below the bottom of any excavation made during construction operations, or, at least two feet below the surface of any compaction operations.
- 7. It is mandated by federal regulations that all excavations, whether they be utility trenches, basement excavations or footing excavations, be constructed in accordance with OSHA guidelines. It is our understanding that these regulations are being strictly enforced and if they are not closely followed, the owner and the contractor could be liable for substantial penalties.



5.0 PAVEMENT SECTION SUGGESTIONS

Flexible pavement sections in this geographic area typically consist of an asphaltic concrete wearing course, a limerock base course and a stabilized subgrade (sub-base). Based on the expected traffic loading and our experience in the area, the typical pavement section thicknesses noted in **Table B** shown on the following page are considered suitable for this project.

Type of	Layer	Material Description	Layer Thick	ness (inches)	
Pavement			Light Duty	Medium Duty	
	(A)	Florida DOT Asphalt Type S	1.5	2.5	
Flexible	(B)	Crushed limerock with a minimum LBR of 100, compacted to 98% of the Modified Proctor maximum dry density	6.0	8.0	
Flexible	(SB)	Stabilized sub-grade (sub-base) fill with a minimum LBR of 40 compacted to 95% of the Modified Proctor maximum dry density	12.0	12.0	
	(C)	Florida DOT Portland Cement Concrete	5.0	6.0	
Rigid	(SB)	Granular subgrade compacted to 95% or more of the Modified Proctor maximum dry density	12.0	12.0	
(A) = Asphat(C) = Concre		te, (B) = Base Course, (SB) = Stabilized Su	b-grade (Sub-t	base)	
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Table B: Typical Pavement Section Suggestions

The base course materials in the pavements should consist of crushed limerock having a minimum Limerock Bearing Ratio (LBR) of 100. Base materials should meet the requirements presented in the latest revisions of the Florida Department of Transportation "Specifications for Road and Bridge Construction", Section 911 (limestone). The base course should be compacted to at least 98 percent of the material's maximum dry density (ASTM D-1557). The subgrade should be stabilized to a depth of 12 inches to achieve a minimum LBR of 40. This can be obtained by blending base material (limerock) with the existing subgrade soils. The required mixing ratio should be determined by laboratory testing. The stabilized subgrade should be compacted to at least 95 percent of the maximum dry density (ASTM D-1557).

Where dumpsters are to be parked on the pavement, so that considerable load is transferred from relatively small steel supports, it is recommended that rigid concrete pavement be constructed. In addition, in areas utilized for loading and unloading heavy duty-trucks considerations should also be given to using a rigid pavement. A minimum portland concrete pavement thickness of 5 inches should be used in parking areas (light duty) and 6 inches in loading areas (medium duty), if rigid pavements are to be employed. The subgrade soils below concrete pavements should be compacted to a minimum density of 95% of the modified Proctor maximum dry density (ASTM D-1557). The LBR requirement for the subgrade soils is only required for the "flexible" pavement section.



The concrete should be reinforced as necessary to withstand the traffic loadings anticipated and jointed to reduce the potential for crack development. The minimum rigid pavement thickness recommended herein is based upon concrete with an unconfined compressive strength of 4,000 pounds per square inch (psi) and a modulus of rupture of 450 psi. Fill that may be required to raise grades in pavement areas should be compacted to at least 95 percent of the material's maximum dry density (ASTM D-1557).

Finally, we recommend that a minimum separation of at least two feet be maintained between the bottom of the base and the estimated seasonal high groundwater table levels.

Actual pavement section thicknesses and the reinforcement details for the rigid pavement section should be provided by the Design Civil Engineer based on traffic loads, volume, and the owner's design life requirements. The noted sections represent minimum thicknesses for typical local construction practices and, as such, periodic maintenance should be anticipated. All pavement materials and construction procedures should conform to FDOT, American Concrete Institute (ACI), or appropriate city/county requirements.

6.0 REPORT LIMITATIONS

Our professional services have been performed, findings obtained, and recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices at the time of this report. This company is not responsible for the conclusions, opinions or recommendations made by others based on this data. No other warranties are implied or expressed. After the plans and specifications are complete, it is recommended that PSI be provided the opportunity to review the final design and specifications, in order to verify that the earthwork and recommendations are properly interpreted and implemented. At that time, it may be necessary to submit supplemental recommendations.

The scope of investigation was intended to evaluate soil conditions within the influence of the proposed shallow foundations. The analyses and recommendations submitted in this report are based upon the data obtained from the soil borings performed at the locations indicated. If any subsoil variations become evident during the course of this project, a re-evaluation of the recommendations contained in this report will be necessary after we have had an opportunity to observe the characteristics of the conditions encountered. The applicability of the report should also be reviewed in the event significant changes occur in the design, nature or location of the project.

The scope of our services did not include a formal environmental assessment for the presence or absence of hazardous or toxic materials in the soil and groundwater. Any statements in this report regarding odors, staining of soils, or other unusual conditions observed are strictly for the information of our client.

This report has been prepared for the exclusive use of Hanlex Development, LLC and their design consultants, for the specific application to the design and construction of the proposed Dollar General store in Margate, Broward County, Florida.



7.0 CLOSURE

We trust this report is adequate for your current needs; however, should you have any questions or should additional information be required, please do not hesitate to contact our office at (305) 471-7725.

Respectfully submitted,

Professional Service Industries, Inc. Certificate of Authorization No: 3684

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lan Kinnear, P.E. Chief Engineer FL. License No. 32614

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Dhuruva (Dru) Badri, P.E. Department Manager FL. License No. 68718

cc: Addressee (3 and PDF) PSI NCG, Attention: Ms. Teri Hebner, P.E. (PDF) PSI NCG, Attention: Ms. Lindsey Wright (PDF) File (1 and PDF)

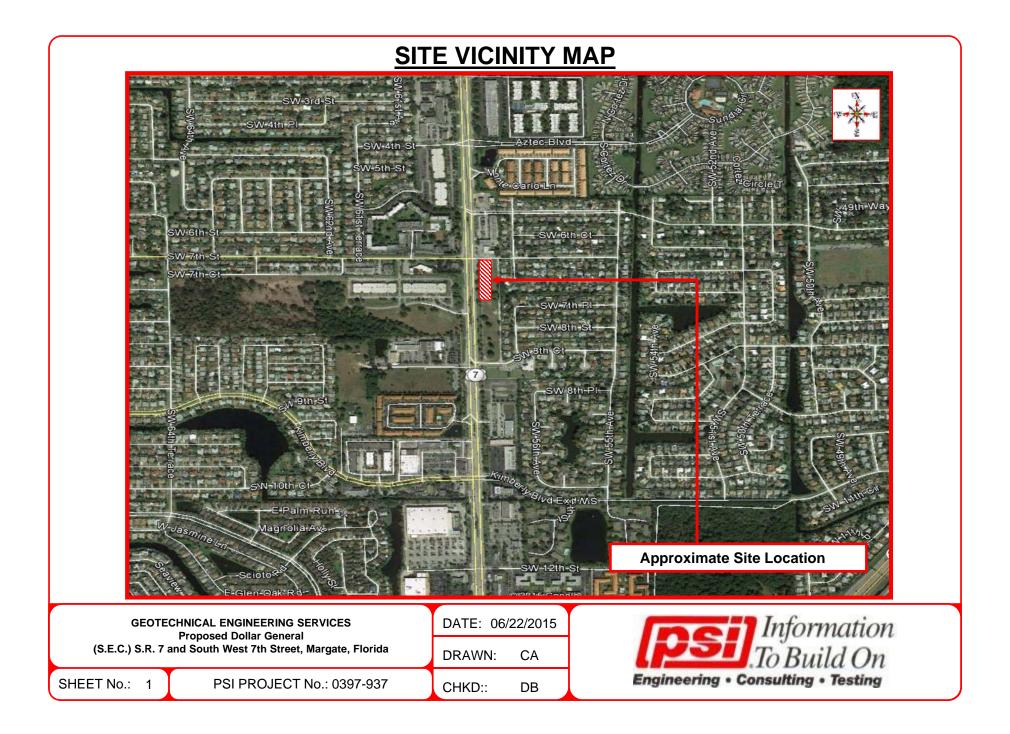
APPENDIX

Sheet 1:	Site Vicinity Map
Sheet 2:	Site Photographs
Sheet 3:	Boring Location Plan
	Boring Logs
Table 1:	Summary of Percolation Test Result
	Schematic of Usual Open-Hole Percolation Test

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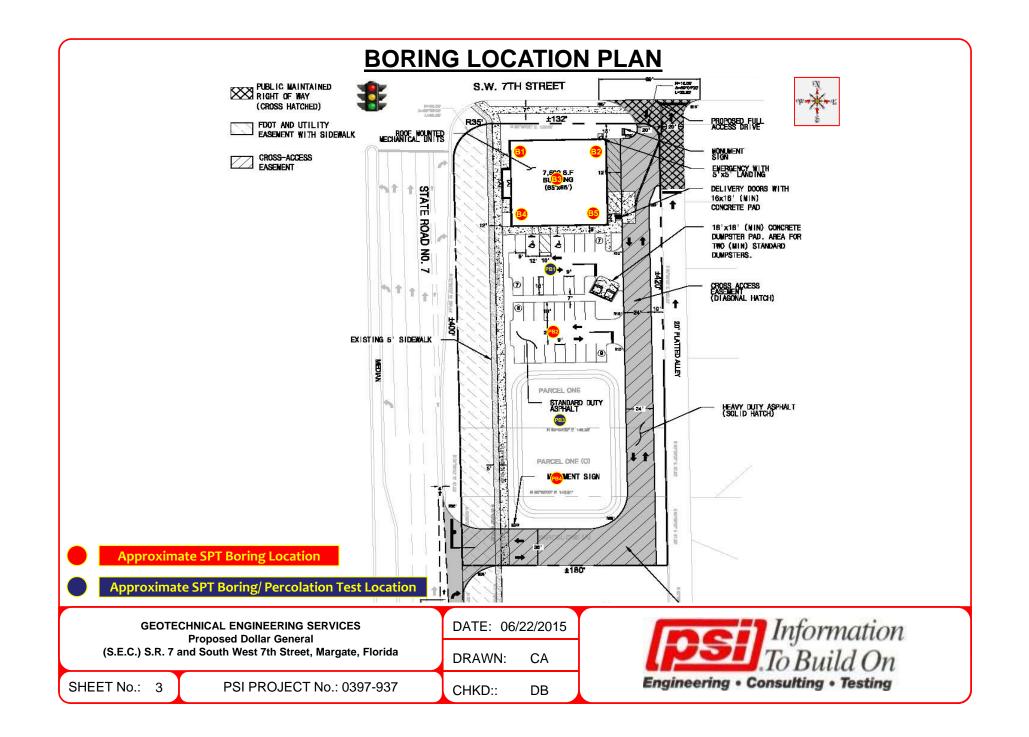


APPENDIX



SITE PHOTOGRAPHS

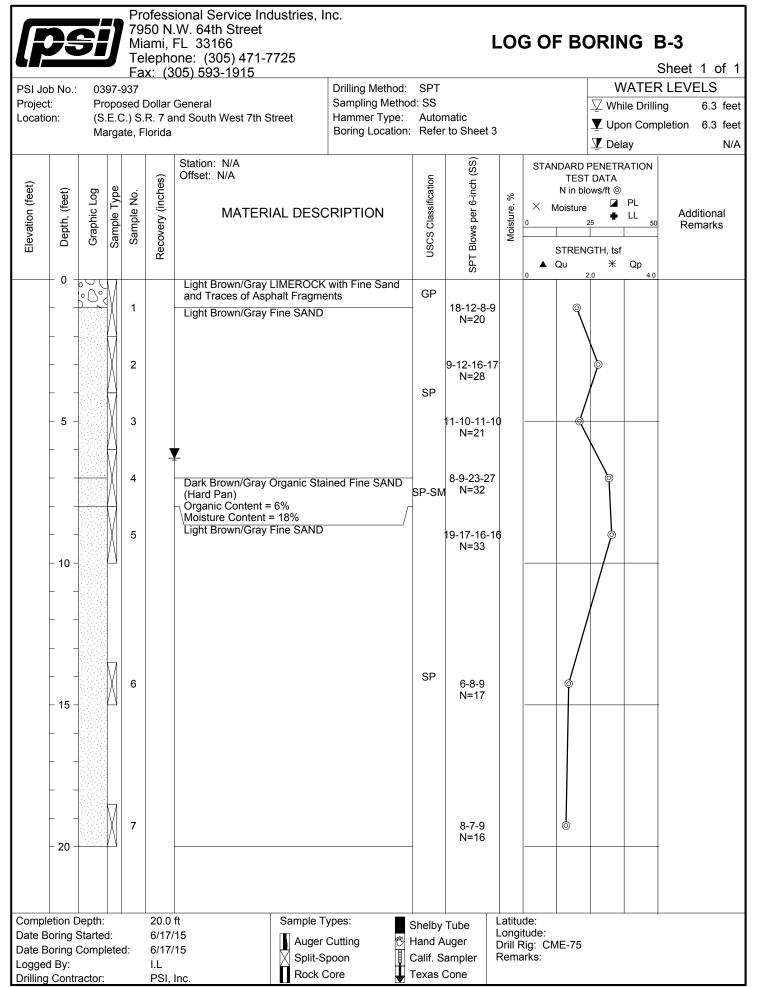




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TABLE 1: SUMMARY OF PERCOLATION TEST RESULTS PROPOSED DOLLAR GENERAL - BROWARD COUNTY, FL (S.E.C.) S.R. 7 AND S.W. 7TH STREET MARGATE, FLORIDA PSI PROJECT No.: 0397-937

Test	Date	Diam	eter	Depth of	Depth to Grou	ndwater Level	Hydraulic	Saturated Hole	Average	K, Hydraulic
No.	Performed	Casing	Perforated PVC	Hole	Below Ground Surface (Feet)		Head, H2	Depth, Ds	Flow Rate, Q	Conductivity
		(Inches)	(Inches)	(Feet)	Prior to Test	During Test	(Feet)	(Feet)	(gpm)	cfs/ft ² -ft
PB-1	17-Jun-15	6	4	10.0	6.6	0.0	6.6	3.4	2.3	7.1E-05
PB-3	17-Jun-15	6	4	10.0	6.3	0.0	6.3	3.7	2.2	7.0E-05

Note:

(1) The above hydraulic conductivity values are for a french drain installed to the same depth as the borehole tests. The values represent an ultimate value. The designer should apply the appropriate factor of safety.

(2) The hydraulic conductivity values were calculated based on the South Florida Water Management District's USUAL OPEN HOLE CONSTANT HEAD percolation test procedure as shown on the following page.

(3) A diameter of six inches was used in the computation of the Hydraulic Conductivity value presented in the above table.



H_I = AVERAGE HEAD ON UNSATURATED HOLE SURFACE (FT.HEAD)

ELEV. "A"= PROPOSED TRENCH BOTTOM ELEV.

