

# **SURFACE WATER MANAGEMENT CALCULATIONS**

## **MARQUESA**

**CITY OF MARGATE, BROWARD COUNTY, FLORIDA**

**HSQ PROJECT No.: 1703-18**

*Prepared For:*

**TC MC MARGATE APARTMENTS, LLC.**

7480 Southwest 40th Street, Suite #700

Miami, Florida 33155

*Prepared By:*



**HSQ GROUP, INC.**

Engineers • Planners • Surveyors

5951 Northwest 173rd Drive, Suite 4

Miami, Florida 33015

(786) 534-3621 Phone

**DATE: AUGUST 25, 2018**

# PRE-DEVELOPMENT CALCULATIONS

*Prepared By:*



**HSQ GROUP, INC.**

Engineers • Planners • Surveyors

5951 Northwest 173rd Drive, Suite 4

Miami, Florida 33015

(786) 534-3621 Phone

**GIVEN:**

**A. LAND USE SUMMARY:**

- |    |                     |                     |
|----|---------------------|---------------------|
| 1. | Lake Area =         | 0.000 ac.           |
| 2. | Buildings =         | 1.538 ac.           |
| 3. | Pavement & Others = | 5.623 ac.           |
| 4. | Green Areas =       | 0.952 ac.           |
| 5. | <b>Total =</b>      | <b><u>8.113</u></b> |

**DESIGN CRITERIA:**

**A. WATER QUALITY CRITERIA:**

1. If a wet detention system, then whichever is the greater of the following:
  - a. The first inch of runoff from the entire project site.
  - b. The amount of 2.5 inches times the percent impervious for the project site.
2. If a dry detention system, then 75% of the volume required for the wet detention system.
3. If a retention system, then 50% of the volume required.
4. If the property is zoned "Commercial", at least 0.5 inches of retention or dry detention pre-treatment will be required.
5. Any detention system shall be designed to discharge no more than 0.5 inches of the detained volume per day.

**B. WATER QUANTITY CRITERIA:**

**1. DESIGN EVENTS AND RAINFALL AMOUNTS:**

- a. Design Event for Minimum Road Elevation:
 

Frequency:	10 year
Duration:	1 day
Amount:	9.00 inches
- b. Design Event for Minimum Discharge Elevation:
 

Frequency:	25 year
Duration:	3 day
Amount:	15.00 inches
- c. Design Event for Minimum Finish Floor Elevation:
 

Frequency:	100 year
Duration:	3 day
Amount:	20.00 inches

**2. ADDITIONAL DESIGN INFORMATION:**

- a. Design Water / Control Elevation: 6.50 NAVD.
- b. Drainage Basin / Canal Number: S.F.W.M.D. C-14 BASIN

## **COMPUTATIONS:**

### **A. WATER QUALITY COMPUTATIONS:**

1. Compute the first inch of runoff from the entire developed project site:
 
$$= 1.00 \text{ inch} \times 8.113 \text{ acres} \times (1 \text{ foot} / 12 \text{ inches})$$

$$= \underline{\underline{0.676 \text{ ac-ft for the first inch of runoff}}}$$
2. Compute 2.5 inches times the percent impervious for the developed project site:
  - a. Site area for water quality pervious / impervious calculations only:
 
$$= \text{Total Project} - (\text{Lake Area} + \text{Buildings})$$

$$= 8.113 \text{ acres} - (0.000 \text{ acres} + 1.538 \text{ acres})$$

$$= \underline{\underline{6.575 \text{ acres of site area for water quality calculations}}}$$
  - b. Impervious area for water quality pervious / impervious calculations only:
 
$$= \text{Site area for water quality} - \text{Pervious area}$$

$$= 6.575 \text{ acres} - 0.952 \text{ acres}$$

$$= \underline{\underline{5.623 \text{ acres of impervious area for water quality calculations}}}$$
  - c. Percentage of impervious area for water quality:
 
$$= \text{Impervious area for water quality} / \text{Site area for water quality} \times 100\%$$

$$= 5.623 \text{ acres} / 6.575 \text{ acres} \times 100\%$$

$$= \underline{\underline{85.52 \% \text{ Impervious}}}$$
  - d. For 2.5 inches times the percentage of impervious area:
 
$$= 2.5 \text{ inches} \times 85.52 \%$$

$$= \underline{\underline{2.138 \text{ inches to be treated}}}$$
  - e. Compute volume required for quality detention:
 
$$= \text{Inches to be treated} \times (\text{Total Site Area} - \text{Lake Area})$$

$$= 2.138 \text{ inches} \times (8.113 \text{ acres} - 0.000 \text{ acres}) \times (1 \text{ foot} / 12 \text{ inches})$$

$$= \underline{\underline{1.445 \text{ ac-ft required for detention storage}}}$$
3. The first inch of runoff from the entire developed site = 0.676 ac-ft  
 2.5 inches times the percentage of impervious area = 1.445 ac-ft

**The volume of 1.445 ac-ft controls**

4. If the project is zoned "Commercial" or if the project were discharging directly to a sensitive receiving body and is more than 40% impervious, 0.5 inches of dry detention pre-treatment must be provided:
 
$$= 0.5 \text{ inches} \times (\text{Total Site Area} - \text{Lake Area})$$

$$= 0.5 \text{ inches} \times (8.113 \text{ acres} - 0.000 \text{ acres}) \times (1 \text{ foot} / 12 \text{ inches})$$

$$= \underline{\underline{0.338 \text{ ac-ft required for pre-treatment}}}$$
5. Compute credit for using one of the following systems:
  - a. Wet detention volume to be provided:
 
$$= \text{Total required detention} - \text{Pre-treatment}$$

$$= 1.445 \text{ ac-ft} - 0.338 \text{ ac-ft}$$

$$= \underline{\underline{1.107 \text{ ac-ft of volume required for wet detention}}}$$
  - b. Dry detention volume to be provided ( 75% of the total required detention volume ):
 
$$= \text{Total required detention volume} \times 75\%$$

$$= 1.445 \text{ ac-ft} \times 75\%$$

$$= \underline{\underline{1.084 \text{ ac-ft of volume required for dry detention}}}$$
  - c. Dry retention volume to be provided ( 50% of the total required detention volume ):
 
$$= \text{Total required detention volume} \times 50\%$$

$$= 1.445 \text{ ac-ft} \times 50\%$$

$$= \underline{\underline{0.723 \text{ ac-ft of volume required for dry retention}}}$$

**B. SUMMARY OF WATER QUALITY COMPUTATIONS:**

Item:	Description:	Quantity:
A.1	First inch of runoff from entire project site =	0.676 ac-ft
A.2	2.5 inches times percent impervious =	1.445 ac-ft
A.3	Volume to be treated =	1.445 ac-ft
A.4	Pre-treatment required for commercial site =	0.338 ac-ft
A.5.a	Wet detention volume required =	1.107 ac-ft
A.5.b	Dry detention volume required =	1.084 ac-ft
A.5.c	Dry retention volume required =	0.723 ac-ft
A.5.d	Exfiltration trench volume required =	1.445 ac-ft

**C. STAGE ELEVATION INFORMATION:**

Item:	Description:	S type	Area ac.	Low ft.	High ft.	I %	C %	Total Area %
1	n / a	V						0.00
2	n / a	L						0.00
3	n / a	V						0.00
4	n / a	L						0.00
5	n / a	V						0.00
6	n / a	L						0.00
7	n / a	L						0.00
8	Green Areas	L	0.952	10.40	12.80	0	50	11.73
9	Pavement Areas	L	5.332	10.55	12.80	100	100	65.72
10	Sidewalk Areas	L	0.291	12.00	12.50	100	100	3.59
11	Building Coverage Area	V	1.538	12.19	12.64	100	100	18.96
<b>Total:</b>			<b>8.113</b>	<b>10.40</b>	<b>12.80</b>	<b>88.27</b>	<b>94.13</b>	<b>100.0</b>

