

1000 Corporate Drive, Ft. Lauderdale, FL 33334
Tel: 954-202-7000
Fax: 954-202-7070

Calculated By: KNN
Checked By: ROT

PROPOSED DRAINAGE CALCULATIONS

Design Criteria:

Basin Control Elevation: 8.00 NAVD

Proposed Acreages

Lake Areas (A_L):	0 sf	or	0.000 ac
Roof Areas (A_R):	5,673 sf	or	0.130 ac
Paved Areas (A_P):	29,018 sf	or	0.666 ac
Green Areas (A_G):	11,650 sf	or	0.267 ac
Total (A_T):	46,341 sf	or	1.064 ac

Compute Required Water Quality Volume:

- 1) Provide at least 1 inch over the developed project:

$$\begin{aligned}
 V_{PRE} &= 1 \text{ inch} \times A_T \times 1 \text{ ft} / 12 \text{ inches} \\
 &= 1 \times 1.064 / 12 \\
 &= 0.09 \text{ ac-ft or } 1.06 \text{ ac-in}
 \end{aligned}$$

- 2) Provide 2.5" over % impervious area:

- a) Site Area for water quality pervious/impervious calculation:

$$\begin{aligned}
 A_S &= A_T - (A_L + A_R) \\
 &= 1.064 - .130 \\
 &= 0.93 \text{ ac of site area for water quality pervious/impervious}
 \end{aligned}$$

- b) Impervious area for water quality pervious/impervious calculation:

$$\begin{aligned}
 A_{IMP} &= A_S - A_G \\
 &= .94 - .267 \\
 &= 0.67 \text{ ac of impervious area for water quality pervious/impervious}
 \end{aligned}$$

- c) Percent of impervious for water quality calculation:

$$\begin{aligned}
 &= A_{IMP} / A_S \times 100\% \\
 &= 0.666 / 0.93 \\
 &= 71.3\% \text{ impervious}
 \end{aligned}$$

- d) For 2.5" times the percent impervious:

$$\begin{aligned}
 &= 2.5" \times \% \text{ impervious area} \\
 &= 2.5 \times 0.713 \\
 &= 1.78 \text{ inches to be treated}
 \end{aligned}$$

- e) Compute volume required volume for quality detention

$$\begin{aligned}
 V_{PRE} &= \text{inches to be treated} \times (A_T - A_L) \\
 &= 1.78 \times (1.064 - 0) \times 1 \text{ foot} / 12 \text{ inches} \\
 &= 0.16 \text{ ac-ft or } 1.89 \text{ ac-in}
 \end{aligned}$$

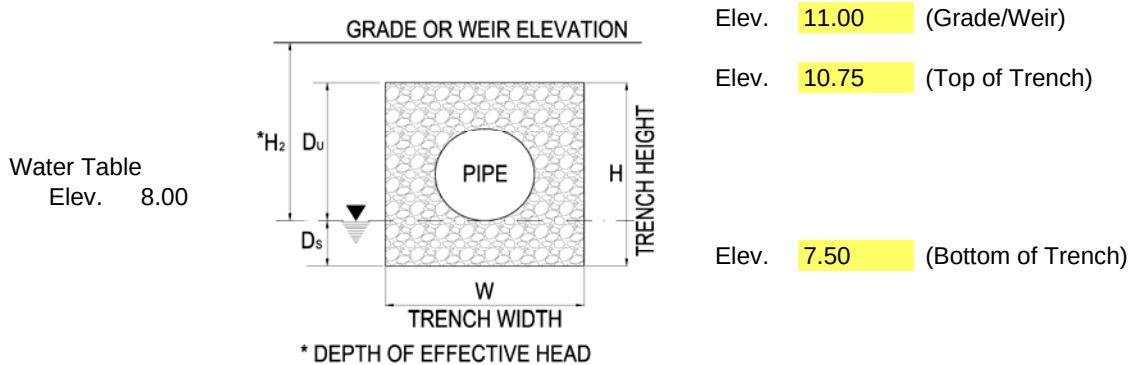
- 3) Since the 1.89 ac-in is greater than the 1.064 ac-in computed for the first inch of runoff the volume of 1.89 ac-in controls.