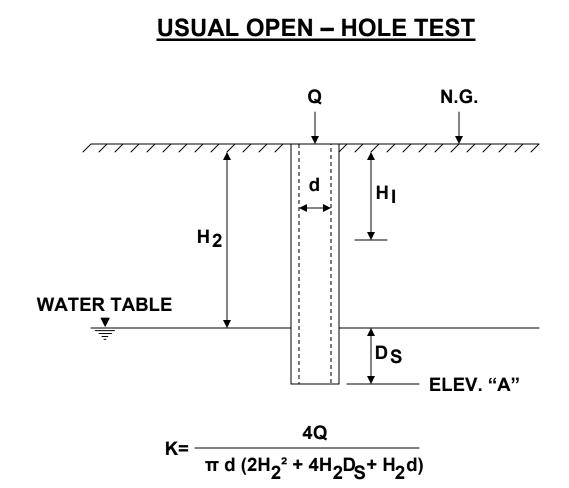
D_S = SATURATED HOLE DEPTH (FEET)

H₂ = DEPTH TO WATER TABLE (FEET)

d= DIAMETER OF TEST HOLE (FEET)

Q= "STABILIZED" FLOW RATE (CFS)

K= HYDRAULIC CONDUCTIVITY (CFS/FT.² - FT.HEAD)

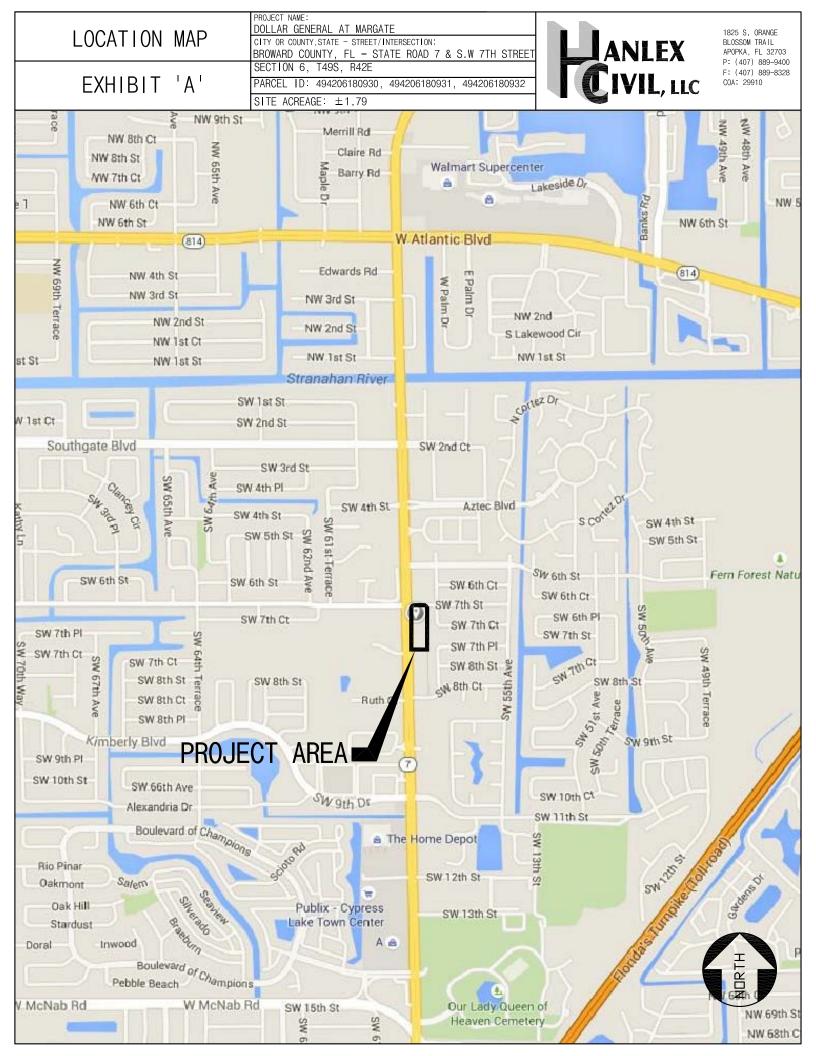


6. District Maintenance Acknowledgement

I, Hanlex Development, LLC, acknowledge the stormwater maintenance requirements established by the Florida Department of Environmental Protection (FDEP) including, but not limited to, (a) Removal of trash and debris, (b) Inspection of inlets and outlets, (c) Removal of sediments or vegetation when the storage volume or conveyance capacity of the stormwater management system is below design levels, and (d) Stabilization and restoration of eroded areas. As the owner of the project, I agree to adhere to said maintenance requirements for the proposed development.

7. Appendix

A. LOCATION MAP



B. USDA SOILS MAP



HYDROLOGIC SOILS	PROJECT NAME: DOLLAR GENERAL AT MARGATE	1825 S. ORANGE
GROUP	CITY OR COUNTY, STATE - STREET/INTERSECTION: BROWARD COUNTY, FL - STATE ROAD 7 & S.W 7TH STREET	ANLEX BLOSSOM TRAIL APOPKA, FL 32703 DY (M72) SPO
EXHIBIT 'B2'	SECTION 6, T49S, R42E PARCEL ID: 494206180930, 494206180931, 494206180932	P: (407) 889-9400 F: (407) 889-8328 COA: 29910
	SITE ACREAGE: ±1.79	

Hydrologic Soil Group

Hydrologic Soil Group— Summary by Map Unit — Broward County, Florida, East Part (FL606)										
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI						
15	Immokalee fine sand, 0 to 2 percent slopes	B/D	2.7	100.0%						
Totals for Area of Inter	est		2.7	100.0%						

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

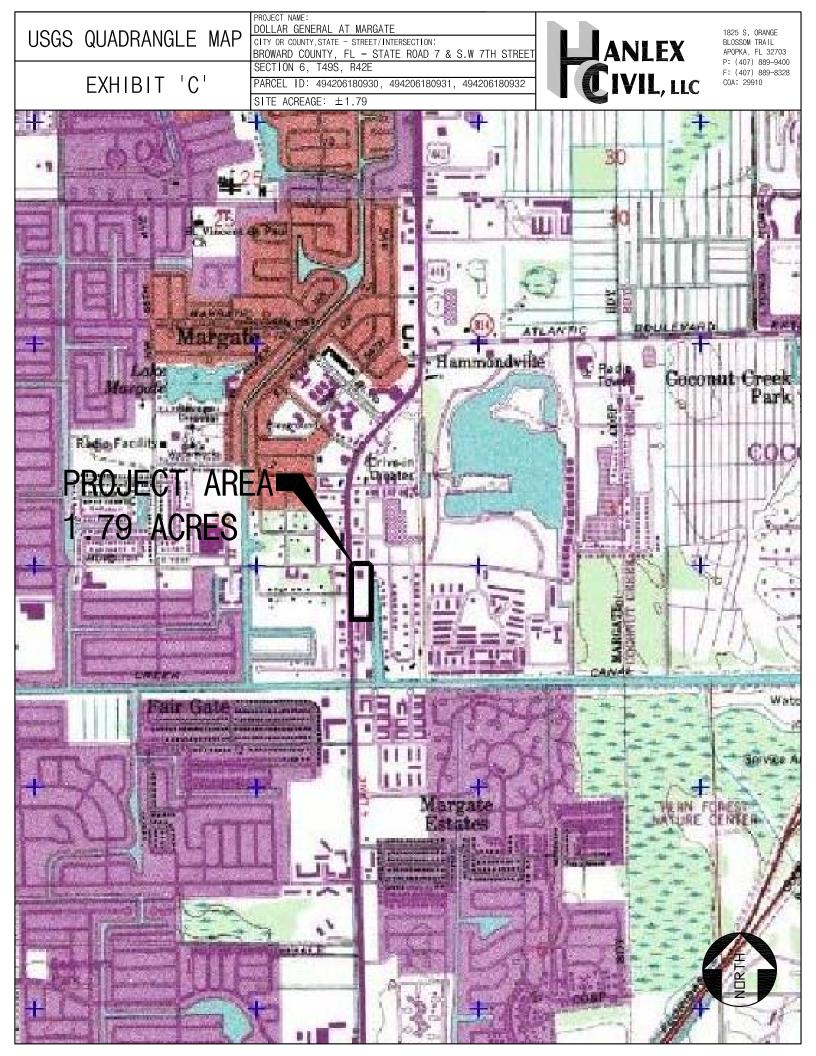
Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified

C. USGS Quad Maps



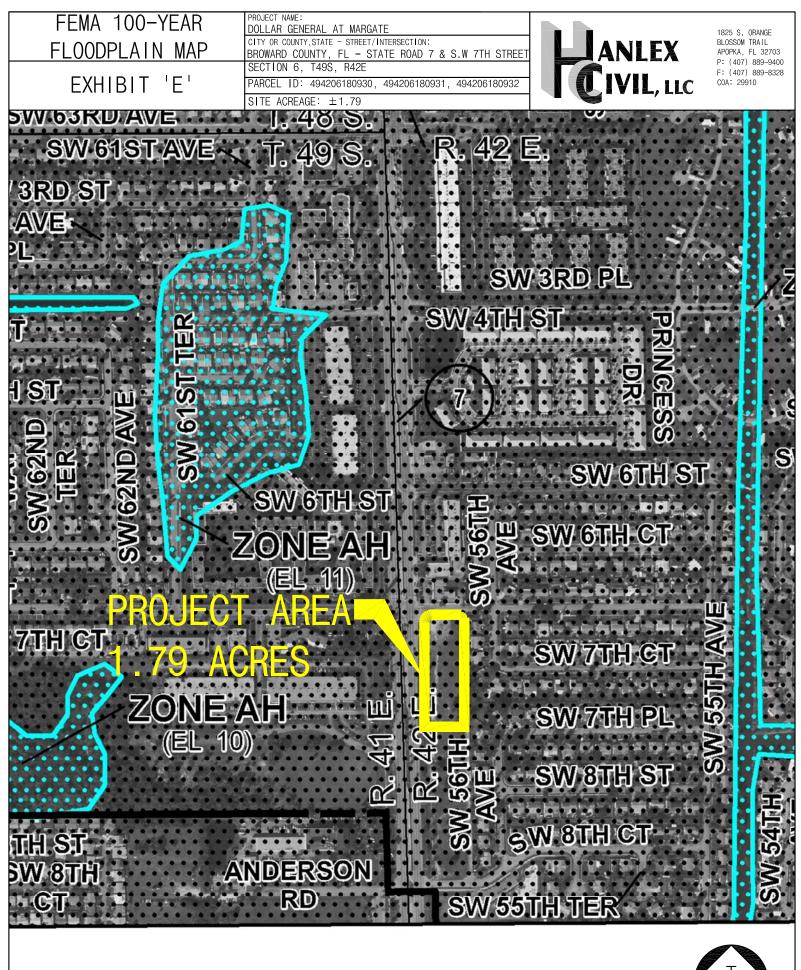
D. AERIAL MAP

AERIAL PHOTOGRAPH	PROJECT NAME: DOLLAR GENERAL AT MARGATE CITY OR COUNTY, STATE - STREET/INTERSECTION: BROWARD COUNTY, FL - STATE ROAD 7 & S.W 7TH STREET	1825 S. ORANGE BLOSSOM TRAIL APOPKA, FL 32703 D: (VG7) DB0 0400
EXHIBIT 'D'	SECTION 6, T49S, R42E PARCEL ID: 494206180930, 494206180931, 494206180932 SITE ACREAGE: ±1.79	P: (407) 889-9400 F: (407) 889-8328 COA: 29910