\* Abbreviations: S = Storage; ( V = Vertical Storage & L = Linear Storage )  
I = Impervious  
C = Compaction; ( Use the following compaction factors: 0%, 50%, 100% )

**D. SCS CURVE NUMBER AND SOIL STORAGE CALCULATIONS:**

1. Soil Moisture Storage Table:  
Existing Soil Type: **3** DEPRESSIONAL

Depth to Water Table ft.	Cumulative Water Storage ( Pre.-Dev. ) in.	Compacted Water Storage ( Post 50% ) in.	Compacted Water Storage ( Post 100% ) in.
1	0.60	0.53	0.45
2	2.10	1.84	1.58
3	4.40	3.85	3.30
4	6.80	5.95	5.10

2. Available Soil Storage Calculation:

Item:	Description:	Elev. ft.	S in.	Area acres	Stored ac-in
1	n / a	0.00	0.00	0.000	0.00
2	n / a	0.00	0.00	0.000	0.00
3	n / a	0.00	0.00	0.000	0.00
4	n / a	0.00	0.00	0.000	0.00
5	n / a	0.00	0.00	0.000	0.00
6	n / a	0.00	0.00	0.000	0.00
7	n / a	0.00	0.00	0.000	0.00
8	Green Areas	11.60	5.95	0.952	5.66
9	Pavement Areas	11.68	5.10	0.000	0.00
10	Sidewalk Areas	12.25	5.10	0.000	0.00
11	Building Coverage Area	12.42	5.10	0.000	0.00
<b>Total:</b>		<b>11.60</b>	<b>21.25</b>	<b>0.952</b>	<b>5.66</b>

\* Abbreviations: S = Soil Storage  
P = Pervious

3. Moisture Storage Calculation ( S ):  
= Available soil storage / Total Site Area  
= 5.66 ac-in / 8.113 acres  
= **0.70 inches**
4. SCS Curve Number Calculation ( CN ):  
= 1000 / ( S + 10 )  
= 1000 / ( 0.698 + 10 )  
= **93**

#### E. SURFACE STORAGE CALCULATIONS:

1. Stage vs. Storage Calculations:

Stage ft.	Item:	STORAGE ( ac-ft )											Total ac-ft
		1 ac-ft	2 ac-ft	3 ac-ft	4 ac-ft	5 ac-ft	6 ac-ft	7 ac-ft	8 ac-ft	9 ac-ft	10 ac-ft	T ac-ft	
6.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
7.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.09	0.09
7.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.19	0.19
8.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.28	0.28
8.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.37	0.37
9.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.47	0.47
9.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.56	0.56
10.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.56	0.56
10.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.56	0.56
11.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.07	0.24	0.00	0.56	0.87
11.25		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.14	0.58	0.00	0.56	1.28
11.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.24	1.07	0.00	0.56	1.87
11.75		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.36	1.71	0.00	0.56	2.63
12.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.51	2.49	0.00	0.56	3.56
12.25		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.68	3.42	0.02	0.56	4.68
12.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.87	4.51	0.07	0.56	6.01
12.75		0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.10	5.73	0.15	0.56	7.54
13.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.33	7.06	0.22	0.56	9.18
13.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.81	9.73	0.36	0.56	12.46
14.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.28	12.40	0.51	0.56	15.75

\* Abbreviations: T = Exfiltration Trench

#### E. MINIMUM BUILDING FINISH FLOOR ELEVATION CALCULATIONS (ZERO DISCHARGE):

1. The rainfall amount for the 100-Year, 3-Day storm event:  
= **20.00 in.**
2. Compute inches of runoff, Q:  
=  $(P - (0.2 S))^2 / (P + (0.8 X S))$   
=  $(20.00 \text{ in.} - (0.2 \times 0.70 \text{ in.}))^2 / (20.00 \text{ in.} + (0.8 \times 0.70 \text{ in.}))$   
= **19.19 inches of runoff**
3. Compute volume of runoff:  
= (Inches of Runoff) X (Project Area)  
= 19.19 inches X 8.113 acres X (1 foot / 12 inches)  
= **12.97 ac-ft of storage required (zero discharge)**
4. From the stage vs storage curve, **12.97** ac-ft corresponds to elevation **13.58 NAVD.**

**G. MINIMUM DISCHARGE ELEVATION CALCULATIONS (ZERO DISCHARGE):**

1. The rainfall amount for the 25-Year, 3-Day storm event:  
= **15.00 in.**
2. Compute inches of runoff, Q:  
=  $(P - (0.2 S))^2 / (P + (0.8 X S))$   
=  $(15.00 \text{ in.} - (0.2 \times 0.70 \text{ in.}))^2 / 15.00 \text{ in.} + (0.8 \times 0.70 \text{ in.})$   
= **14.19 inches of runoff**
3. Compute volume of runoff:  
= (Inches of Runoff) X (Project Area)  
= 14.19 inches X 8.113 acres X (1 foot / 12 inches)  
= **9.60 ac-ft of storage required (zero discharge)**
4. From the stage vs storage curve, **9.60** ac-ft corresponds to elevation **13.06 NAVD.**

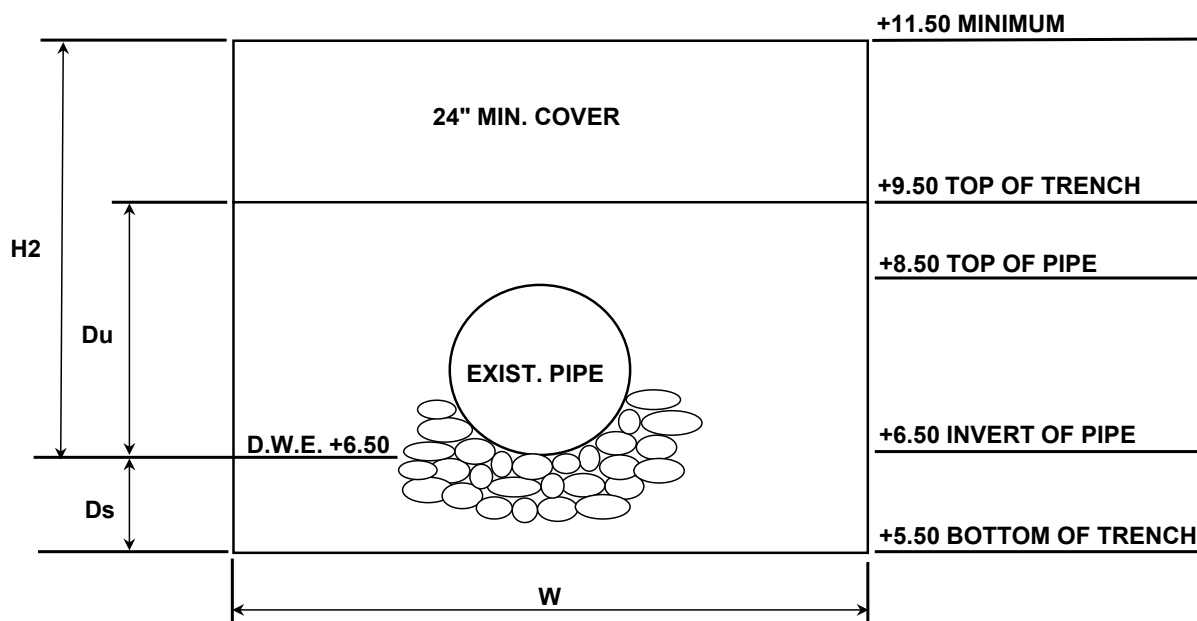
**H. MINIMUM ROAD CROWN ELEVATION CALCULATIONS (ZERO DISCHARGE):**

1. The rainfall amount for the 10-Year, 1-Day storm event:  
= **9.00 in.**
2. Compute inches of runoff, Q:  
=  $(P - (0.2 S))^2 / (P + (0.8 X S))$   
=  $(9.00 \text{ in.} - (0.2 \times 0.70 \text{ in.}))^2 / 9.00 \text{ in.} + (0.8 \times 0.70 \text{ in.})$   
= **8.21 inches of runoff**
3. Compute volume of runoff:  
= (Inches of Runoff) X (Project Area)  
= 8.21 inches X 8.113 acres X (1 foot / 12 inches)  
= **5.55 ac-ft of storage required (zero discharge)**
4. From the stage vs storage curve, **5.55** ac-ft corresponds to elevation **12.41 NAVD.**