Exfiltration Trench Calculations

K-Value:

Test Hole #	(cfs/ft ² /ft hd)	
P-1*	3.48E-04	* Walgreens SWM2009-059-0
P-2	3.07E-04	
K _{AVG}	3.28E-04	

Trench:



K =	3.28E-04	cfs/ft ² - ft head
H ₂ =	3.25	ft
W =	6.00	ft
D _u =	2.75	ft
D _s =	0.50	ft
H = Du + Ds =	3.25	ft

1) Trench Length for Water Quality Requirements:

$$V = 1.89 \text{ ac-in or } 0.16 \text{ ac-ft}$$

$$L = \frac{V}{K(H_2W + 2H_2D_u - D_u^2 + 2H_2D_s) + (1.39 \times 10^{-4})WD_u}$$

$$L = 144.0 \text{ feet}$$

2) Compute Maximum Length Trench:

$$V = 3.28 \text{ inches} \times 1.064 \text{ acres} = 3.49 \text{ ac-in or } 0.29 \text{ ac-ft}$$

$$L = 266.0 \text{ feet}$$

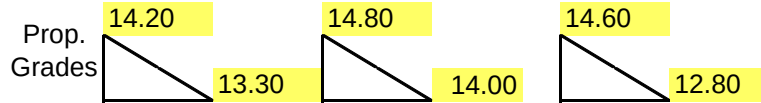
3) Compute Provided Trench Volume:

$$L = 144 \text{ feet}$$

$$V = L \times (K(H_2W + 2H_2D_u - D_u^2 + 2H_2D_s) + (1.39 \times 10^{-4})WD_u)$$

$$V = 1.89 \text{ ac-in or } 0.16 \text{ ac-ft}$$

STAGE STORAGE AREA CALCULATION - PROPOSED



Stage	Pavement Area Area 0.585 0.000 (ac.-ft.)	Sidewalk Area Area 0.084 0.000 (ac.-ft.)	Landscape Area Area 0.267 0.000 (ac.-ft.)	Total Affected Site 0.936
11.00	0.00	0.00	0.00	0.00
11.20	0.00	0.00	0.00	0.00
11.40	0.00	0.00	0.00	0.00
11.60	0.00	0.00	0.00	0.00
11.80	0.00	0.00	0.00	0.00
12.00	0.00	0.00	0.00	0.00
12.20	0.00	0.00	0.00	0.00
12.40	0.00	0.00	0.00	0.00
12.60	0.00	0.00	0.00	0.00
12.80	0.00	0.00	0.00	0.00
13.00	0.00	0.00	0.00	0.00
13.20	0.00	0.000	0.012	0.01
13.40	0.00	0.000	0.027	0.03
13.60	0.03	0.000	0.048	0.08
13.80	0.08	0.000	0.074	0.16
14.00	0.16	0.000	0.107	0.27
14.20	0.26	0.002	0.146	0.41
14.40	0.38	0.008	0.190	0.58
14.60	0.50	0.019	0.241	0.76
14.80	0.61	0.034	0.294	0.94
15.00	0.73	0.050	0.348	1.13
15.20	0.85	0.067	0.401	1.32
15.40	0.96	0.084	0.455	1.50
15.60	1.08	0.101	0.508	1.69

DESIGN CRITERIA

PROPOSED LAND USE SUMMARY

Areas:	Square Ft.	Acres	Percent
Lake	0	0.00	0.0%
Building	5,673	0.130	12.2%
Paved and Sidewalk	29,018	0.666	62.6%
Pervious	11,650	0.267	25.1%
Total Area:	46,341	1.064	100.0%