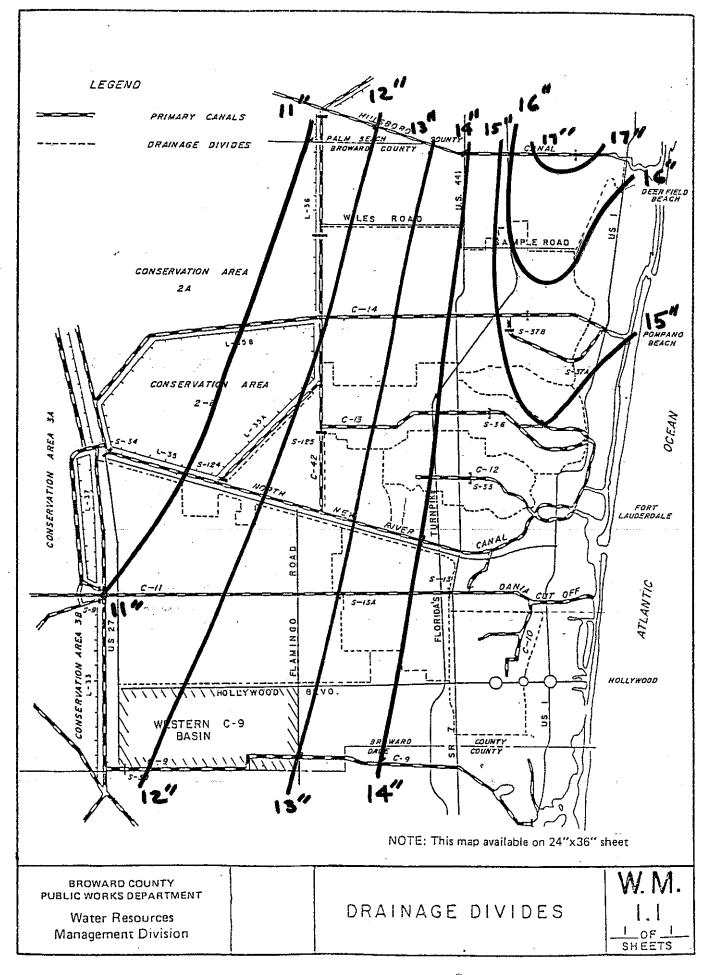


E. FEMA FLOOD INSURANCE RATE MAP





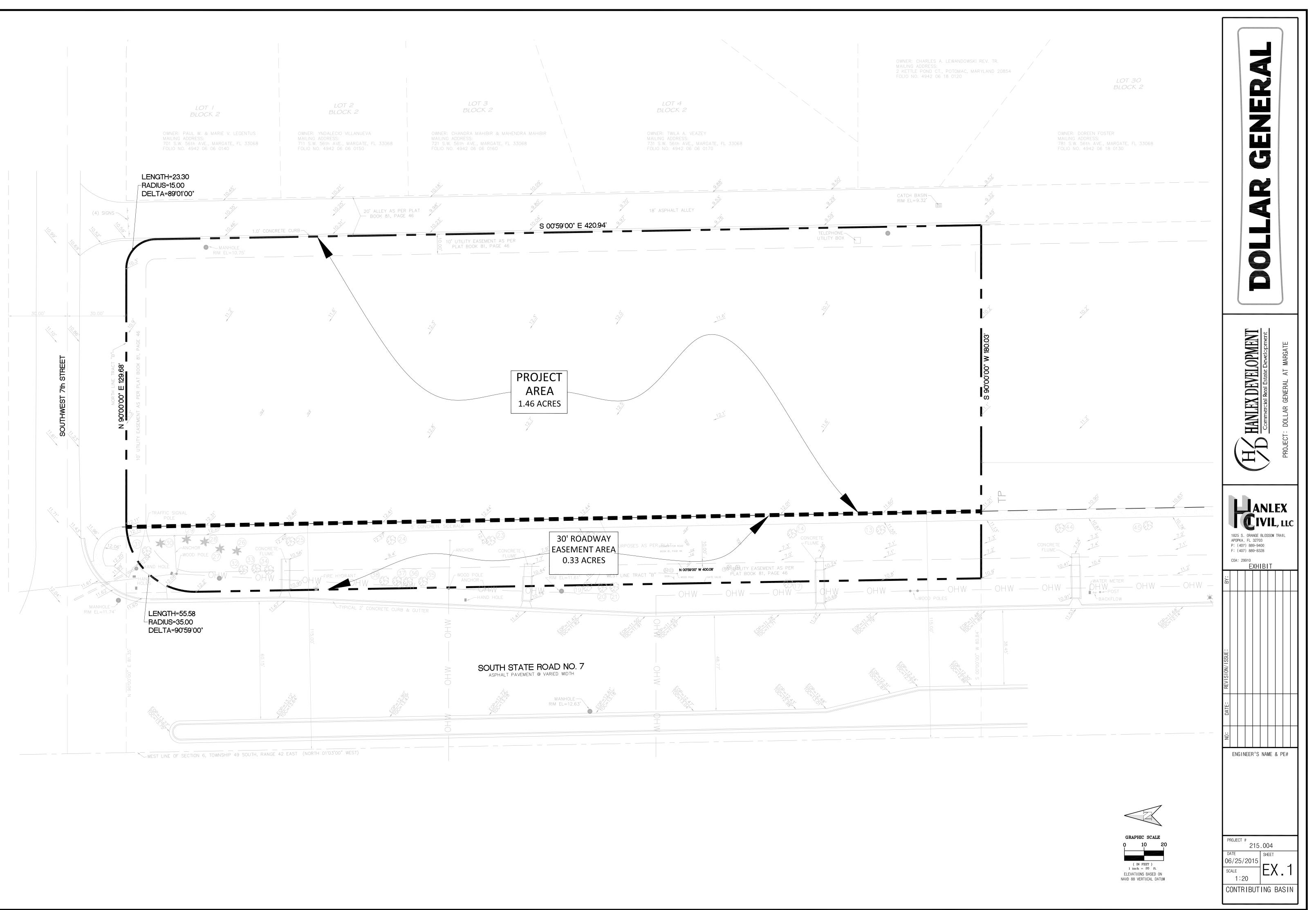
F. RAINFALL MAPS



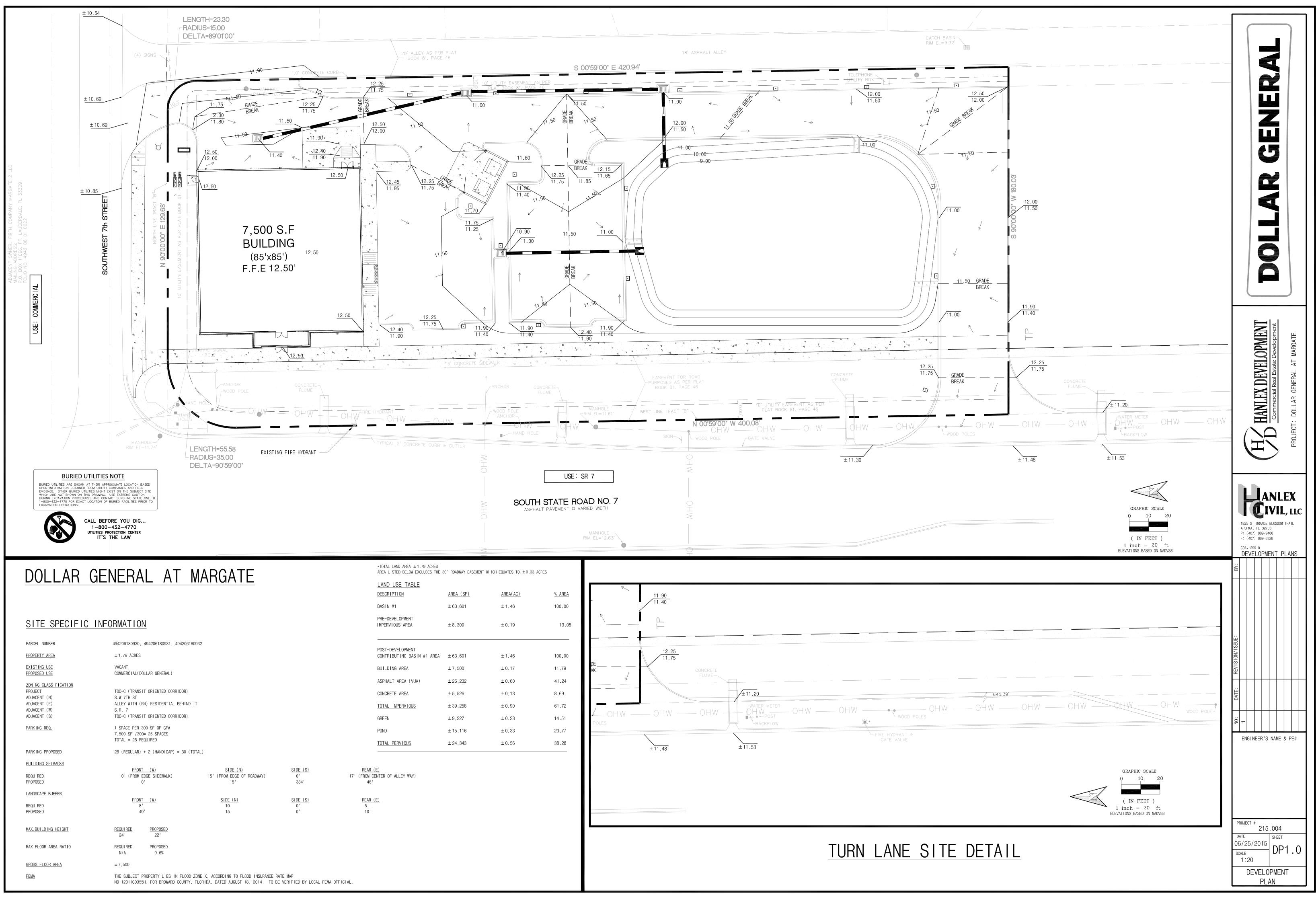
252995001⁵⁰0429566955550000044

2 SUR-3D

G.PRE-BASIN MAP



H. POST-BASIN MAP



I. CALCULATIONS AND RESULTS

a.) Pre-Development

Pre-Development Site Summary for Dollar General Margate

1.10 acre-ft

9 inches

Pre-Development		Soil Storage Per Browa	rd County Flatwoods Soil
Total Site/Pervious Area	1.46 Acres	Depth To Water Table	Cumulative Water Storage
		1	0.6"
Soil Storage Calculations		2	2.5"
		3	5.4"
Wet Season Water El	7.6 Feet	4	9.0"
Average Groundwater El	7.6 Feet	*Reduced by 25% if it is compac	ted
Average Site El	11.6 Feet		
Depth to water table	4.0 Feet	(4' Max allowed)	
Available Ground Storage		9 inches	

Available Ground Storage Storage Available in pervious area Converting to site wide moisture storage, S

Flood Stage Criteria

Q =	$(P - (0.2 * S))^2$				
¥ -	(P + (0.8 * S))				

100 Year 3 Day Storm

P =	18 inches
S =	9 inches
Q =	10.414 inches

Volume 1.267 acre-ft Stage 12.47 Feet

*The Stage is calculated by dividing the volume by the Site area as the pre-development condition is an undeveloped parcel

25 Year 3 Day Storm

P =	14 inches
S =	9 inches
Q =	7.021 inches

Volume0.854 acre-ftStage12.19 Feet

*The Stage is calculated by dividing the volume by the Site area, as the pre-development condition is an undeveloped parcel

5 Year 24 Hour Storm

P = S =	7 inches 9 inches
Q =	1.904 inches
Volume	0.232 acre-ft
Stage	11.76 Feet

*The Stage is calculated by dividing the volume by the Site area, as the pre-development condition is an undeveloped parcel

I. CALCULATIONS AND RESULTS

b.) Post-Development

Post-Development Site Summary for Dollar General Margate

Pervious Area	0.57 Acres	Soil Storage Per Broward County Flatwoods Soil		
Total Site Area	1.46 Acres	Depth To Water Table	Cumulative Water Storage	
		1	0.6"	
Soil Storage Calculations		2	2.5"	
		3	5.4"	
Wet Season Water El	7.5	4	9.0"	
Average Groundwater El	7.5	*Reduced by 25% if it is compace	cted	
Average Site El	11.5			

4.0 Feet

Available Ground Storage, Flatwoods Soils Storage Available in pervious area Converting to site wide moisture storage, S

Flood Stage Criteria

Depth to water table

$$Q = \frac{(P - (0.2 * S))^2}{(P + (0.8 * S))}$$

100 Year 3 Day Storm

P =	18 inches		
S =	2.61 inches		
Q =	15.20 inches		
Volume	1.850 acre-ft		
Stage	12.00 Feet		
*The Stage was interpolated based off the Stage-Storage chart and graph			

25 Year 3 Day Storm

P =	14 inches		
S =	2.61 inches		
Q =	11.29 inches		
Volume	1.374 acre-ft		
Stage	11.72 Feet		
*The Stage was interpolated based off the Stage-Storage chart and graph			

5 Year 24 Hour Storm

P =	7 inches		
S =	2.61 inches		
Q =	4.62 inches		
Volume	0.562 acre-ft		
Stage	10.75 Feet		
*The Stage was interpolated based off the Stage-Storage chart and graph			

(4' Max allowed)

6.75	inches
0.32	acre-ft
2.61	inches

Dollar General at Margate						
	Pre-Development Vs. Post Development Areas					
		Pre-Devel	lopment Cond	ditions		
Description	<u>Total Area</u> (sf)	<u>Total Area</u> (sf)	<u>Total Area</u> (ac)	<u>Percent</u>		
Total Site area	63,601	63,601	1.46	100.00%		
Building Area	0					
Asphalt Pavement	0	8,300	0.19	13.05%		
Concrete/Sidewalk	8,300					
Green Area	55,301	55,301	1.27	86.95%		
		Post Deve	lopment Con	ditions		
Description	<u>Total Area</u> (sf)	<u>Total Area</u> (sf)	<u>Total Area</u> (ac)	<u>Percent</u>		
Basin Area	63,601	63,601	1.46	100.00%		
Building Area	7,500					
Asphalt Pavement	26,232	39,258	0.90	61.73%		
Concrete/Sidewalk	5,526					
Pond Area	15,116	15,116	0.35	23.77%		
Green Area	9,227	9,227	0.21	14.51%		

Dollar General at Margate Post-Development Stage-Storage							
Storage Storage Storage Storage Storage Storage Storage Storage						Total Storage (Ac-Ft)	
9.0	10,766	0.0	0.0	0.0	0.0	353.0	0.01
10.0	12,430	12,430.0	0.0	0.0	0.0	353.0	0.30
11.0	15,116	15,116.0	0.0	0.0	77.1	77.1	0.65
11.5	15,116	7,558.0	18,006.8	18,006.8	3,032.0	1,516.0	1.27
12.00	15,116	7,558.0	26,232.0	13,116.0	9,227.0	4,613.5	1.85
12.50	15,117	7,558.5	26,232.0	13,116.0	10,027.0	5,013.5	2.44

Table 2

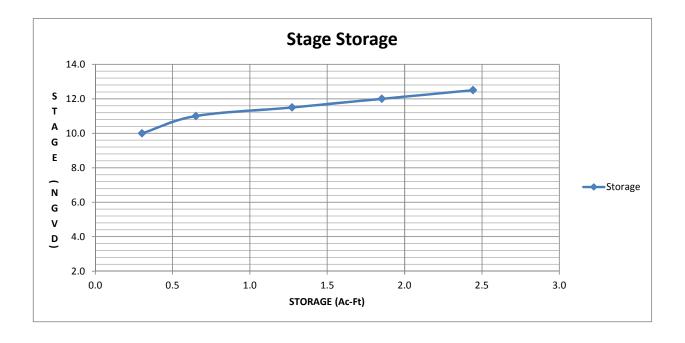
*Site (Green Area/Sidewalks)

 $\ast\ast$ Total Storage Starts with total Trench Storage for the storm water pipes system.

Site Summary for Dollar General Margate

Perimeter Berm Summary Table (NGVD)								
Storm	Pre-Development		Post-Development					
	Volume (Ac-	Stage	Volume (Ac-	Stage	Post < Pre Stage			
	Ft)	(NGVD, Ft)	Ft)	(NGVD, Ft)				
25 Year 72 Hr	0.85	12.19	1.37	11.72	YES			
Post Development Perimeter Berm at Elevation 11.75 (NGVD)								

Finished Floor Elevation Summary Table (NGVD)									
Storm	Pre-Development		Post-Development		FFE Set Above FEMA Flood				
	Volume (Ac-	Stage	Volume (Ac-	Stage	Elevation of 10				
	Ft)	(NGVD, Ft)	Ft)	(NGVD, Ft)					
100 Year 72 Hr	1.27	12.47	1.85	12.00	12.5				



I. CALCULATIONS AND RESULTS

c.) Treatment Volume Per SFWMD Guidelines

Project Data

Project Name:	Dollar General At Margate
Simulation Description:	Recovery Analysis
Project Number:	215.004
Engineer :	
Supervising Engineer:	Jason Bullard
Date:	06-26-2015

Aquifer Data

Base Of Aquifer Elevation, [B] (ft datum):	5.60
Water Table Elevation, [WT] (ft datum):	7.60
Horizontal Saturated Hydraulic Conductivity, [Kh] (ft/day):	6.00
Fillable Porosity, [n] (%):	20.00
Unsaturated Vertical Infiltration Rate, [lv] (ft/day):	3.0
Maximum Area For Unsaturated Infiltration, [Av] (ft2):	15116.0

Geometry Data

Equivalent Pond Length, [L] (ft):	190.0	
Equivalent Pond Width, [W] (ft):	80.0	

Ground water mound is expected to intersect the pond bottom

Stage vs Area Data

Stage (ft datum)	Area (ft²)
9.00	10766.0
10.00	12430.0
11.00	15116.0

Scenario Input Data

Scenario 1 :: Recovery Analysis

Hydrograph Type:	Slug Load
Modflow Routing:	Routed with infiltration
Treatment Volume (ft3)	8179

Initial ground water level (ft datum) 7.60 (default)