# **I. EXFILTRATION TRENCH CALCULATIONS:**

1. Design Formula:  $L = (FS ((\%WQ \cdot V1) + V2)) / (K((2 \cdot H2 \cdot Du) - (Du^2) + (2 \cdot H2 \cdot Ds)) + (1.39 \times 10^{-4} \cdot W \cdot Du))$
2. Design Information:
  - V1 = Exfiltrated Volume (Pre-treatment): 0.00 ac-in
  - V2 = Exfiltrated Volume (Additional Storage): 0.00 ac-in
  - W = Trench Width: 10.00 ft.
  - K = Hydraulic Conductivity: 1.00E-04 cfs/sq-ft per ft head
  - H2 = Depth of Water Table: 5.00 ft.
  - Du = Non-Saturated Trench Depth: 3.00 ft.
  - Ds = Saturated Trench Depth: 1.00 ft.
3. Existing Exfiltration Trench: **1.850 ft.**
4. Exfiltration Trench Storage Provided: **6.72 ac-in** or **0.56 ac-ft**



# POST-DEVELOPMENT CALCULATIONS

*Prepared By:*



**HSQ GROUP, INC.**

Engineers • Planners • Surveyors

5951 Northwest 173rd Drive, Suite 4

Miami, Florida 33015

(786) 534-3621 Phone

**GIVEN:**

**A. LAND USE SUMMARY:**

- |    |                     |                     |
|----|---------------------|---------------------|
| 1. | Lake Area =         | 0.000 ac.           |
| 2. | Buildings =         | 1.706 ac.           |
| 3. | Pavement & Others = | 3.877 ac.           |
| 4. | Green Areas =       | 2.530 ac.           |
| 5. | <b>Total =</b>      | <b><u>8.113</u></b> |

**DESIGN CRITERIA:**

**A. WATER QUALITY CRITERIA:**

1. If a wet detention system, then whichever is the greater of the following:
  - a. The first inch of runoff from the entire project site.
  - b. The amount of 2.5 inches times the percent impervious for the project site.
2. If a dry detention system, then 75% of the volume required for the wet detention system.
3. If a retention system, then 50% of the volume required.
4. If the property is zoned "Commercial", at least 0.5 inches of retention or dry detention pre-treatment will be required.
5. Any detention system shall be designed to discharge no more than 0.5 inches of the detained volume per day.

**B. WATER QUANTITY CRITERIA:**

**1. DESIGN EVENTS AND RAINFALL AMOUNTS:**

- a. Design Event for Minimum Road Elevation:
 

Frequency:	10 year
Duration:	1 day
Amount:	9.00 inches
- b. Design Event for Minimum Discharge Elevation:
 

Frequency:	25 year
Duration:	3 day
Amount:	15.00 inches
- c. Design Event for Minimum Finish Floor Elevation:
 

Frequency:	100 year
Duration:	3 day
Amount:	20.00 inches

**2. ADDITIONAL DESIGN INFORMATION:**

- a. Design Water / Control Elevation: 6.50 NAVD.
- b. Drainage Basin / Canal Number: S.F.W.M.D. C-14 BASIN

## **COMPUTATIONS:**

### **A. WATER QUALITY COMPUTATIONS:**

1. Compute the first inch of runoff from the entire developed project site:
 
$$= 1.00 \text{ inch} \times 8.113 \text{ acres} \times (1 \text{ foot} / 12 \text{ inches})$$

$$= \underline{\underline{0.676 \text{ ac-ft for the first inch of runoff}}}$$
2. Compute 2.5 inches times the percent impervious for the developed project site:
  - a. Site area for water quality pervious / impervious calculations only:
 
$$= \text{Total Project} - (\text{Lake Area} + \text{Buildings})$$

$$= 8.113 \text{ acres} - (0.000 \text{ acres} + 1.706 \text{ acres})$$

$$= \underline{\underline{6.407 \text{ acres of site area for water quality calculations}}}$$
  - b. Impervious area for water quality pervious / impervious calculations only:
 
$$= \text{Site area for water quality} - \text{Pervious area}$$

$$= 6.407 \text{ acres} - 2.530 \text{ acres}$$

$$= \underline{\underline{3.877 \text{ acres of impervious area for water quality calculations}}}$$
  - c. Percentage of impervious area for water quality:
 
$$= \text{Impervious area for water quality} / \text{Site area for water quality} \times 100\%$$

$$= 3.877 \text{ acres} / 6.407 \text{ acres} \times 100\%$$

$$= \underline{\underline{60.51 \% \text{ Impervious}}}$$
  - d. For 2.5 inches times the percentage of impervious area:
 
$$= 2.5 \text{ inches} \times 60.51 \%$$

$$= \underline{\underline{1.513 \text{ inches to be treated}}}$$
  - e. Compute volume required for quality detention:
 
$$= \text{Inches to be treated} \times (\text{Total Site Area} - \text{Lake Area})$$

$$= 1.513 \text{ inches} \times (8.113 \text{ acres} - 0.000 \text{ acres}) \times (1 \text{ foot} / 12 \text{ inches})$$

$$= \underline{\underline{1.023 \text{ ac-ft required for detention storage}}}$$
3. The first inch of runoff from the entire developed site = 0.676 ac-ft  
 2.5 inches times the percentage of impervious area = 1.023 ac-ft

**The volume of 1.023 ac-ft controls**

4. If the project is zoned "Commercial" or if the project were discharging directly to a sensitive receiving body and is more than 40% impervious, 0.5 inches of dry detention pre-treatment must be provided:
 
$$= 0.5 \text{ inches} \times (\text{Total Site Area} - \text{Lake Area})$$

$$= 0.5 \text{ inches} \times (8.113 \text{ acres} - 0.000 \text{ acres}) \times (1 \text{ foot} / 12 \text{ inches})$$

$$= \underline{\underline{0.338 \text{ ac-ft required for pre-treatment}}}$$
5. Compute credit for using one of the following systems:
  - a. Wet detention volume to be provided:
 
$$= \text{Total required detention} - \text{Pre-treatment}$$

$$= 1.023 \text{ ac-ft} - 0.338 \text{ ac-ft}$$

$$= \underline{\underline{0.685 \text{ ac-ft of volume required for wet detention}}}$$
  - b. Dry detention volume to be provided ( 75% of the total required detention volume ):
 
$$= \text{Total required detention volume} \times 75\%$$

$$= 1.023 \text{ ac-ft} \times 75\%$$

$$= \underline{\underline{0.767 \text{ ac-ft of volume required for dry detention}}}$$
  - c. Dry retention volume to be provided ( 50% of the total required detention volume ):
 
$$= \text{Total required detention volume} \times 50\%$$

$$= 1.023 \text{ ac-ft} \times 50\%$$

$$= \underline{\underline{0.511 \text{ ac-ft of volume required for dry retention}}}$$