STAGE STORAGE AREA CALCULATION

Stage	Site Stage-Storage (previous page)	Exfiltration Trench Storage (ac.-ft.)	Total Storage Area (ac.-ft.)
11.00	0.00	0.16	0.16
11.20	0.00	0.16	0.16
11.40	0.00	0.16	0.16
11.60	0.00	0.16	0.16
11.80	0.00	0.16	0.16
12.00	0.00	0.16	0.16
12.20	0.00	0.16	0.16
12.40	0.00	0.16	0.16
12.60	0.00	0.16	0.16
12.80	0.00	0.16	0.16
13.00	0.00	0.16	0.16
13.20	0.01	0.16	0.17
13.40	0.03	0.16	0.19
13.60	0.08	0.16	0.23
13.80	0.16	0.16	0.31
14.00	0.27	0.16	0.42
14.20	0.41	0.16	0.57
14.40	0.58	0.16	0.74
14.60	0.76	0.16	0.91
14.80	0.94	0.16	1.10
15.00	1.13	0.16	1.29
15.20	1.32	0.16	1.47
15.40	1.50	0.16	1.66
15.60	1.69	0.16	1.85

Soil Storage

Land Use Summary:

	Acres	Percent
Lake Areas (A_L):	0.000	0.0%
Roof Areas (A_R):	0.130	12.2%
Paved Areas (A_P):	0.666	62.6%
Green Areas (A_G):	0.267	25.1%
Total (A_T):	1.064	100.0%

Compacted Soil Storage per
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Depth to Water Table (feet)	Water Storage (inches)
1	0.45
2	1.88
3	4.05
4	6.75

Average Pervious Grade (Elev.): 13.70
Depth to Water Table: 5.70 ft
Soil Storage at Average Depth (S_S): 6.75 inches

Weighted S value:

$$= S_S \times \% \text{ Pervious}$$

$$= 6.75 \times 0.251$$

$$= \boxed{1.69 \text{ inches}}$$

Rainfalls

10-Year, 1 day Storm =	$\boxed{6.20 \text{ inches}}$
25-Year, 3 day Storm =	$\boxed{14.50 \text{ inches}}$
100-Year, 3 day Storm =	$\boxed{18.50 \text{ inches}}$

Results from Stage Storage Elevations

$$\begin{aligned} \text{Runoff } (Q) &= (P - 0.2S)^2 / (P + 0.8S) \\ &= (6.20 - 0.2(1.69))^2 / (6.20 + 0.8(1.69)) \\ &= 4.55 \text{ inches of total runoff} \end{aligned}$$

$$\begin{aligned} \text{Runoff Volume} &= Q * \text{Project Area} \\ &= 1.064 \text{ ac} * 4.55" = 4.84 \text{ acre-inches} = \mathbf{0.40} \text{ acre-ft.} \end{aligned}$$

Maximum Stage for 10-Year 1-day Storm (zero discharge)	13.96
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$$\begin{aligned} \text{Runoff } (Q) &= (P - 0.2S)^2 / (P + 0.8S) \\ &= (14.5 - 0.2(1.69))^2 / (14.5 + 0.8(1.69)) \\ &= 12.65 \text{ inches of total runoff} \end{aligned}$$

$$\begin{aligned} \text{Runoff Volume} &= Q * \text{Project Area} \\ &= 1.064 \text{ ac} * 12.30" = 13.46 \text{ acre-inches} = \mathbf{1.12} \text{ acre-ft.} \end{aligned}$$

Maximum Stage for 25-Year 3-Day Storm (zero discharge)	14.82
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$$\begin{aligned} \text{Runoff } (Q) &= (P - 0.2S)^2 / (P + 0.8S) \\ &= (18.5 - 0.2(1.69))^2 / (18.5 + 0.8(1.69)) \\ &= 16.62 \text{ inches of total runoff} \end{aligned}$$

$$\begin{aligned} \text{Runoff Volume} &= Q * \text{Project Area} \\ &= 1.064 \text{ ac} * 16.25" = 17.68 \text{ acre-inches} = \mathbf{1.47} \text{ acre-ft.} \end{aligned}$$

Maximum Stage for 100-Year 3-Day Storm (zero discharge)	15.20
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