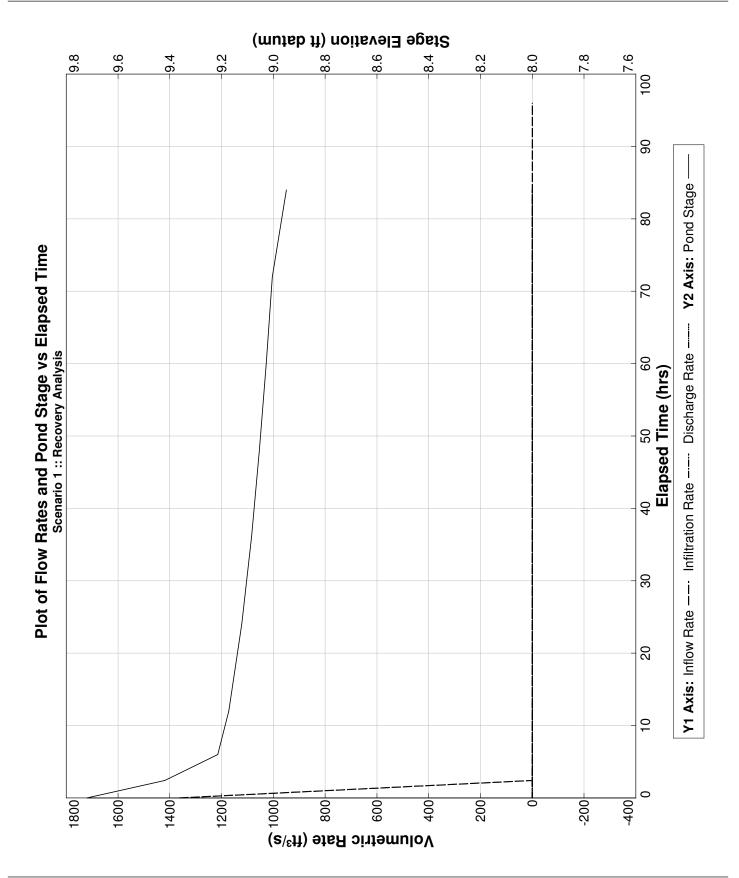
Time After Storm Event	Time After Storm Event
(days)	(days)
0.100	2.000
0.250	2.500
0.500	3.000
1.000	3.500
1.500	4.000

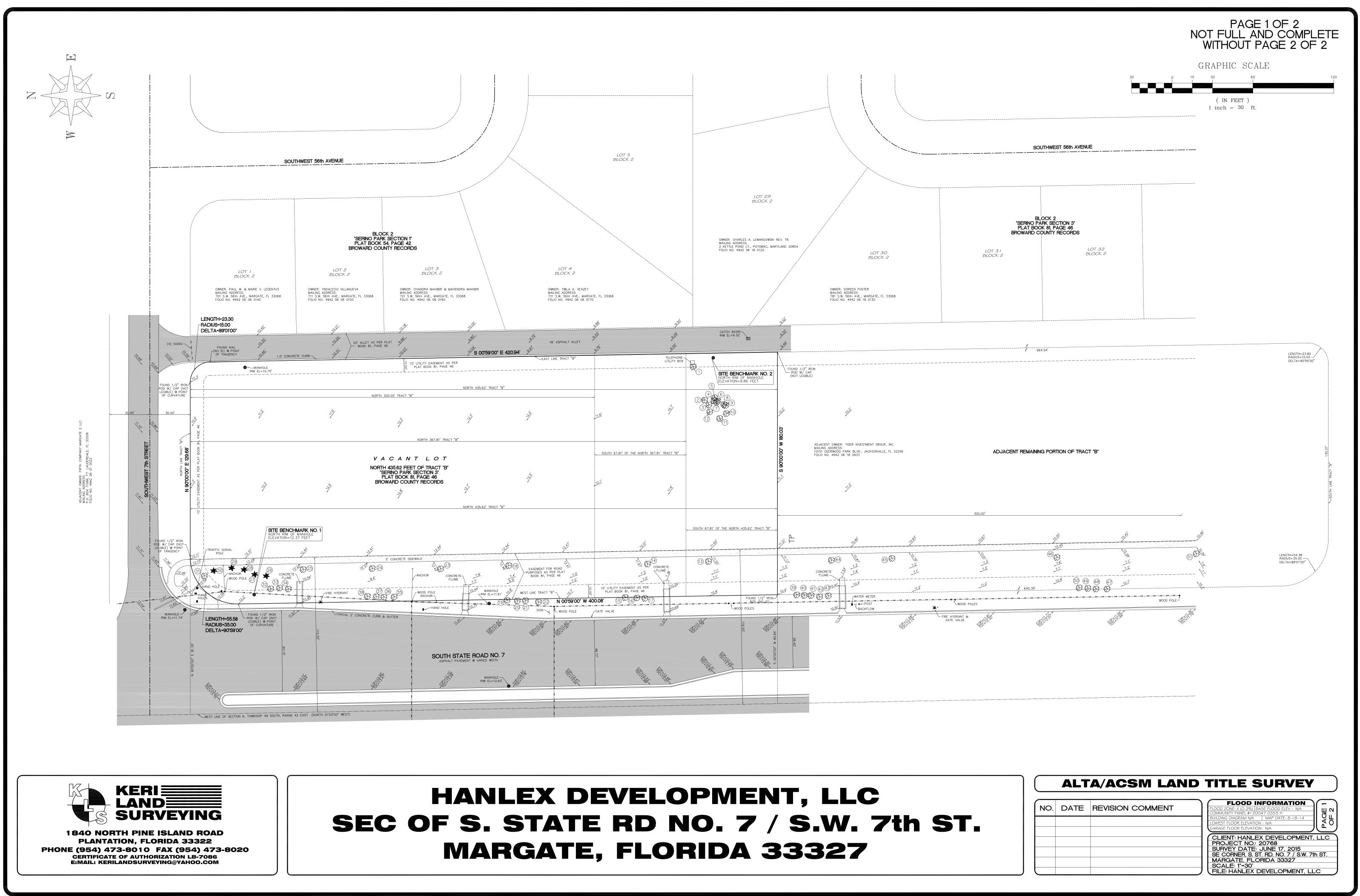
PONDS Version 3.3.0265 Retention Pond Recovery - Refined Method Copyright 2012 Devo Seereeram, Ph.D., P.E.

Summary of Results :: Scenario 1 :: Recovery Analysis

	Time (hours)	Stage (ft datum)	Rate (ft ³ /s)	Volume (ft³)
Stage				
Minimum Maximum	0.000 0.002	7.60 9.72		
nflow				
Rate - Maximum - Positive	0.002		1363.1670	
Rate - Maximum - Negative Cumulative Volume - Maximum Positive	None 0.002		None	8179.0
Cumulative Volume - Maximum Positive	None			None
Cumulative Volume - End of Simulation	96.000			8179.0
nfiltration				
Rate - Maximum - Positive	0.002		0.4154	
Rate - Maximum - Negative	None		None	0170.0
Cumulative Volume - Maximum Positive	84.000 None			8179.0 None
Cumulative Volume - Maximum Negative Cumulative Volume - End of Simulation	96.000			8179.0
	30.000			0179.0
Combined Discharge	N I		N I	
Rate - Maximum - Positive Rate - Maximum - Negative	None None		None None	
Cumulative Volume - Maximum Positive	None		None	None
Cumulative Volume - Maximum Volume	None			None
Cumulative Volume - End of Simulation	96.000			0.0
Discharge Structure 1 - inactive				
Rate - Maximum - Positive	disabled		disabled	
Rate - Maximum - Negative	disabled		disabled	
Cumulative Volume - Maximum Positive	disabled			disabled
Cumulative Volume - Maximum Negative Cumulative Volume - End of Simulation	disabled disabled			disabled disabled
Discharge Structure 2 - inactive				
Rate - Maximum - Positive	disabled		disabled	
Rate - Maximum - Negative	disabled		disabled	
Cumulative Volume - Maximum Positive	disabled			disabled
Cumulative Volume - Maximum Negative Cumulative Volume - End of Simulation	disabled			disabled disabled
	disabled			disabled
Discharge Structure 3 - inactive Rate - Maximum - Positive	disabled		disabled	
Rate - Maximum - Negative	disabled		disabled	
Cumulative Volume - Maximum Positive	disabled		2.545104	disabled
Cumulative Volume - Maximum Negative	disabled			disabled
Cumulative Volume - End of Simulation	disabled			disabled
Pollution Abatement:	~~~~~			7000 7
36 Hour Stage and Infiltration Volume	36.000	9.08		7263.7
72 Hour Stage and Infiltration Volume	72.000	9.00		8133.3
		·\\		COVERS WITH
			72 HOURS	>

PONDS Version 3.3.0265 Retention Pond Recovery - Refined Method Copyright 2012 Devo Seereeram, Ph.D., P.E.





LEGAL DESCRIPTION:

PARCEL I:

THE SOUTH 67.81 FEET OF THE NORTH 367.81 FEET OF TRACT "B" OF "SERINO PARK SECTION 3" ACCORDING TO THE PLAT THEREOF AS RECORDED IN PLAT BOOK 81 AT PAGE 46 OF THE PUBLIC RECORDS OF BROWARD COUNTY, FLORIDA.

PARCEL II:

THE NORTH 300 FEET OF TRACT "B" OF "SERINO PARK SECTION 3" ACCORDING TO THE PLAT THEREOF AS RECORDED IN PLAT BOOK 81 AT PAGE 46 OF THE PUBLIC RECORDS OF BROWARD COUNTY, FLORIDA.

PARCEL III:

THE SOUTH 67.81 FEET OF THE NORTH 435.62 FEET OF TRACT "B" OF "SERINO PARK SECTION 3" ACCORDING TO THE PLAT THEREOF AS RECORDED IN PLAT BOOK 81 AT PAGE 46 OF THE PUBLIC RECORDS OF BROWARD COUNTY, FLORIDA.

NOTES

1) ELEVATIONS SHOWN HEREON ARE RELATIVE TO THE NORTH AMERICAN VERTICAL DATUM OF 1988. BASED ON BROWARD COUNTY BENCHMARK NO. 2543, ELEVATION = 13.050 FEET.

2) UNDERGROUND IMPROVEMENTS AND/OR UNDERGROUND ENCROACHMENTS NOT LOCATED OR SHOWN. 3) BEARINGS SHOWN HEREON ARE BASED ON THE NORTH

LINE OF TRACT "B" BEING DUE EAST. (AS PER RECORD PLAT) 4) ROOF OVERHANG NOT LOCATED UNLESS OTHERWISE

SHOWN. 5) LANDS SHOWN HEREON CONTAINING 78,105 SQUARE

FEET, (1.7930 ACRES), MORE OR LESS.

6) THIS SURVEY IS FOR BUILDING PERMITTING PURPOSES ONLY.

7) REASONABLE EFFORTS WERE MADE REGARDING THE EXISTENCE AND LOCATION OF UNDERGROUND UTILITIES. THIS FIRM, HOWEVER, DOES NOT ACCEPT RESPONSIBILITY FOR THIS INFORMATION. BEFORE EXCAVATION, CONTACT THE APPROPRIATE UTILITY COMPANIES FOR FIELD VERIFICATION.

8) THIS SURVEY IS CERTIFIED EXCLUSIVELY TO: HANLEX DEVELOPMENT, LLC, A FLORIDA LIMITED LIABILITY COMPANY OLD REPUBLIC NATIONAL TITLE INSURANCE COMPANY

9) THIS SITE LIES IN SECTION 6, TOWNSHIP 49 SOUTH, RANGE 42 EAST, BROWARD COUNTY, FLORIDA. 10) ALL BEARINGS/ANGLES AND DISTANCES SHOWN HEREON ARE BOTH MEASURED ON THE GROUND AND RECORD (AS PER PLAT) UNLESS OTHERWISE SHOWN. 11) — OHW — OHW — DENOTES OVERHEAD WIRES.

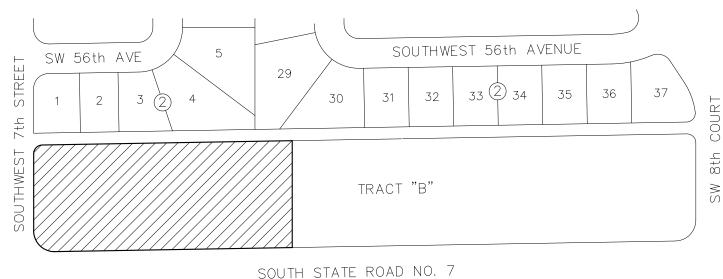
12) JON DENOTES EXISTING GRADE ELEVATION.

TITLE COMMITMENT NOTES:

THE SURVEY SHOWN HEREON WAS PREPARED WITH THE BENEFIT OF A TITLE COMMITMENT ISSUED BY OLD REPUBLIC NATIONAL TITLE INSURANCE COMPANY (AGENT FILE NO.: XXX-XXX, WITH AN EFFECTIVE DATE OF JUNE XX, 2015. ITEMS NUMBER 1 THROUGH XX OF SCHEDULE B-II WERE REVIEWED AND SHOWN HEREON WHEN APPLICABLE.







LOCATION SKETCH NOT TO SCALE

HEIGHT

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CANOPY

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		TREE TABLE
NO.	SPECIES	DIAMETER
Х	Х	Х
Х	Х	Х
Х	Х	Х
Х	Х	Х
Х	Х	Х
Х	Х	Х
Х	Х	Х
Х	Х	Х
Х	Х	X
Х	Х	X
Х	Х	Х
Х	Х	Х
Х	Х	X
Х	Х	Х
Х	Х	Х
Х	Х	X
Х	Х	X
Х	Х	X
Х	Х	X
Х	Х	X
Х	Х	X
Х	Х	X
Х	Х	X
Х	Х	X
Х	Х	X

HANLEX DEVELOPMENT, LLC S. STATE RD NO. 7 / S.W. 7th ST. MARGATE, FLORIDA 33327

NO.	DATE	REVISION COMMENT

PAGE 2 OF 2 NOT FULL AND COMPLETE WITHOUT PAGE 1 OF 2

	-	TREE TABLE		-
NO.	SPECIES	DIAMETER	HEIGHT	CANOPY
Х	Х	Х	Х	Х
Х	Х	X	Х	Х
X	Х	X	Х	Х
Х	Х	X	Х	Х
Х	Х	X	Х	Х
Х	Х	X	Х	Х
Х	Х	X	Х	Х
Х	Х	X	Х	Х
X	Х	X	Х	Х
X	Х	X	Х	Х
Х	Х	X	Х	Х
Х	Х	X	Х	Х
X	Х	X	Х	Х
Х	Х	X	Х	Х
Х	Х	X	Х	Х
Х	Х	X	Х	Х
X	Х	X	Х	Х
Х	Х	X	Х	Х
X	Х	X	Х	Х
X	Х	X	Х	Х
X	Х	X	Х	Х
X	Х	X	Х	Х
Х	Х	Х	Х	Х
X	Х	Х	Х	Х
Х	X	Х	Х	Х

ALTA/ACSM LAND TITLE SURVEY

CERTIFICATION ANLEX DEVELOPMENT, LLC, A FLORIDA LIMITED LIABILITY COMPANY, DLD REPUBLIC NATIONAL TITLE INSURANCE COMPANY, XXXX THIS IS TO CERTIFY THAT THIS MAP OR PLAT AND THE SURVEY ON WHICH I IS BASED WERE MADE IN ACCORDANCE WITH THE 2011 MINIMUM STANDARD DETAIL REQUIREMENTS FOR ALTAVACSM LAND TITLE SURVEYS. JOINTLY ESTABLISHED AND ADOPTED BY ALTA AND NSPS, AND INCLUDES ITEMS 1, 2, 3, 4, 7a, 8, 11a, 13 AND 14 OF TABLE A THEREOF. THE FIELD WORK WAS COMPLETED ON JULY 17, 2015.