**B. SUMMARY OF WATER QUALITY COMPUTATIONS:**

Item:	Description:	Quantity:
A.1	First inch of runoff from entire project site =	0.676 ac-ft
A.2	2.5 inches times percent impervious =	1.023 ac-ft
A.3	Volume to be treated =	1.023 ac-ft
A.4	Pre-treatment required for commercial site =	0.338 ac-ft
A.5.a	Wet detention volume required =	0.685 ac-ft
A.5.b	Dry detention volume required =	0.767 ac-ft
A.5.c	Dry retention volume required =	0.511 ac-ft
A.5.d	Exfiltration trench volume required =	1.023 ac-ft

**C. STAGE ELEVATION INFORMATION:**

Item:	Description:	S type	Area ac.	Low ft.	High ft.	I %	C %	Total Area %
1	n / a	V						0.00
2	n / a	L						0.00
3	n / a	V						0.00
4	n / a	L						0.00
5	n / a	V						0.00
6	n / a	L						0.00
7	n / a	L						0.00
8	Green Areas	L	2.530	11.00	12.50	0	50	31.18
9	Pavement Areas	L	3.167	11.00	12.50	100	100	39.04
10	Sidewalk Areas	L	0.710	11.50	13.00	100	100	8.75
11	Building Coverage Area	V	1.706	13.00	13.50	100	100	21.03
<b>Total:</b>			<b>8.113</b>	<b>11.00</b>	<b>13.50</b>	<b>68.82</b>	<b>84.41</b>	<b>100.0</b>

\* Abbreviations: S = Storage; ( V = Vertical Storage & L = Linear Storage )  
I = Impervious  
C = Compaction; ( Use the following compaction factors: 0%, 50%, 100% )

**D. SCS CURVE NUMBER AND SOIL STORAGE CALCULATIONS:**

1. Soil Moisture Storage Table:  
Existing Soil Type: **3** DEPRESSIONAL

Depth to Water Table ft.	Cumulative Water Storage ( Pre.-Dev. ) in.	Compacted Water Storage ( Post 50% ) in.	Compacted Water Storage ( Post 100% ) in.
1	0.60	0.53	0.45
2	2.10	1.84	1.58
3	4.40	3.85	3.30
4	6.80	5.95	5.10

2. Available Soil Storage Calculation:

Item:	Description:	Elev. ft.	S in.	Area acres	Stored ac-in
1	n / a	0.00	0.00	0.000	0.00
2	n / a	0.00	0.00	0.000	0.00
3	n / a	0.00	0.00	0.000	0.00
4	n / a	0.00	0.00	0.000	0.00
5	n / a	0.00	0.00	0.000	0.00
6	n / a	0.00	0.00	0.000	0.00
7	n / a	0.00	0.00	0.000	0.00
8	Green Areas	11.75	5.95	2.530	15.05
9	Pavement Areas	11.75	5.10	0.000	0.00
10	Sidewalk Areas	12.25	5.10	0.000	0.00
11	Building Coverage Area	13.25	5.10	0.000	0.00
<b>Total:</b>		<b>11.75</b>	<b>21.25</b>	<b>2.530</b>	<b>15.05</b>

\* Abbreviations: S = Soil Storage  
P = Pervious

3. Moisture Storage Calculation ( S ):  
= Available soil storage / Total Site Area  
= 15.05 ac-in / 8.113 acres  
= **1.86 inches**
4. SCS Curve Number Calculation ( CN ):  
= 1000 / ( S + 10 )  
= 1000 / ( 1.855 + 10 )  
= **84**

#### E. SURFACE STORAGE CALCULATIONS:

1. Stage vs. Storage Calculations:

Stage ft.	Item:	STORAGE ( ac-ft )											Total ac-ft
		1 ac-ft	2 ac-ft	3 ac-ft	4 ac-ft	5 ac-ft	6 ac-ft	7 ac-ft	8 ac-ft	9 ac-ft	10 ac-ft	T ac-ft	
6.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
7.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.13	0.13
7.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.26	0.26
8.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.39	0.39
8.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.52	0.52
9.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.65	0.65
9.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.78	0.78
10.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.78	0.78
10.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.78	0.78
11.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.78	0.78
11.25		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.05	0.07	0.00	0.78	0.90
11.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.21	0.26	0.00	0.78	1.26
11.75		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.47	0.59	0.01	0.78	1.87
12.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.84	1.06	0.06	0.78	2.74
12.25		0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.32	1.65	0.13	0.78	3.88
12.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	1.90	2.38	0.24	0.78	5.29
12.75		0.00	0.00	0.00	0.00	0.00	0.00	0.00	2.53	3.17	0.37	0.78	6.85
13.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	3.16	3.96	0.53	0.78	8.44
13.50		0.00	0.00	0.00	0.00	0.00	0.00	0.00	4.43	5.54	0.89	0.78	11.64
14.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00	5.69	7.13	1.24	0.78	14.84

\* Abbreviations: T = Exfiltration Trench

#### E. MINIMUM BUILDING FINISH FLOOR ELEVATION CALCULATIONS (ZERO DISCHARGE):

1. The rainfall amount for the 100-Year, 3-Day storm event:  
= **20.00 in.**
2. Compute inches of runoff, Q:  
=  $(P - (0.2 S))^2 / (P + (0.8 X S))$   
=  $(20.00 \text{ in.} - (0.2 X 1.86 \text{ in.}))^2 / (20.00 \text{ in.} + (0.8 X 1.86 \text{ in.}))$   
= **17.93 inches of runoff**
3. Compute volume of runoff:  
= (Inches of Runoff) X (Project Area)  
= 17.93 inches X 8.113 acres X (1 foot / 12 inches)  
= **12.12 ac-ft of storage required (zero discharge)**
4. From the stage vs storage curve, **12.12** ac-ft corresponds to elevation **13.58 NGVD.**



**G. MINIMUM DISCHARGE ELEVATION CALCULATIONS (ZERO DISCHARGE):**

1. The rainfall amount for the 25-Year, 3-Day storm event:  
= **15.00 in.**
2. Compute inches of runoff, Q:  
=  $(P - (0.2 S))^2 / (P + (0.8 X S))$   
=  $(15.00 \text{ in.} - (0.2 \times 1.86 \text{ in.}))^2 / (15.00 \text{ in.} + (0.8 \times 1.86 \text{ in.}))$   
= **12.98 inches of runoff**
3. Compute volume of runoff:  
= (Inches of Runoff) X (Project Area)  
= 12.98 inches X 8.113 acres X (1 foot / 12 inches)  
= **8.78 ac-ft of storage required (zero discharge)**
4. From the stage vs storage curve, **8.78** ac-ft corresponds to elevation **13.05 NGVD.**

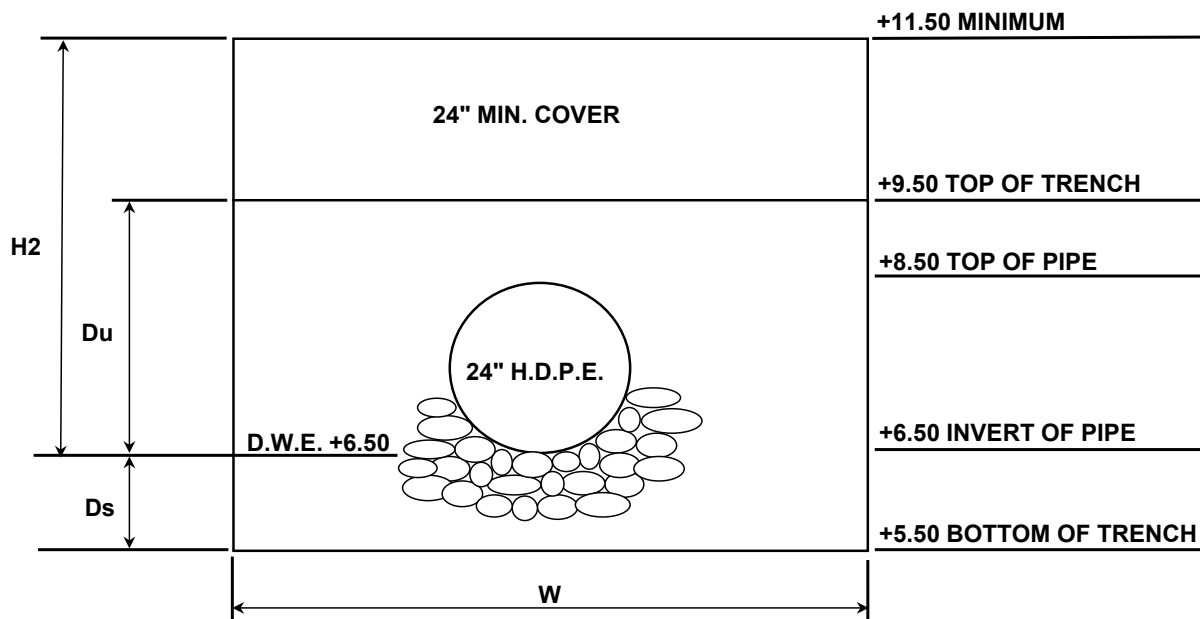
**H. MINIMUM ROAD CROWN ELEVATION CALCULATIONS (ZERO DISCHARGE):**

1. The rainfall amount for the 10-Year, 1-Day storm event:  
= **9.00 in.**
2. Compute inches of runoff, Q:  
=  $(P - (0.2 S))^2 / (P + (0.8 X S))$   
=  $(9.00 \text{ in.} - (0.2 \times 1.86 \text{ in.}))^2 / (9.00 \text{ in.} + (0.8 \times 1.86 \text{ in.}))$   
= **7.10 inches of runoff**
3. Compute volume of runoff:  
= (Inches of Runoff) X (Project Area)  
= 7.10 inches X 8.113 acres X (1 foot / 12 inches)  
= **4.80 ac-ft of storage required (zero discharge)**
4. From the stage vs storage curve, **4.80** ac-ft corresponds to elevation **12.41 NGVD.**



#### I. EXFILTRATION TRENCH CALCULATIONS:

1. Design Formula:  $L = (FS ((\%WQ*V1) + V2)) / (K((H2*W) + (2*H2*Du) - (Du^2) + (2*H2*Ds)) + (1.39 \times 10^{-4} * W * Du))$
2. Design Information:
  - V1 = Exfiltrated Volume (Pre-treatment): 0.00 ac-in
  - V2 = Exfiltrated Volume (Additional Storage): 0.00 ac-in
  - W = Trench Width: 8.00 ft.
  - K = Hydraulic Conductivity: 1.00E-04 cfs/sq-ft per ft head
  - H2 = Depth of Water Table: 5.00 ft.
  - Du = Non-Saturated Trench Depth: 3.00 ft.
  - Ds = Saturated Trench Depth: 1.00 ft.
3. Exfiltration Trench Provided: **1,800 ft.**
4. Exfiltration Trench Storage Provided: **9.39 ac-in** or **0.78 ac-ft**



#### J. PRE VS. POST DEVELOPMENT CALCULATION SUMMARY:

MARQUESA (NAVD.)			
PHASE	10 Y - 1 D	25 Y - 3 D	100 Y - 3D
PRE-DEVELOPMENT	12.41	13.06	13.58
POST-DEVELOPMENT	12.41	13.05	13.58

# REFERENCE MATERIAL

*Prepared By:*



**HSQ GROUP, INC.**

Engineers • Planners • Surveyors  
5951 Northwest 173rd Drive, Suite 4  
Miami, Florida 33015  
(786) 534-3621 Phone

# National Flood Hazard Layer FIRMette



## Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS		Without Base Flood Elevation (BFE) Zone A, V, A99
		With BFE or Depth Zone AE, AO, AH, VE, AR
		Regulatory Floodway
OTHER AREAS OF FLOOD HAZARD		0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X
		Future Conditions 1% Annual Chance Flood Hazard Zone X
		Area with Reduced Flood Risk due to Levee. See Notes. Zone X
		Area with Flood Risk due to Levee Zone D
OTHER AREAS		NO SCREEN Area of Minimal Flood Hazard Zone X
		Effective LOMRs
GENERAL STRUCTURES		Area of Undetermined Flood Hazard Zone D
		Channel, Culvert, or Storm Sewer
		Levee, Dike, or Floodwall
OTHER FEATURES		20.2 Cross Sections with 1% Annual Chance Water Surface Elevation
		17.5
		Coastal Transect
		Base Flood Elevation Line (BFE)
		Limit of Study
		Jurisdiction Boundary
MAP PANELS		Coastal Transect Baseline
		Profile Baseline
		Hydrographic Feature
		Digital Data Available
		No Digital Data Available
		Unmapped



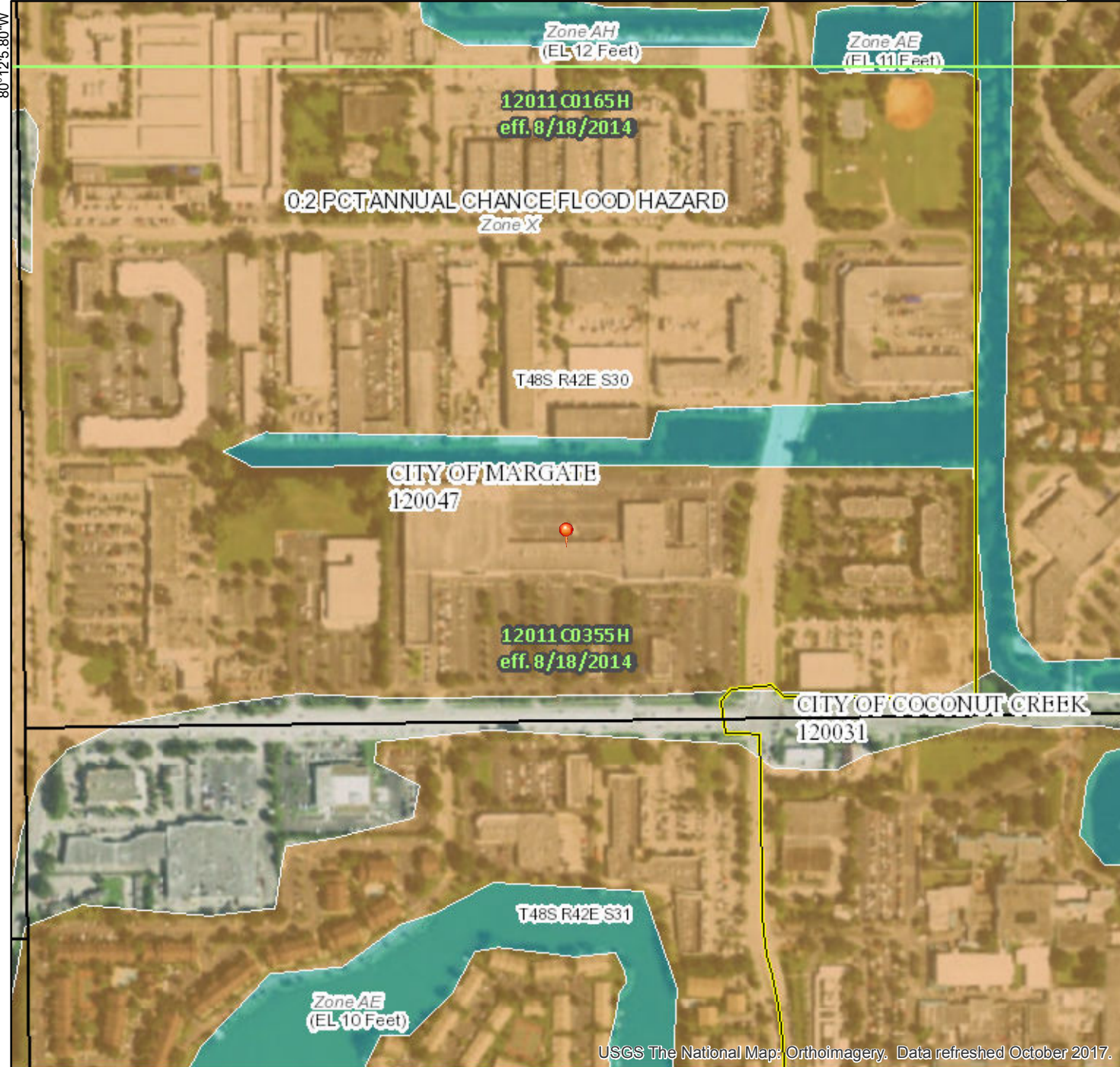
The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **8/25/2018 at 9:34:25 PM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