> SIGNATURE DATE IAY KERI PROFESSIONAL SURVEYOR AND MAPPER NO. 5721 STATE OF FLORIDA

100 M		
FLOOD IN	FORMATION	<pre>N</pre>
FLOOD ZONE: X (0.2%) BA	SE FLOOD ELEV.: N/A	
COMMUNITY PANEL #1200	47 0355 H	
BUILDING DIAGRAM N/A	MAP DATE: 8-18-14	ΙΫ́́Ļ
LOWEST FLOOR ELEVATION	: N/A	⊴ 0
GARAGE FLOOR ELEVATION	I: N/A	(L ·
		April 19
CLIENT: HANLE	K DEVELOPMENT.	LLC
CLIENT: HANLE	<u>CDEVELOPMENT,</u> 20768	LLC
	20768	LLC
PROJECT NO.: 2 SURVEY DATE: 3	20768 JUNE 17, 2015	
PROJECT NO.: 2 SURVEY DATE: 3	20768 JUNE 17, 2015 7. RD. NO. 7 / S.W. 7t	

SCALE: 1"=30' FILE: HANLEX DEVELOPMENT, LLC

PROJECT DIRECTORY

OWNER:	TIGER INVESTMENT GROUP INC. 10151 DEERWOOD PARK BLVD JACKSONVILLE, FL 32256
DEVELOPER:	HANLEX MARGATE, LLC 1825 S. ORANGE BLOSSOM TRAIL APOPKA, FL 32703 PHONE: (407) 889-9400 FAX: (407) 889-8328
CIVIL ENGINEER:	HANLEX CIVIL, LLC 1825 S. ORANGE BLOSSOM TRAIL APOPKA, FL 32703 PHONE: (407) 889-4154 FAX: (407) 410-0351
GEOTECH ENGINEER:	PROFESSIONAL SERVICES INDUSTRIES, INC 7950 N.W. 64 STREET MIAMI, FLORIDA 33166 PHONE: (305) 471-7721 FAX: (305) 593-1915
SURVEYOR:	KERI LAND SURVEY 1840 NORTH PINE ISLAND ROAD PLANTATION, FL 33322 PHONE: (954) 473-8010 FAX: (954) 473-8020
ARCHITECT:	RABITS & ROMANO 245 SOUTH HIGHLAND STREET SUITE 9 MOUNT DORA, FL 32757 PHONE: (352) 385-1030 FAX: (352) 385-1035

	GEN	NERAL REVISION LOG	
NO:	DATE:	REVISION/ISSUE:	BY
1			
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UTILITY PROVIDERS

POWER:

WATER/SEWER:

DUKE ENERGY 452 EAST CROWN POINTE RD WINTER GARDEN, FL 33787 P: (407) 905-3321

BROWARD COUNTY OES 2300 W. COMMERCIAL ET LAUDERDALE EL

GAS:

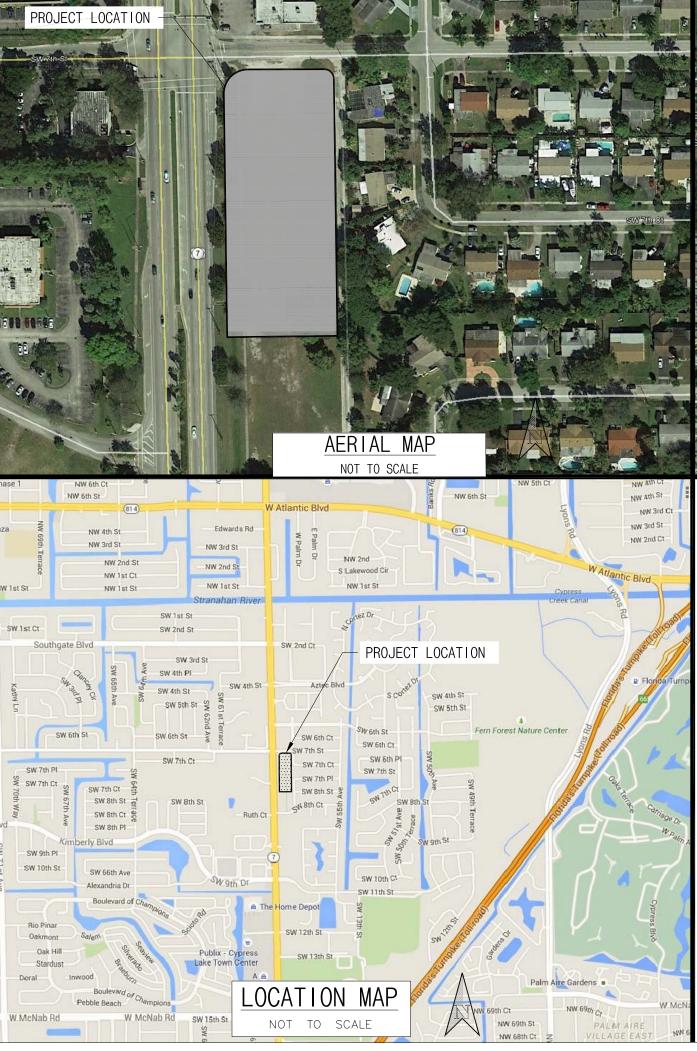
TELEPHONE:

2300 W. COMMERCIAL BOULEVARD FT. LAUDERDALE, FL 33309 P: (954) 847-2761

TECO PEOPLES GAS - SOUTH FLORIDA 5101 NW 21ST AVENUE, SUITE 460 FT. LAUDERDALE, FL 33309 P: (954) 453-0824

AT&T 8601 W. SUNRISE BOULEVARD PLANTATION, FL 33322 P: (954) 723-2540

DEVELOPMENT PLANS FOR HANLEX DEVELOPMENT, LLC DOLLAR GENERAL AT MARGATE



PARCEL:494206180930 494206180931 494206180932 MARGATE, FLORIDA 33068



LEGAL DESCRIPTION

PARCEL I:

THE SOUTH 67.81 FEET OF THE NORTH 367.81 FEET OF TRACT "B" OF "SERINO PARK SECTION 3" ACCORDING TO THE PLAT THEREOF AS RECORDED IN PLAT BOOK 81 AT PAGE 46 OF THE PUBLIC RECORDS OF BROWARD COUNTY, FLORIDA.

PARCEL II:

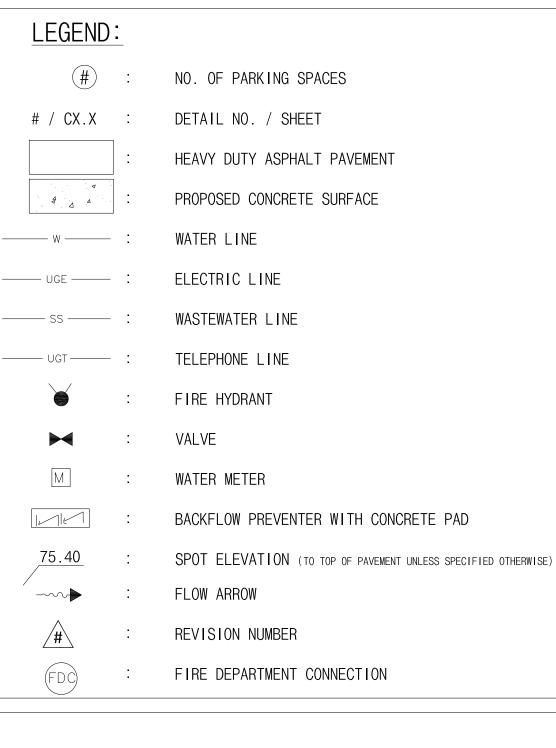
THE NORTH 300 FEET OF TRACT "B" OF "SERINO PARK SECTION 3" ACCORDING TO THE PLAT THEREOF AS RECORDED IN PLAT BOOK 81 AT PAGE 46 OF THE PUBLIC RECORDS OF BROWARD COUNTY, FLORIDA.

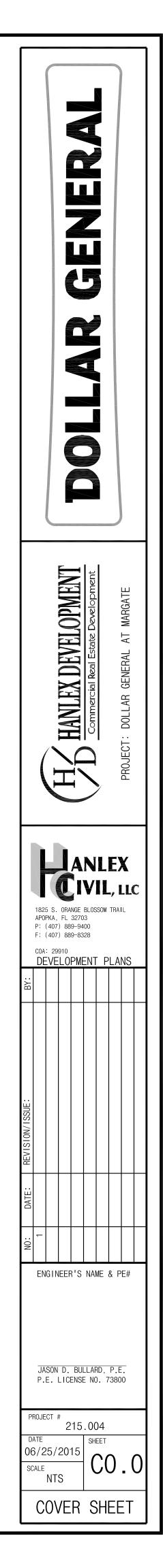
PARCEL III:

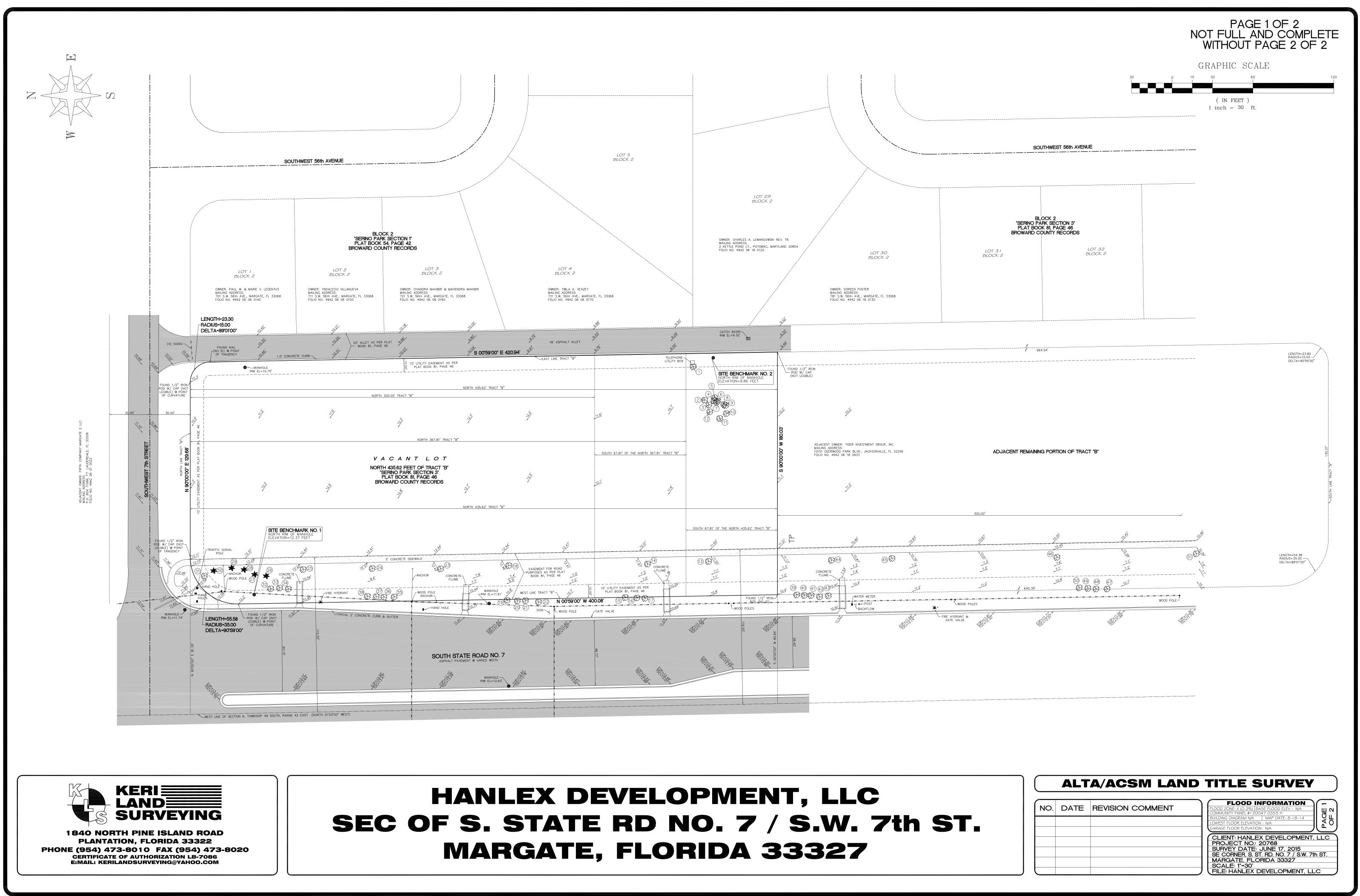
THE SOUTH 67.81 FEET OF THE NORTH 435.62 FEET OF TRACT "B" OF "SERINO PARK SECTION 3" ACCORDING TO THE PLAT THEREOF AS RECORDED IN PLAT BOOK 81 AT PAGE 46 OF THE PUBLIC RECORDS OF BROWARD COUNTY, FLORIDA.

PLAN SHEET INDEX

CO.0	COVER SHEET
	SURVEY
DP1.0	DEVELOPMENT PLAN
CP.1	PHOTOMETRIC PLAN
L1.0	LANDSCAPE PLAN
L2.0	IRRIGATION PLAN
L3.0	TREE DEMOLITION PLAN







LEGAL DESCRIPTION:

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NOTES

1) ELEVATIONS SHOWN HEREON ARE RELATIVE TO THE NORTH AMERICAN VERTICAL DATUM OF 1988. BASED ON BROWARD COUNTY BENCHMARK NO. 2543, ELEVATION = 13.050 FEET.

2) UNDERGROUND IMPROVEMENTS AND/OR UNDERGROUND ENCROACHMENTS NOT LOCATED OR SHOWN. 3) BEARINGS SHOWN HEREON ARE BASED ON THE NORTH

LINE OF TRACT "B" BEING DUE EAST. (AS PER RECORD PLAT) 4) ROOF OVERHANG NOT LOCATED UNLESS OTHERWISE

SHOWN. 5) LANDS SHOWN HEREON CONTAINING 78,105 SQUARE

FEET, (1.7930 ACRES), MORE OR LESS.

6) THIS SURVEY IS FOR BUILDING PERMITTING PURPOSES ONLY.

7) REASONABLE EFFORTS WERE MADE REGARDING THE EXISTENCE AND LOCATION OF UNDERGROUND UTILITIES. THIS FIRM, HOWEVER, DOES NOT ACCEPT RESPONSIBILITY FOR THIS INFORMATION. BEFORE EXCAVATION, CONTACT THE APPROPRIATE UTILITY COMPANIES FOR FIELD VERIFICATION.

8) THIS SURVEY IS CERTIFIED EXCLUSIVELY TO: HANLEX DEVELOPMENT, LLC, A FLORIDA LIMITED LIABILITY COMPANY OLD REPUBLIC NATIONAL TITLE INSURANCE COMPANY

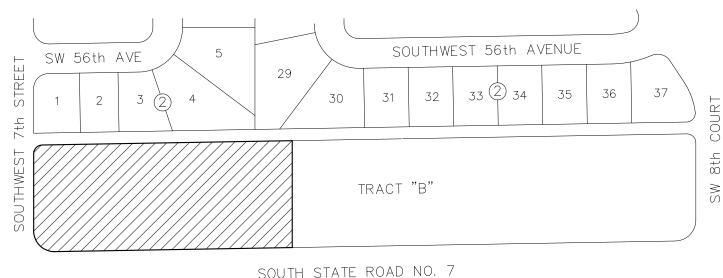
9) THIS SITE LIES IN SECTION 6, TOWNSHIP 49 SOUTH, RANGE 42 EAST, BROWARD COUNTY, FLORIDA. 10) ALL BEARINGS/ANGLES AND DISTANCES SHOWN HEREON ARE BOTH MEASURED ON THE GROUND AND RECORD (AS PER PLAT) UNLESS OTHERWISE SHOWN. 11) — OHW — OHW — DENOTES OVERHEAD WIRES. 12) DENOTES EXISTING GRADE ELEVATION.

THE SURVEY SHOWN HEREON WAS PREPARED WITH THE BENEFIT OF A TITLE COMMITMENT ISSUED BY OLD REPUBLIC NATIONAL TITLE INSURANCE COMPANY (AGENT FILE NO.: XXX-XXX, WITH AN EFFECTIVE DATE OF JUNE XX, 2015. ITEMS NUMBER 1 THROUGH XX OF SCHEDULE B-II WERE REVIEWED AND SHOWN HEREON WHEN APPLICABLE.



1840 NORTH PINE ISLAND ROAD PLANTATION, FLORIDA 33322 PHONE (954) 473-8010 FAX (954) 473-8020 **CERTIFICATE OF AUTHORIZATION LB-7086** E:MAIL: KERILANDSURVEYING@YAHOO.COM

TITLE COMMITMENT NOTES:



LOCATION SKETCH NOT TO SCALE

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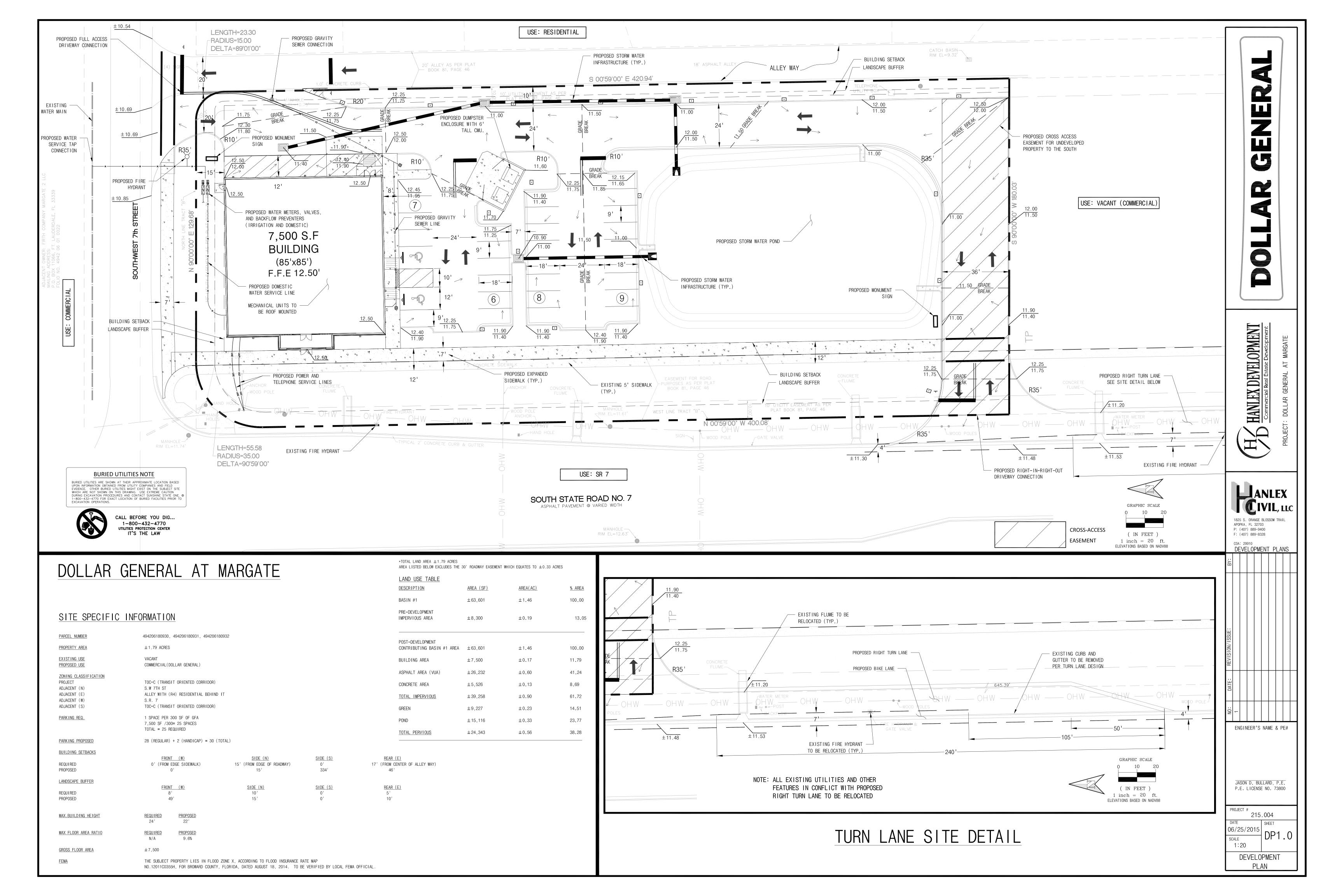
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NO.	SPECIES	DIAMETER
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HANLEX DEVELOPMENT, LLC S. STATE RD NO. 7 / S.W. 7th ST. MARGATE, FLORIDA 33327

NO.	DATE	REVISION COMMENT	ALTA/ACSM LAND TITLE SURVEY
			CERTIFICATION: TO: HANLEX DEVELOPMENT, LLC, A FLORIDA LIMITED LIABILITY COMPANY, OLD REPUBLIC NATIONAL TITLE INSURANCE COMPANY, XXXX THIS IS TO CERTIFY THAT THIS MAP OR PLAT AND THE SURVEY ON WHICH IT IS BASED WERE MADE IN ACCORDANCE WITH THE 2011 MINIMUM STANDARD DETAIL REQUIREMENTS FOR ALTA/ACSM LAND TITLE SURVEYS, JOINTLY ESTABLISHED AND ADOPTED BY ALTA AND NSPS, AND INCLUDES ITEMS 1, 2, 3, 4, 7a, 8, 11a, 13 AND 14 OF TABLE A THEREOF. THE FIELD WORK WAS COMPLETED ON JULY 17, 2015. JAY KERI SIGNATURE DATE PROFESSIONAL SURVEYOR AND MAPPER NO. 5721 STATE OF FLORIDA

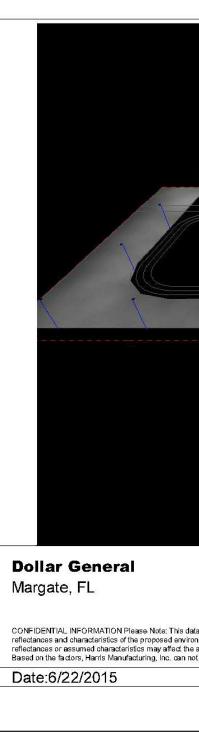
PAGE 2 OF 2 NOT FULL AND COMPLETE WITHOUT PAGE 1 OF 2

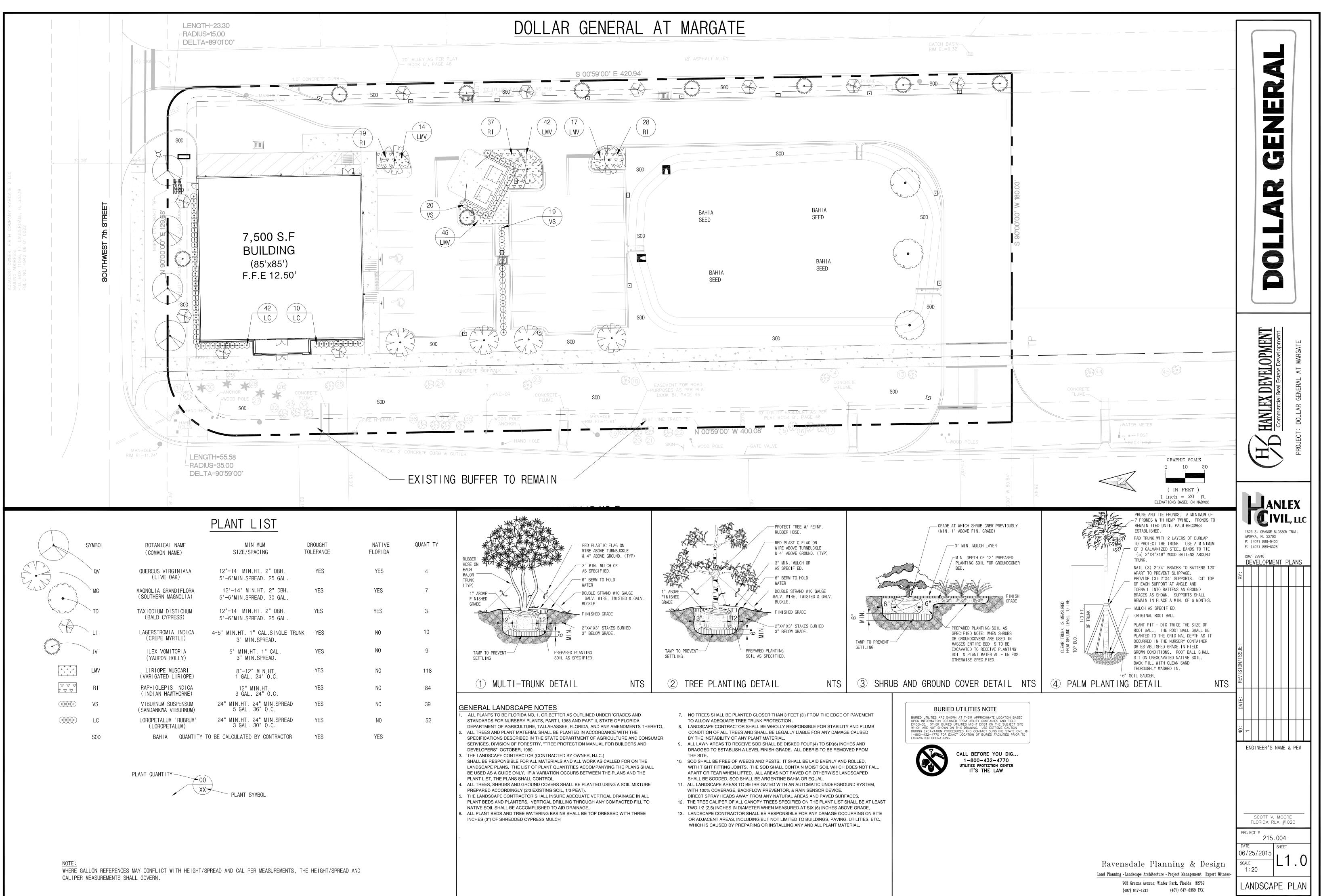
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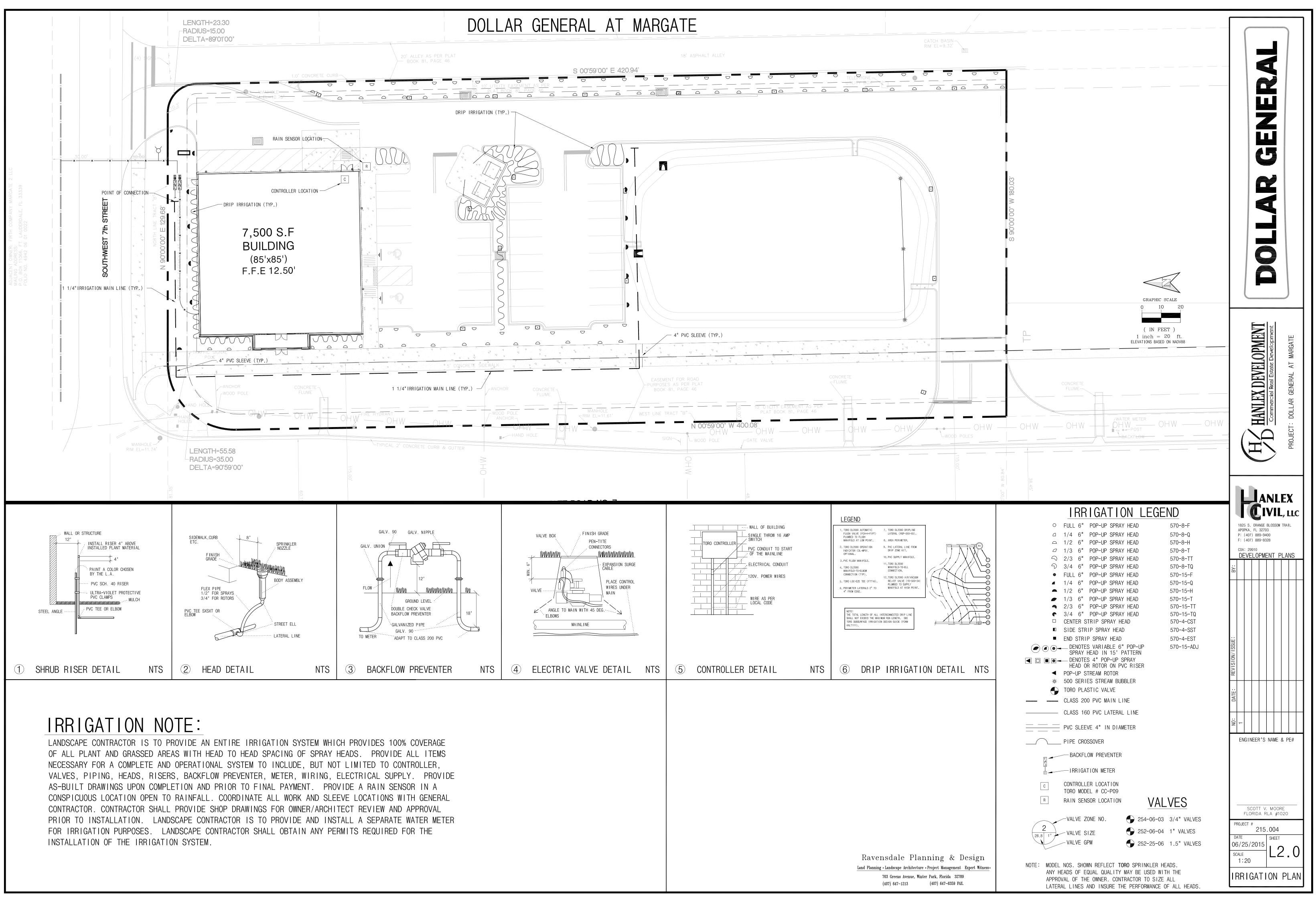
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Symbol Qty Label Description Arrangement Lumens LLF 1 7 MFHID-WPF-100PMH 12in 100W Full Cutoff Wall Pack SINGLE 7600 0.720 1 7 MFHID-PLL-100MH SBL 100W Parking Lot Lighting w. SBL SINGLE 7600 0.720 1 MFHID-PLL-100MH SBL 100W Parking Lot Lighting w. SBL SINGLE 7600 0.720 1 MFHID-PLL-150MH SBL 150W Parking Lot Lighting w. SBL SINGLE 13300 0.720 1 MFHID-PLL-70MH SBL 20 343.379 0 1 MFHID-PLL-70MH SBL 70W Parking Lot Lighting w. SBL SINGLE 5300 0.720 1 MFHID-PLL-70MH SBL 70W Parking Lot Lighting w. SBL SINGLE 0 0 1 MFHID-PLL-70MH SBL 70W Parking Lot Lighting w. SBL SINGLE 5300 0.720 1 MFHID-PLL-70MH SBL 70W Parking Lot Lighting w. SBL SINGLE 5300 0.720 1 MFHID-PLL-70MH SBL 70W Parking Lot Lighting w. SBL SINGLE 5300 0.720 1 MFHID-PLL-70MH SBL 70W Park	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	M. FI FI FI O O O O FI +O +O +O +O +O +O +O +O +O FI FI O		$\begin{array}{cccccccccccccccccccccccccccccccccccc$