26°15'2.01"N



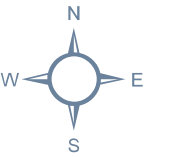
USGS The National Map: Orthoimagery. Data refreshed October 2017.

0 250 500 1,000 1,500 2,000 Feet 1:6,000

26°14'29.74"N

80°1'28.35"W



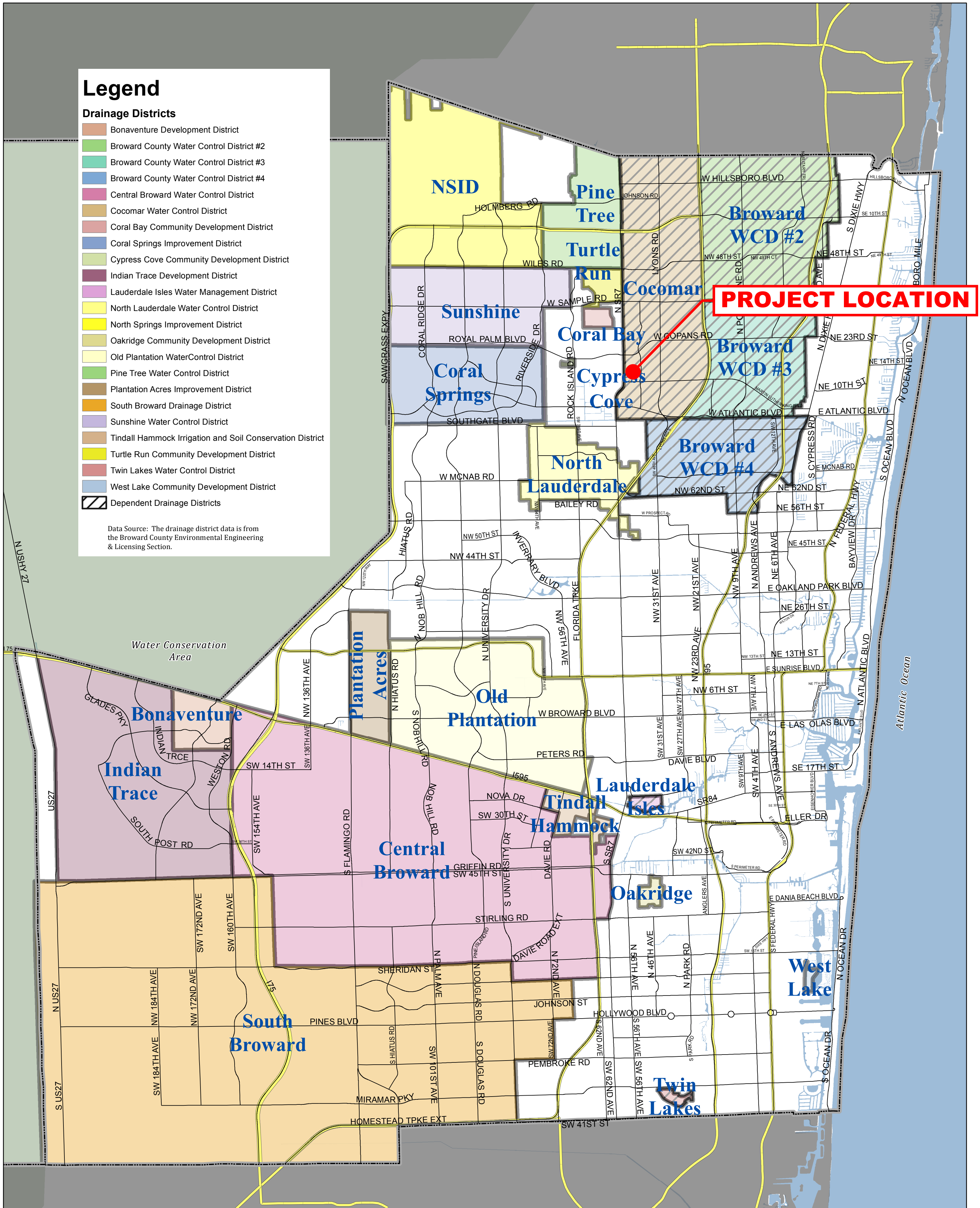


## Legend

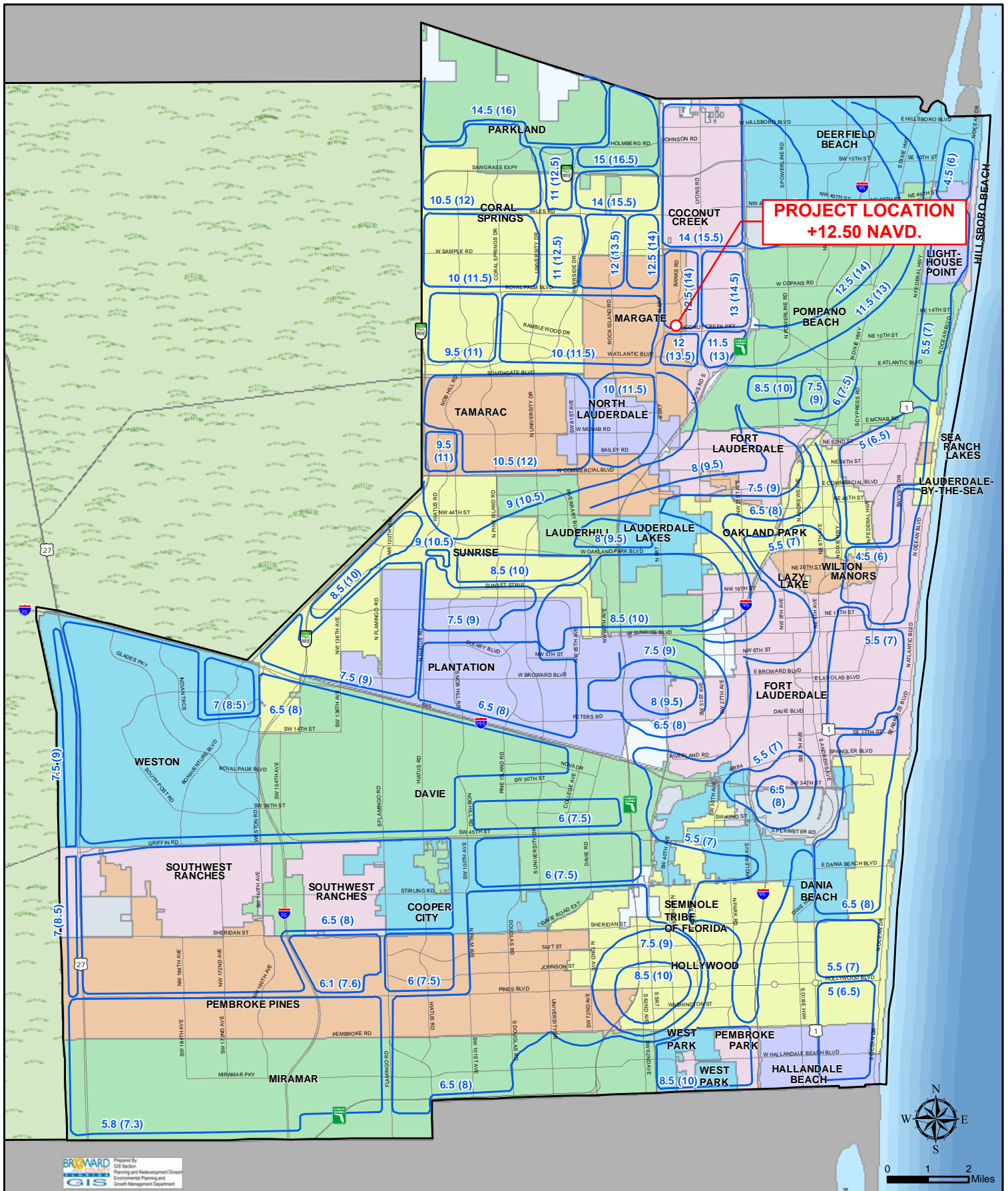
### Drainage Districts

- Bonaventure Development District
- Broward County Water Control District #2
- Broward County Water Control District #3
- Broward County Water Control District #4
- Central Broward Water Control District
- Cocomar Water Control District
- Coral Bay Community Development District
- Coral Springs Improvement District
- Cypress Cove Community Development District
- Indian Trace Development District
- Lauderdale Isles Water Management District
- North Lauderdale Water Control District
- North Springs Improvement District
- Oakridge Community Development District
- Old Plantation Water Control District
- Pine Tree Water Control District
- Plantation Acres Improvement District
- South Broward Drainage District
- Sunshine Water Control District
- Tindall Hammock Irrigation and Soil Conservation District
- Turtle Run Community Development District
- Twin Lakes Water Control District
- West Lake Community Development District
- Dependent Drainage Districts

Data Source: The drainage district data is from the Broward County Environmental Engineering & Licensing Section.







**100 Year Flood Contours NAVD (NGVD)**  
**Example: 6.5 (8)**

This map is for conceptual purposes only and should not be used for legal boundary determinations.

Elevations converted from NGVD to NAVD using the FEMA approved conversion factor for Broward County of (-)1.5, based on 1997 FEMA Flood Data

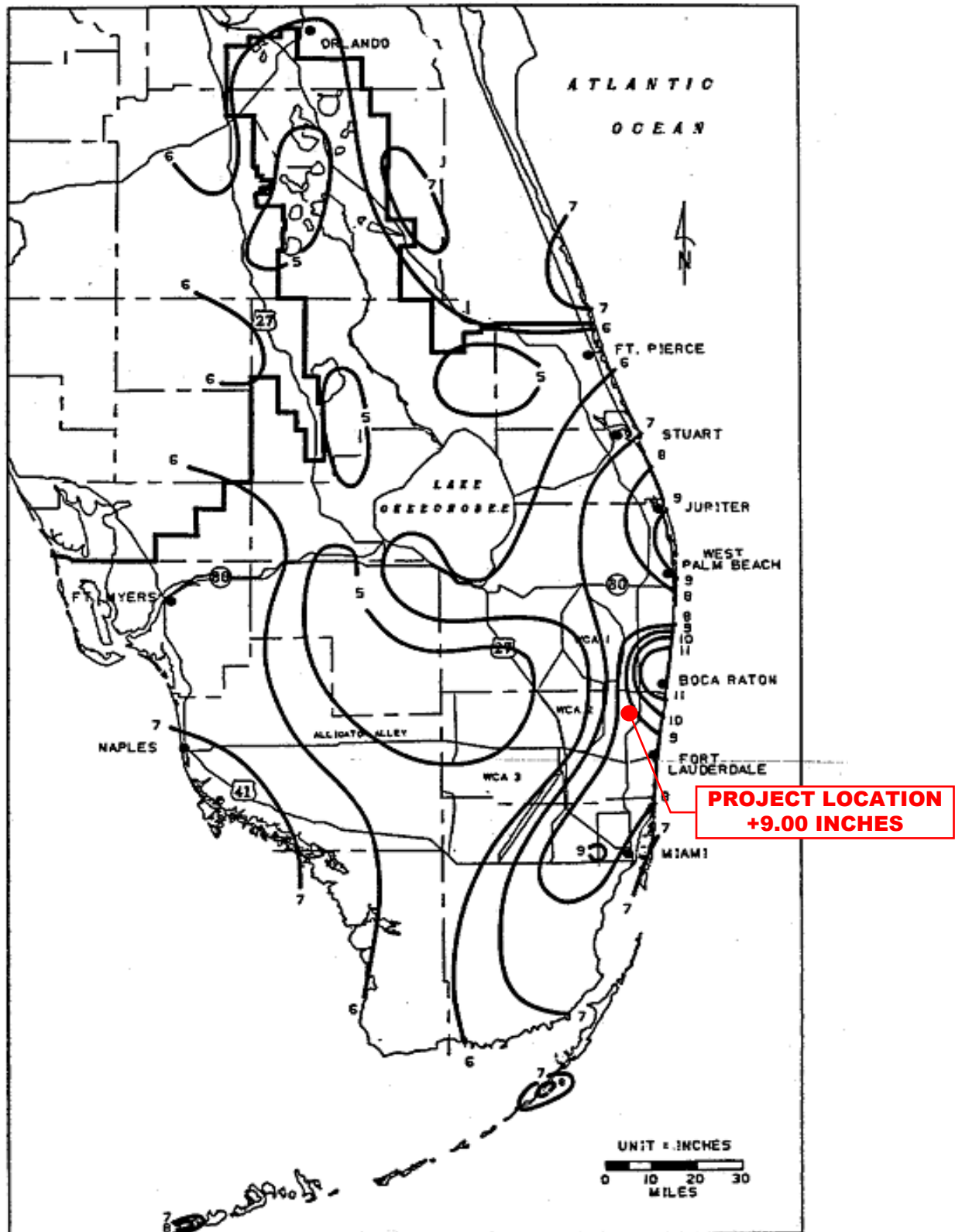


FIGURE C-4. 1-DAY RAINFALL: 10-YEAR RETURN PERIOD

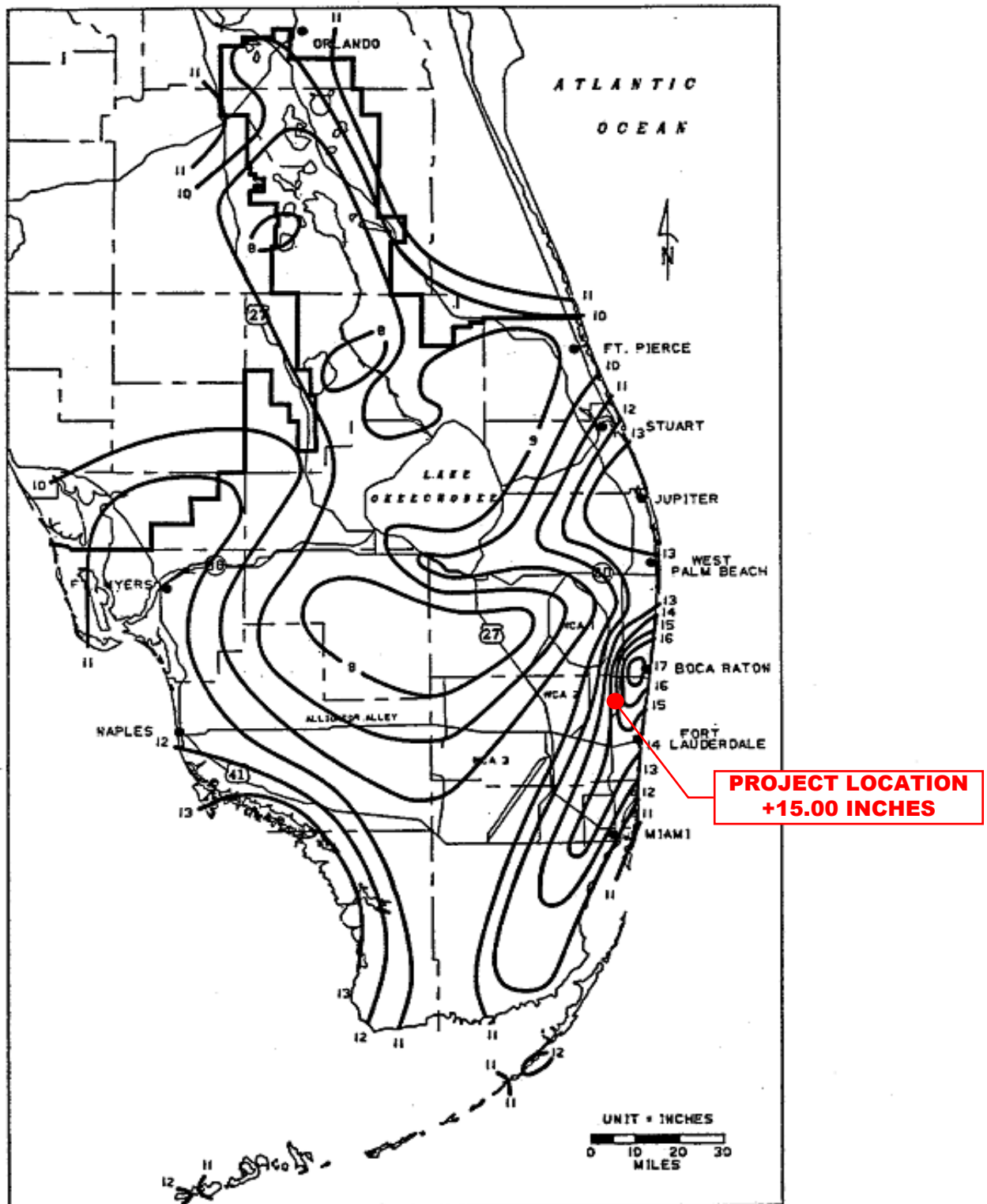


FIGURE C-8. 3-DAY RAINFALL: 25-YEAR RETURN PERIOD

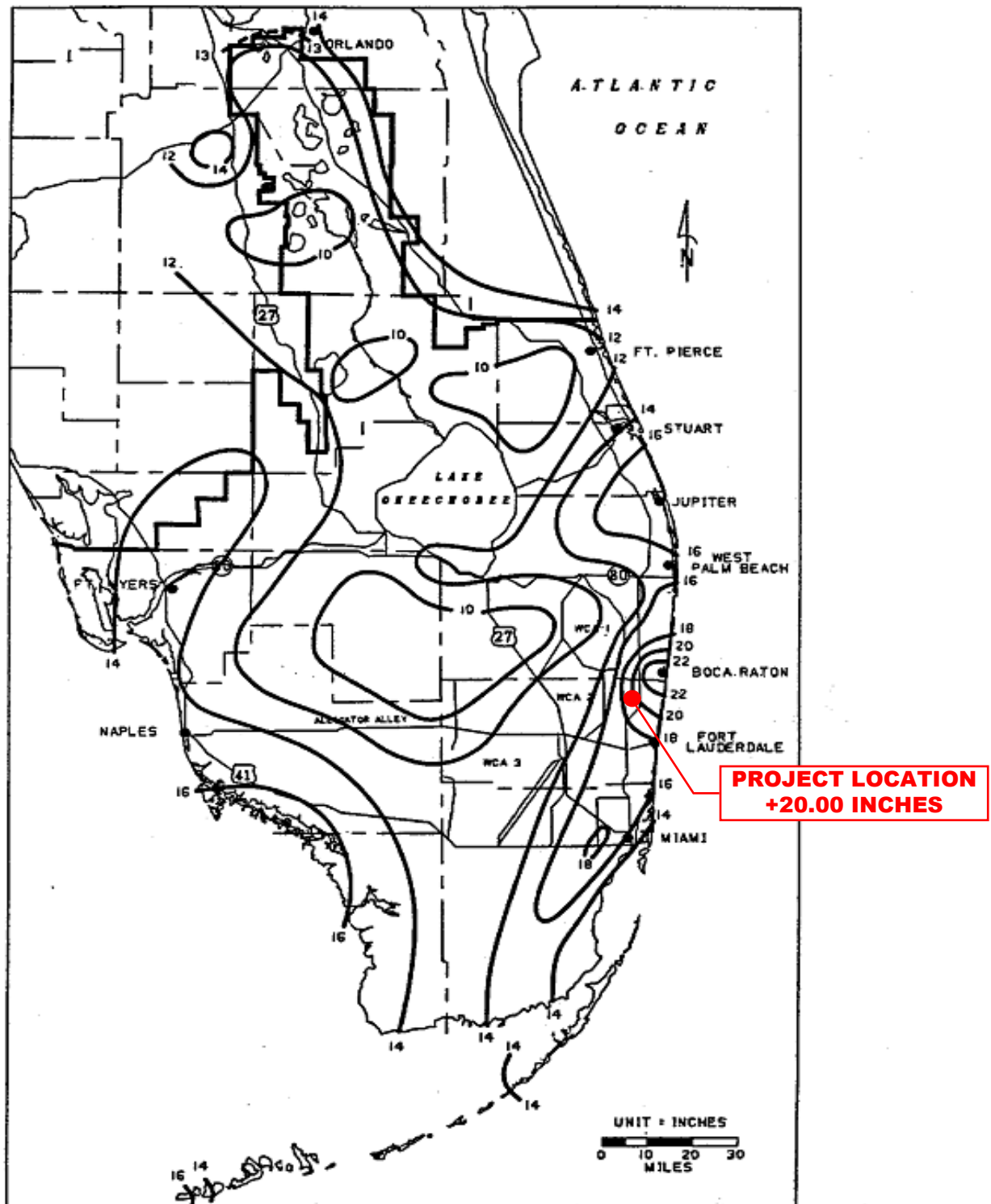


FIGURE C-9. 3-DAY RAINFALL: 100-YEAR RETURN PERIOD