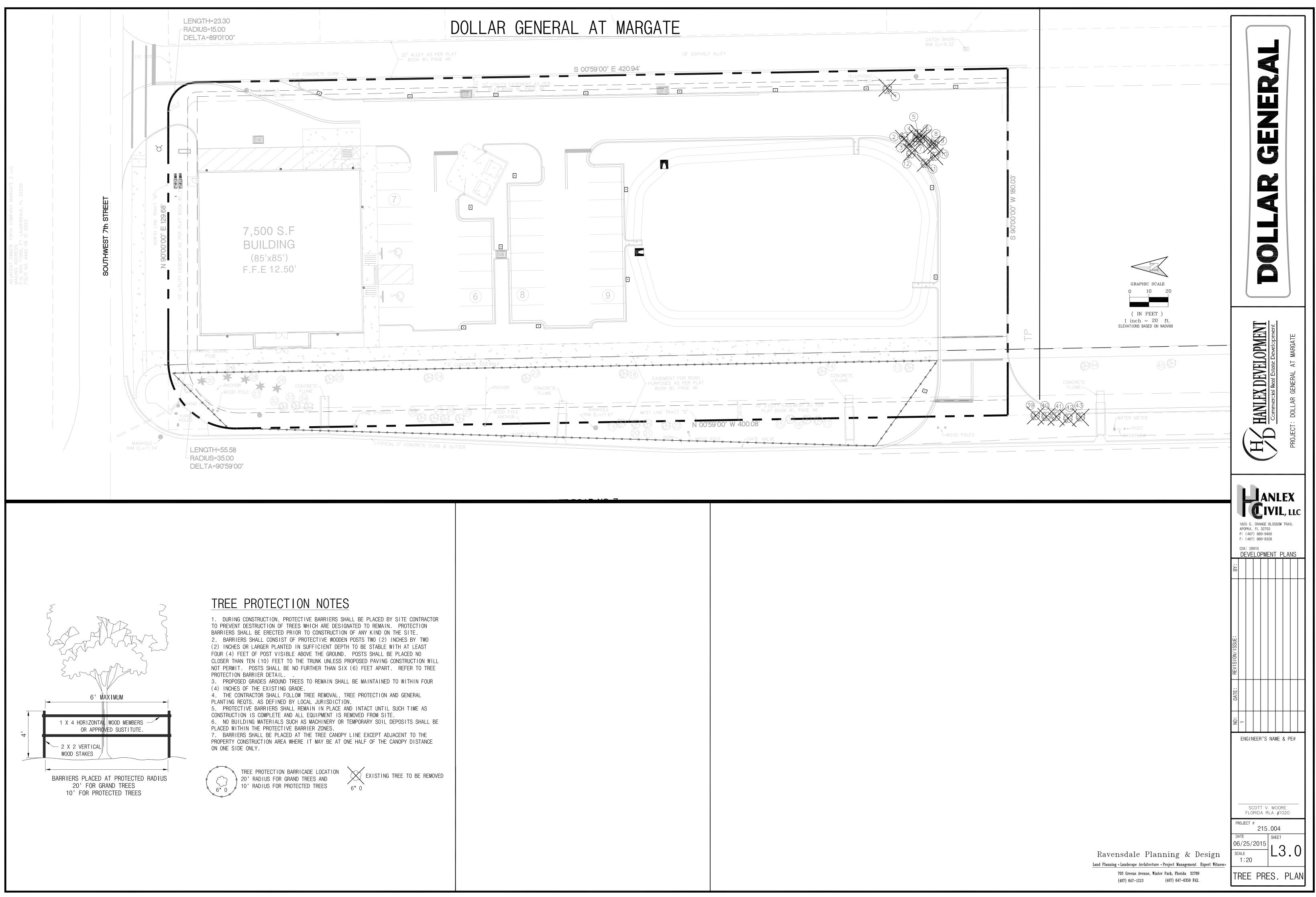
HANLEX DEVELOPMENT EXAMPLEX DEVELOPMENT Commercial Real Estate Development PROJECT: DOLLAR GENERAL AT MARGATE
Image: Construction of the second system APOPKA, FL 32703 P: (407) 889-9400 F: (407) 889-8328 COA: 29910 DEVELOPMENT PLANS Image: Construction of the second system Image: Construction of the second syst

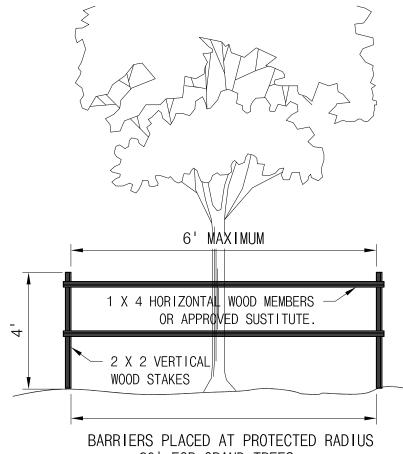


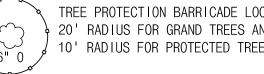


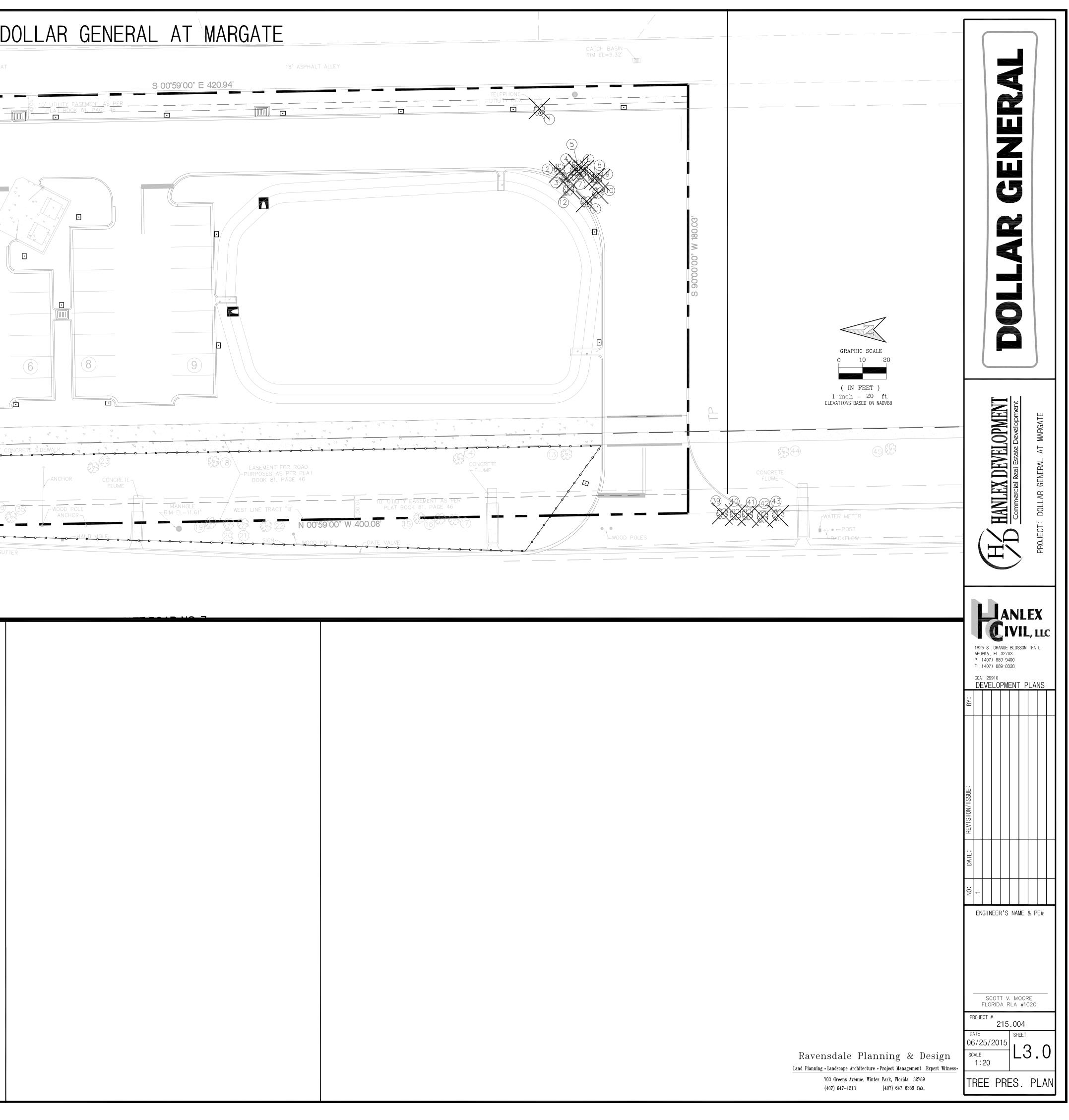
SYMBOL	BOTANICAL NAME (COMMON NAME)	MINIMUM SIZE/SPACING	DROUGHT TOLERANCE	NATIVE FLORIDA	QUANTITY
QV	QUERCUS VIRGINIANA (LIVE OAK)	12'-14' MIN.HT. 2" DBH. 5'-6'MIN.SPREAD. 25 GAL.	YES	YES	4
MG	MAGNOLIA GRANDIFLORA (SOUTHERN MAGNOLIA)	12'-14' MIN.HT. 2" DBH. 5'-6'MIN.SPREAD. 30 GAL.	YES	YES	7
TD	TAXIODIUM DISTICHUM (BALD CYPRESS)	12'-14' MIN.HT. 2" DBH. 5'-6'MIN.SPREAD. 25 GAL.	YES	YES	3
LI	LAGERSTROMIA INDICA (CREPE MYRTLE)	4-5' MIN.HT. 1" CAL.SINGLE TRUNK 3' MIN.SPREAD.	YES	NO	10
IV	ILEX VOMITORIA (YAUPON HOLLY)	5' MIN.HT. 1" CAL. 3' MIN.SPREAD.	YES	NO	9
LMV	LIRIOPE MUSCARI (VARIGATED LIRIOPE)	8"-12" MIN.HT. 1 GAL. 24" O.C.	YES	NO	118
RI	RAPHIOLEPIS INDICA (INDIAN HAWTHORNE)	12" MIN.HT. 3 GAL. 24" O.C.	YES	NO	84
VS	VIBURNUM SUSPENSUM (SANDANKWA VIBURNUM)	24" MIN.HT. 24" MIN.SPREAD 5 GAL. 36" O.C.	YES	NO	39
LC	LOROPETALUM 'RUBRUM' (LOROPETALUM)	24" MIN.HT. 24" MIN.SPREAD 3 GAL. 30" O.C.	YES	NO	52
SOD	BAHIA QUANTITY	TO BE CALCULATED BY CONTRACTOR	YES	YES	

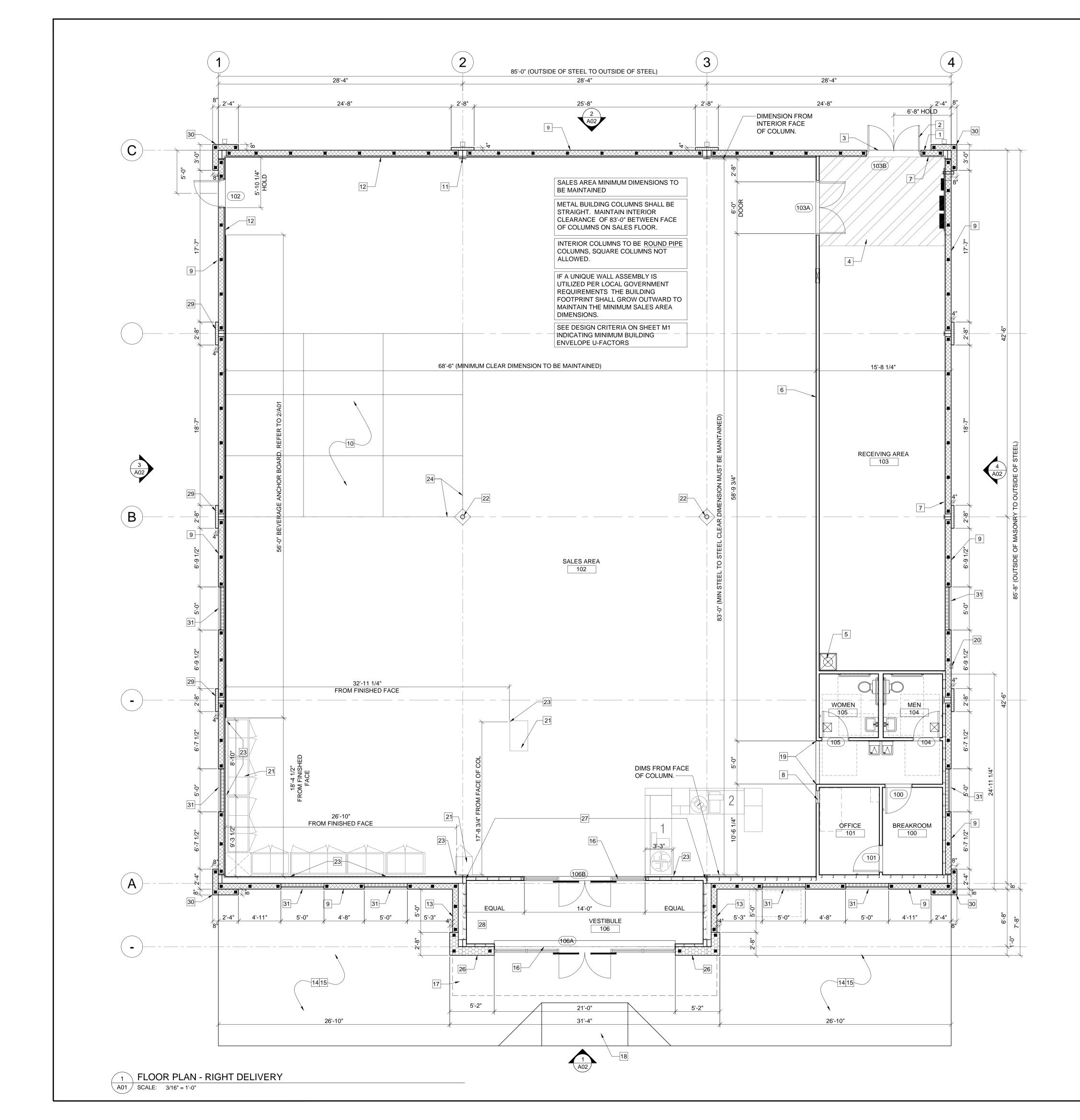


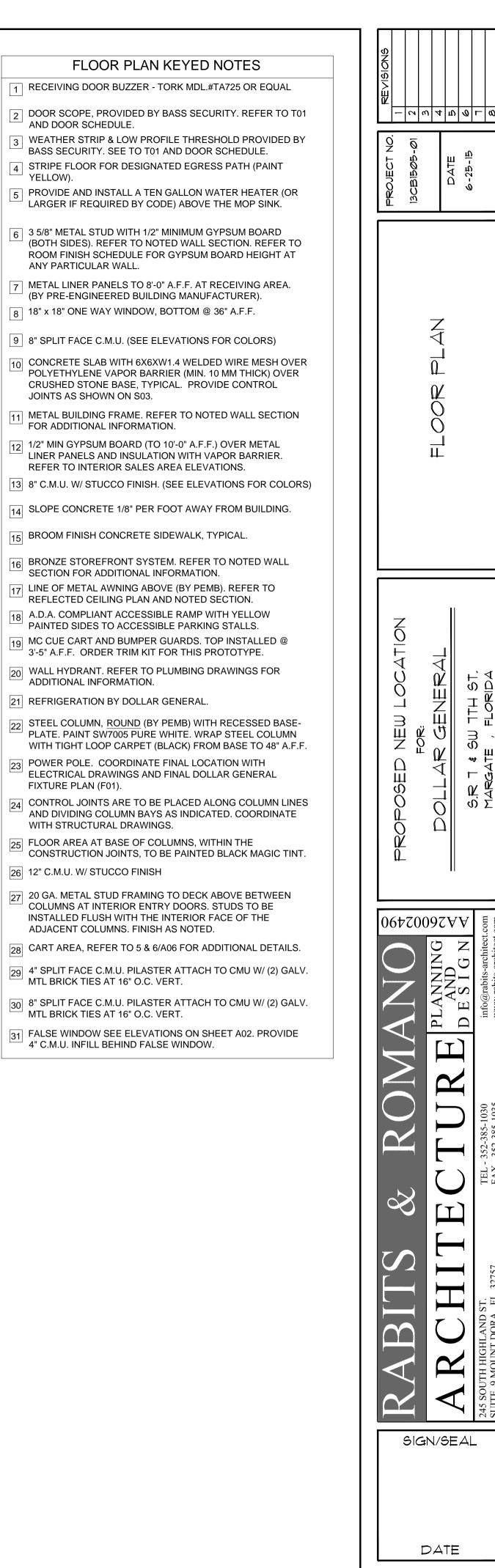










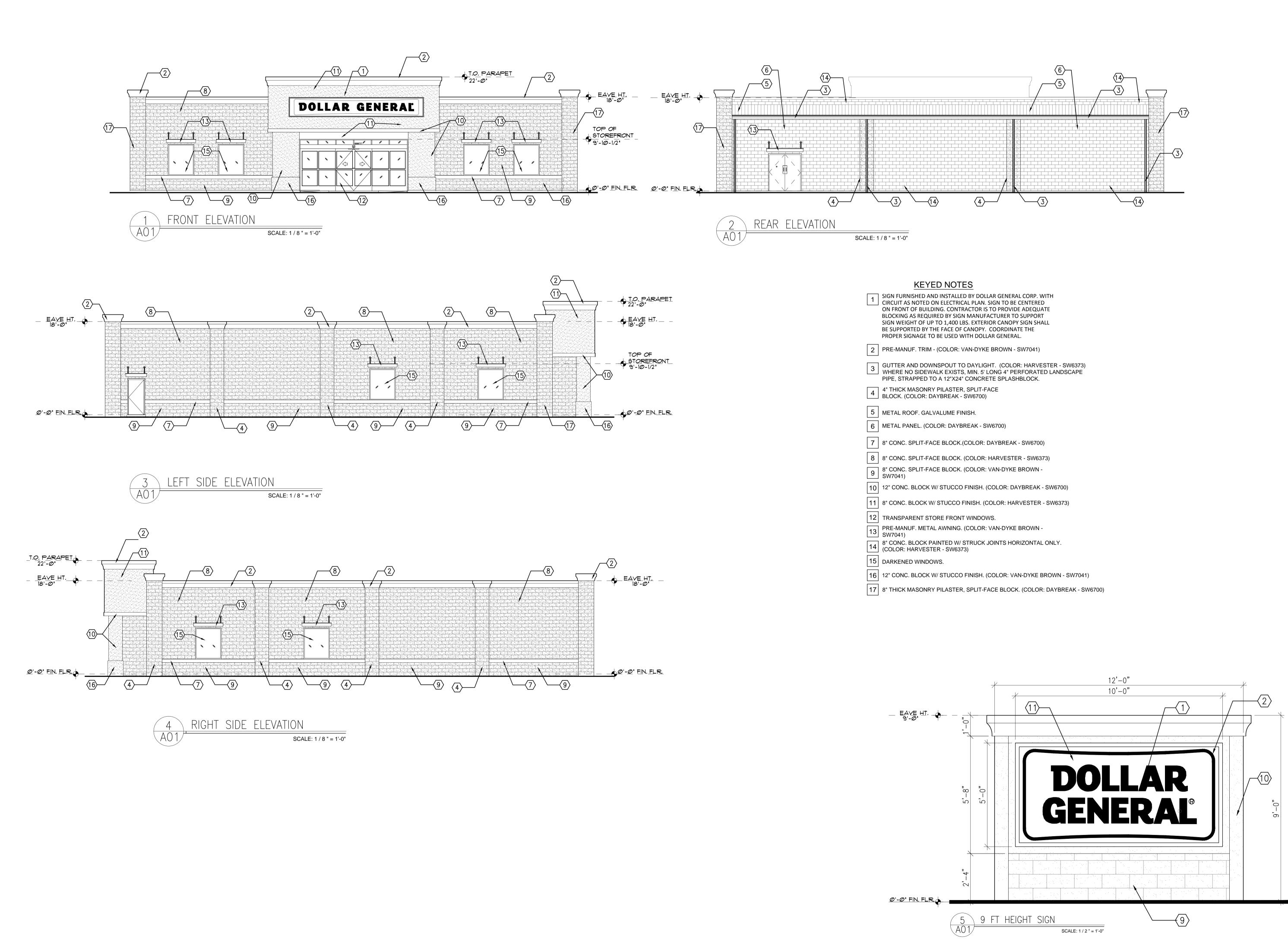


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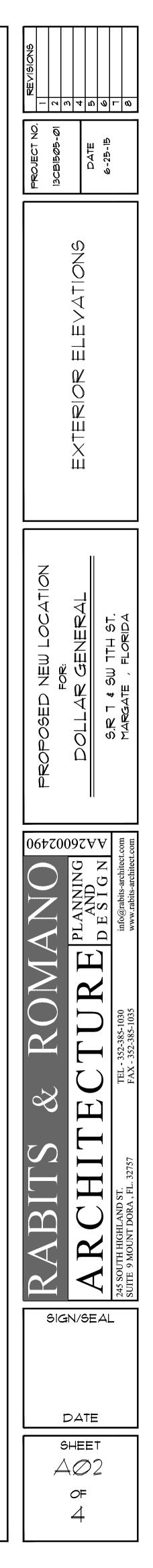
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SOUTH-WEST ELEVATION

NORTH-WEST ELEVATION

REVIEWS REVENCES NG REVISIONS ISCRIBADER NG ISCRIBADEANO DATE 6 6 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
PROPOSED NEW LOCATION FOR: DOLLAR GENERAL S.R. T & SW TTH ST. MARGATE , FLORIDA
Predention ROMMANO 00 Predention ROMANO Reconstruction Predention Reconstruction Reconstruction Roman Dora, FL. 3273 Reconstruction
DATE SHEET AØ3 OF 4





NORTH-EAST ELEVATION

SOUTH-EAST ELEVATION

RENDERING Pare 2-2-19 10 10 10 10 10 10 10 10 10 10 10 10 10	
PROPOSED NEW LOCATION FOR: Took DOLLAR GENERAL SR 1 & SW TTH ST.	MARGATE , FLORIDA
TECTURE	TEL - 352-385-1030 info@rabits-architect.com 32757 FAX - 352-385-1035 www.rabits-architect.com
SIGN/SEAL SHEET A@4 OF 4	245 SOUTH HIGHLAND ST. SUITE 9 MOUNT DORA , FL. 32757

Letter of Authorization

I, the undersigned, owner or legal representative of the owner of the properties located at the Southeast corner of SW 7th St and SR 7 (PID #s 4942-06-18-0930, 4942-06-18-0931, and 4942-06-18-0932); in Margate, FL hereby authorize Jeremy Anderson, Will Anderson, Jason Bullard, Maria Peña, Felix Granados, Sean Sherlin, and Nathan Wolfe of Hanlex Civil, LLC to act as my agent to obtain permits associated with the development of the properties, as required from City of Margate, Broward County, FDOT, SFWMD, FDEP and/or any other jurisdictional entity authorized to issue permits associated with the development of the subject property referenced above.

Any reproduced copy of this signed original shall be deemed to be an original counterpart of this Letter of Authorization.

Tiger Inverment Group. Inc. By: Captlin A Cast U.C.

Signed:

Cynthia A. Cash

Printed Name:

10151 DECRWOOD PARK Blud. Billy 300 Saite 110 JAchsony. Me FL 32256

Address:

1

Sworn to and subscribed before me this 23 nd day of <u>fine</u> <u>Ausanhanan</u> Notary Public, State of Florida My Commission Expires:	_, 2015.
SUSANNA M. DAUGHERTY Commission # FF 032740 Expires July 20, 2017	

ded Thru Troy Fain Insurance 809-365-7019

CERTIFICATE OF CORPORATE RESOLUTION BY CONSENT OF SOLE DIRECTOR

The undersigned, ROBERT HIGEL, being the sole Director of TIGER INVESTMENT GROUP, INC. (hereinafter referred to as "Corporation"), a corporation organized and existing under the laws of the State of Florida, does hereby certify the following:

- 1. The undersigned is the sole Director of the Corporation
- 2. The undersigned has personal knowledge of the matters set forth in this Certificate
- 3. The Corporation is a duly formed and validly existing Corporation in good standing under the laws of the State of Florida
- 4. The Articles and By-Laws of the Corporation are in full force and effect and are unamended as of the date of this Certificate.
- 5. On June 23, 2015, in accordance with the Corporation's By-Laws, the undersigned as sole Director hereby consented to, approved and adopted the following resolutions as if adopted at a meeting of the Board of Directors held for this purpose, which resolutions have not been amended or revoked in any manner whatsoever and are in full force and effect on the date hereof:

WHEREAS, Tiger Investment Group, Inc. (the "Corporation") has entered into an agreement (together with all addenda, amendments, extensions and exhibits thereto, the ""Contract") with Hanlex Development, LLC and/or its assigns (collectively, the "Purchaser") to sell to Purchaser certain real property located in Broward County, Florida and more particularly described in such Contract (the "Property");

WHEREAS, the Corporation wishes to consent to the terms and conditions of said Contract;

THEREFORE RESOLVED, that the Corporation, approves, ratifies and accepts the terms and provisions of that certain Contract with regard to the sale of the Property to Purchaser;

FURTHER RESOLVED, that CYNTHIA A. CASH, as Vice President of the Corporation, is hereby authorized to execute the Contract and to execute any and all documentation required by the Contract and to evidence the consent of the Corporation to the sale and conveyance of the Property to the Purchaser;

FURTHER RESOLVED, that such documents and instruments executed and delivered as aforesaid shall be and constitute the acts and obligations of the Corporation hereby ratifying and confirming the acts of CYNTHIA A. CASH executing and delivering all such documents and instruments, irrespective of whether such acts were performed before or subsequent to the date of adoption hereof, and directing the officers and employees of the Corporation to perform all of the Corporation's obligations and undertakings under each and all such documents and instruments;

FURTHER RESOLVED, that the Corporation is hereby authorized and directed to pay any and all costs and fees associated with the sale of the Property to the extent

expressly required to be paid by the Corporation under the Contract;

FURTHER RESOLVED, that these resolutions shall remain in full force and effect and may be relied upon by the Purchaser and the closing agent until receipt of written notice of any change therein from the Corporation.

6. The following person is duly elected or appointed to the office of the Corporation set forth opposite her name and incumbent in such office as of the date hereof:

<u>NAME</u> Cynthia A. Cash OFFICE Vice President

IN WITNESS WHEREOF, the undersigned has executed this certificate as of the date and year set forth as follows:

Dated: June 23, 2015.

Witnesses:

(sign) (print) (sign ON dua (print)

Robert Hilde Nam Title: Director

STATE OF FLORIDA COUTNY OF DUVAL

The foregoing Resolution was sworn to, subscribed and acknowledged before me this 23 day of June, 2015, by ROBERT HIGEL, as the Director of Tiger Investment Group, Inc., a Florida Corporation and who (X) is personally known to me or (produced ______ as identification.



NOTARY PUBLIC Name:

Commission expires: Serial number: